DESIGN LOADS & DESIGN DATA

1.	BUILDING CODE			2021 INTERNATIONAL BUILDING CODE (IBC)/ ASCE/ SEI 7-22 WITH LOCAL COUNTY AMENDMENTS	
2.	GRAVITY LOADS AND DESIGN DATA				
	A.	DEAD LOADS			
		1.	ROOF		
			STEEL FRAMING, METAL DECK, ETC.	SELF-WEIGHT	
			COLLATERAL	AS NOTED ON LOADING DIAGRAMS	
		2.	FLOOR		
			STEEL FRAMING, COMPOSITE SLAB, ETC.	SELF-WEIGHT	
			COLLATERAL	AS NOTED ON LOADING DIAGRAMS	
		3.	ROOF TOP MECHANICAL PLATFORM		
			STEEL FRAMING, GRATING, ETC.	SELF-WEIGHT	
			COLLATERAL	AS NOTED ON LOADING DIAGRAMS	
	B.	LIVE LOADS		UNIFORM	CONCENTRATED
		1.	ROOF	AS NOTED ON LOADING DIAGRAMS	300 LBS
		2.	FLOOR	AS NOTED ON LOADING DIAGRAMS	2,500 LBS
		3.	ROOF TOP MECHANICAL PLATFORM	AS NOTED ON LOADING DIAGRAMS	300 LBS
		4.	STAIRS & EXIT WAYS	100 PSF (UNREDUCIBLE)	300 LBS
	C.	SNOW LOADS			
		1.	GROUND SNOW LOAD, P g	88 PSF	
		2.	FLAT SNOW LOAD, Pf	72 PSF	
		3.	SNOW EXPOSURE FACTOR, Ce	1.0	
		4.	SNOW OCCUPANCY CATEGORY	IV	
		5.	SNOW LOAD IMPORTANCE FACTOR, I	1.0	
		6.	THERMAL FACTOR, Ct	1.18	
		7.	SNOW DRIFTS	AS NOTED ON LOADING DIAGRAMS	
	D.	1.	1-HR RAIN INTENSITY, i	3.42 IN/H	
3.	LATERAL LOADS & DESIGN DATA				
	A.	WIND DESIGN DATA			
		1.	ULTIMATE DESIGN WIND SPEED, V ult	125 MPH (3-SECOND GUST) VOLUNTARY CLIENT - REQUESTED UPGRADE	
		2.	NOMINAL DESIGN WIND SPEED, V asd	96.8 MPH (3-SECOND GUST)	
		3.	WIND RISK CATEGORY	IV	
		4.	WIND IMPORTANCE FACTOR, I	1.0	
		5.	WIND EXPOSURE CATEGORY	C	
		6.	INTERNAL PRESSURE COEFFICIENT, GC pi	±0.18	
		7.	MEAN ROOF HEIGHT, h (DATA HALL BUILDING)	45'-4"	
		8.	WIDTH OF END ZONE, (a) (DATA HALL BUILDING)	17'-1"	
		9.	MEAN ROOF HEIGHT, h (OFFICE BUILDING)	39'-4 1/4"	
		10.	WIDTH OF END ZONE, (a) (OFFICE BUILDING)	15'-9"	

B. COMPONENTS AND CLADDING (DATA HALL BUILDING)

1. WALLS — SURFACE PRESSURE (PSF)

AREA	10 SF	100 SF	500 SF
NEGATIVE ZONE 4	-42.5	-36.7	-32.7
NEGATIVE ZONE 5	-52.3	-40.8	-32.7
POSITIVE ZONE 4 & 5	39.2	33.5	29.4

2. PARAPET — SURFACE PRESSURE (PSF)

AREA	10 SF	100 SF	500 SF
CASE A: ZONE 2	120.5	94.5	76.4
CASE A: ZONE 3	120.5	94.5	76.4
CASE B: ZONE 2	-71.2	-59.2	-50.8
CASE B: ZONE 3	-81.3	-63.4	-50.8

3. ROOF — SURFACE PRESSURE (PSF)

AREA	10 SF	50 SF	100 SF
NEGATIVE ZONE 1	-68.3	-57.8	-53.3
NEGATIVE ZONE 1'	-39.2	-39.2	-39.2
NEGATIVE ZONE 2 & 3	-90.1	-76.7	-70.9
POSITIVE ZONE 1 & 1'	17.4	16.0	16.0
POSITIVE ZONE 2 & 3	39.2	35.2	33.5
OVERHANG ZONE 1 & 1'	-61.8	-59.2	-58.1
OVERHANG ZONE 2 & 3	-83.6	-65.6	-57.9

C. COMPONENTS AND CLADDING (OFFICE BUILDING)

1. WALLS — SURFACE PRESSURE (PSF)

AREA	10 SF	100 SF	500 SF
NEGATIVE ZONE 4	-41.4	-35.8	-31.8
NEGATIVE ZONE 5	-50.9	-39.7	-31.8
POSITIVE ZONE 4 & 5	38.2	32.6	28.6

2. PARAPET — SURFACE PRESSURE (PSF)

AREA	10 SF	100 SF	500 SF
CASE A: ZONE 2	114.7	90.0	72.8
CASE A: ZONE 3	147.0	103.3	72.8
CASE B: ZONE 2	-67.8	-56.4	-48.4
CASE B: ZONE 3	-77.4	-60.3	-48.4

3. ROOF — SURFACE PRESSURE (PSF)

AREA	10 SF	50 SF	100 SF
NEGATIVE ZONE 1	-66.5	-56.3	-51.9
NEGATIVE ZONE 1'	-38.2	-38.2	-38.2
NEGATIVE ZONE 2	-87.7	-74.6	-69.0
NEGATIVE ZONE 3	-119.5	-93.4	-82.1
POSITIVE ALL ZONES	17.0	16.0	16.0
OVERHANG ZONE 1 & 1'	-60.1	-57.7	-56.6
OVERHANG ZONE 2	-81.3	-63.9	-56.4
OVERHANG ZONE 3	-113.2	-82.6	-69.5

	D.	ROOFTOP MECHANICAL PLATFORM (ASCE 7-16, SEC 29.4.1)		
		1.	BUILDING LENGTH, L	1071'-1 3/4"
		2.	BUILDING WIDTH, B	171'-4"
		3.	PRODUCT OF EXTERNAL PRESSURE COEFFICIENT AND GUST-EFFECT FACTOR TO BE USED IN DETERMINATION OF WIND LOADS FOR ROOFTOP STRUCTURES. (GCr)	1.5
		4.	BASE PRESSURE, q h	36.3 PSF
		5.	WIND NORMAL TO BUILDING LENGTH	485.2 KIPS
		6.	WIND NORMAL TO BUILDING WIDTH	98.2 KIPS
		7.	VERTICAL WIND PRESSURE	1368.8 KIPS
	E.	SEISMIC DESIGN DATA (DATA HALL BUILDING & OFFICE BUILDING)		
		1.	SEISMIC OCCUPANCY CATEGORY	IV
		2.	SEISMIC IMPORTANCE FACTOR, I	1.50
		3.	SHORT PERIOD MAPPED SPECTRAL RESPONSE ACCELERATION, S s	0.170g
		4.	1-SEC PERIOD MAPPED SPECTRAL RESPONSE ACCELERATION, S 1	0.041g
		5.	SITE CLASS	CD
		6.	SHORT PERIOD SPECTRAL RESPONSE COEFFICIENT, S DS	0.130g
		7.	1-SEC PERIOD SPECTRAL RESPONSE COEFFICIENT, S D1	0.050g
		8.	SEISMIC DESIGN CATEGORY	A
		9.	RESPONSE MODIFICATION FACTOR, R	3.00
		10.	SYSTEM OVERSTRENGTH FACTOR, Ω	3.00
		11.	DEFLECTION AMPLIFICATION FACTOR, Cd	3.00
		12.	SEISMIC RESPONSE COEFFICIENT, Cs	0.001
		13.	DESIGN BASE SHEAR, V	0.001W
		14.	BASIC SEISMIC FORCE-RESISTING SYSTEM	STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
		15.	ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PROCEDURE
	F.	SEISMIC DESIGN DATA (ROOFTOP MECHANICAL PLATFORM)		NONBUILDING STRUCTURES SIMILAR TO BUILDINGS, SECTION 15.3.1, ASCE 7-16
		1.	HEIGHT IN STRUCTURE OF POINT OF ATTACHMENT OF COMPONENT WITH RESPECT TO THE BASE, z	55'-0"
		2.	SEISMIC DESIGN CATEGORY	C
		3.	MECHANICAL AND ELECTRICAL COMPONENT	ROOF-MOUNTED STACKS, COOLING AND ELECTRICAL TOWERS LATERALLY BRACED BELOW THEIR CENTER OF MASS, ASCE 7-16, TABLE 13.6-1
		4.	COMPONENT AMPLIFICATION FACTOR, a p	2.50
		5.	COMPONENT RESPONSE MODIFICATION FACTOR, R p	3.00
		6.	OVER-STRENGTH FACTOR, Ω 0	2.00
		7.	SEISMIC DESIGN FORCE, F p	0.30W (ASCE 7-16, SEC 13.3.1)
4.	DEFLECTION LIMITS			
	A.	ROOF MEMBERS		
		1.	LIVE LOAD MAXIMUM ALLOWABLE DEFLECTION	L/360, REF ROOF DEFLECTION DIAGRAMS FOR ADDITIONAL INFORMATION
		2.	TOTAL LOAD MAXIMUM ALLOWABLE DEFLECTION	L/240, REF ROOF DEFLECTION DIAGRAMS FOR ADDITIONAL INFORMATION
	B.	FLOOR MEMBERS		
		1.	LIVE LOAD MAXIMUM ALLOWABLE DEFLECTION	L/360, REF FLOOR DEFLECTION DIAGRAMS FOR ADDITIONAL INFORMATION
		2.	TOTAL LOAD MAXIMUM ALLOWABLE DEFLECTION	L/240, REF FLOOR DEFLECTION DIAGRAMS FOR ADDITIONAL INFORMATION
	C.	BUILDING FRAME		
		1.	MAXIMUM ALLOWABLE STORY DRIFT	H/400 (BASED ON 50-YR WIND)
5.	FIRE RESISTANCE RATING INFORMATION			
	A.	STRUCTURAL FRAME		
		1.	FRAME RATING	UNRESTRAINED

4 DESIGN LOADS AND DESIGN DATA
12" = 1'-0"

QTS Freedom Standard - Version Matrix
Project: QTS DEN1-DC2

V3.0-rev2

Design Deliverable:
QTS Design Standards will be utilized as the Basis Of Design for the Data Center Building.
Design Consultant team to notate Version of QTS Design Standards.

Design Discipline	QTS Design Standard		
	Design Narrative	DD Drawing Set	Reference File
Civil	V3.0	N/A	
General	V3.0	V3.0	
Structural	V3.0	V3.0	
Architectural	V3.0	V3.0	
Fire Alarm	V3.0	V3.0	
Fire Protection	V3.0	V3.0	
Plumbing	V3.0	V3.0	
Process (I&C)	See Below	V3.0	
Mechanical	V3.0	V3.0	
Electrical	V3.0	V3.0	
Technology (Telecom)	V3.0	V3.0	
Audio Visual	N/A	V3.0	
Security	V3.0	V3.0	
Interior Standards	V3.0	See Architectural	3.0-rev1
Signage			INTrev4 EXTrev3
SDIs	N/A	N/A	SDI-018(v3.0) thru 44
OFCI Catalog	V4.0		
SOO Catalog	V2.0		

Design Discipline	Q-Systems Design Standard BODs Version
Data Telecom BOD	Freedom Standard - Data Telecom - 04.14.2024
Physical Security BOD	Freedom Standard - Security Systems - 05.10.2024
DCIM - BMS BOD	Freedom Standard - BMS Content Consolidation - 04.16.2024
DCIM - EPMS BOD	Freedom Standard - EPMS Content Consolidation - 04.16.2024

ABBREVIATION LEGEND

ABBR	DEFINITION	ABBR	DEFINITION
AB	ANCHOR BOLT	LLH	LONG LEG HORIZONTAL
ACI	AMERICAN CONCRETE INSTITUTE	LLV	LONG LEG VERTICAL
ADD'L	ADDITIONAL	LO	LOW
AFF	ABOVE FINISHED FLOOR	LOC	LOCATION
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LONG	LONGITUDINAL
		LSH	LONG SIDE HORIZONTAL
AISI	AMERICAN IRON AND STEEL INSTITUTE	LSV	LONG SIDE VERTICAL
ARCH	ARCHITECTURAL	LWC	LIGHT WEIGHT CONCRETE
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	MAX	MAXIMUM
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	MC	MOMENT CONNECTION
AWS	AMERICAN WELDING SOCIETY	MECH	MECHANICAL
BFF	BELOW FINISHED FLOOR	MFR	MANUFACTURER
BL	BLOCK LINTEL	MIN	MINIMUM
BM	BEAM	MISC	MISCELLANEOUS
BO	BOTTOM OF	MO	MASONRY OPENING
BOS	BOTTOM OF STEEL	MTL	METAL
BOT	BOTTOM	NIC	NOT IN CONTRACT
BP	BASE PLATE	NO	NUMBER
BRG	BEARING	NS	NEAR SIDE
BTWN	BETWEEN	NTS	NOT TO SCALE
CGS	CENTROID OF TENDONS	NWC	NORMAL WEIGHT CONCRETE (150 PCF)
CIP	CAST IN PLACE	OC	ON CENTER
CJ	CONTROL JOINT	OD	OUTSIDE DIAMETER
CL	CENTER LINE	OH	OPPOSITE HAND
CJP	COMPLETE JOINT PENETRATION	PAF	POWDER ACTUATED FASTENER
CLR	CLEAR	PCC	PRECAST CONCRETE
CMU	CONCRETE MASONRY UNIT	PCF	POUNDS PER CUBIC FOOT
COL	COLUMN	PCI	PRECAST AND PRESTRESSED CONCRETE INSTITUTE
CONN	CONNECTION	PE	PROFESSIONAL ENGINEER
CONC	CONCRETE	PEMB	PRE-ENGINEERED METAL BUILDING
CONST	CONSTRUCTION	PENTR	PENETRATION
CONT	CONTINUOUS	PL	PLATE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	PLAM	PLASTIC LAMINATE
DIA	DIAMETER	PLF	POUNDS PER LINEAR FOOT
DBA	DOWEL BAR ANCHOR	PP	PARTIAL PENETRATION
DL	DEAD LOADS	PSF	POUNDS PER SQUARE FOOT
DT	DOUBLE TEE	PSI	POUNDS PER SQUARE INCH
EA	EACH	PT	POST-TENSIONING
EDC	ELECTRICAL DISTRIBUTION CENTER	PTI	POST-TENSIONING INSTITUTE
EF	EACH FACE	QTY	QUANTITY
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM	RAF	RAISED ACCESS FLOOR
EJ	EXPANSION JOINT	REF	REFER TO
EL	ELEVATION	REINF	REINFORCING
ELEC	ELECTRICAL	REQD	REQUIRED
EQ	EQUAL	REV	REVERSE
EQUIV	EQUIVALENT	RO	ROUGH OPENING
EW	EACH WAY	RTU	ROOF TOP UNIT
EXIST	EXISTING	SCHED	SCHEDULE
EXP	EXPANSION	SE	STRUCTURAL ENGINEER
EXT	EXTERIOR	SHT	SHEET
FABR	FABRICATOR	SIM	SIMILAR
FDN	FOUNDATION	SJI	STEEL JOIST INSTITUTE
FF	FINISHED FLOOR	SOG	SLAB ON GRADE
FM	FACTORY MUTUAL	SPECS	SPECIFICATIONS
FRP	FIBER REINFORCED PLASTIC	STAG	STAGGERED
FS	FAR SIDE	STIFF	STIFFENER
FTG	FOOTING	STL	STEEL
FV	FIELD VERIFY	STRUC	STRUCTURAL
GA	GAUGE	T&B	TOP AND BOTTOM
GALV	GALVANIZED	TB	TOP OF BEAM
GC	GENERAL CONTRACTOR	TC	TRANSFER COLUMN
GER	GEOTECHNICAL ENGINEER OF RECORD	THK	THICKNESS
GR	GRADE	TO	TOP OF
GYP BD	GYPSUM BOARD	TOC	TOP OF CONCRETE
H	HEIGHT	TOF	TOP OF FOOTING
HI	HIGH	TOGB	TOP OF GRADE BEAM
HORIZ	HORIZONTAL	TOM	TOP OF MASONRY
HSA	HEADED STUD ANCHOR	TOP	TOP OF PAVING
IBC	INTERNATIONAL BUILDING CODE	TOS	TOP OF STEEL
INFO	INFORMATION	TOSC	TOP OF STRUCTURAL CONCRETE
INT	INTERIOR	TOW	TOP OF WALL
ISO	ISOLATION	TRANS	TRANSVERSE
IT	INVERTED TEE	T&S	TEMPERATURE AND SHRINKAGE
JBE	JOIST BEARING ELEVATION	TYP	TYPICAL
JST	JOIST	UL	UNDERWRITERS LABORATORIES
JT	JOINT	UNO	UNLESS NOTED OTHERWISE
KIP	1000 POUNDS	VERT	VERTICAL
KSI	KIPS PER SQUARE INCH	W	WIDTH
L	LENGTH	WP	WORK POINT
Ld	TENSION DEVELOPMENT LENGTH AS SHOWN IN REINFORCING LAP SCHEDULE	WWF	WELDED WIRE FABRIC
LB	POUNDS		
LL	LIVE LOAD		

3 ABBREVIATION LEGEND
12" = 1'-0"

DELEGATED DESIGN

D1 ENGINEERING DESIGN FOR DELEGATED DESIGN SUBMITTALS PERFORMED BY CONTRACTORS:
- A. THE CONTRACTOR OR RESPONSIBLE SUBCONTRACTOR SHALL HAVE A PROFESSIONAL ENGINEER LICENSED IN THE PROJECT JURISDICTION PERFORM ENGINEERING DESIGN OF THE DELEGATED DESIGN SUBMITTALS AS NOTED BELOW.
- B. EACH DELEGATED DESIGN SUBMITTAL SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROJECT JURISDICTION.
- C. EACH DELEGATED DESIGN SUBMITTAL SHALL BE SUBMITTED TO THE AUTHORITY HAVING JURISDICTION FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.

DD2 THE CONTRACTOR SHALL PROVIDE DELEGATED DESIGN SUBMITTAL FOR:
- A. ANCHORAGE AND BRACING OF INTERIOR MP EQUIPMENT - SEISMIC DESIGN CATEGORY C AND GREATER AND ALL EXTERIOR MEP EQUIPMENT REGARDLESS OF SEISMIC DESIGN CATEGORY INCLUDING BUT NOT LIMITED TO:
  1. CHILLERS
  2. GENERATORS
  3. AHU/CRAH
  4. SWITCHGEAR
  5. TRANSFORMERS
  6. BUSWAYS
  7. HVAC DUCTWORK
- B. DISTRIBUTED SYSTEM (PIPING AND CONDUIT) ANCHORAGE AND BRACING
- C. UNISTRUT FRAMING
- D. PRE-FABRICATED METAL STAIRS, MID-LANDINGS, AND PLATFORMS.
- E. ARCHITECTURAL METALS, ACCESS LADER, AND FALL ARREST SYSTEM.
- F. PRE-FABRICATED HANDRAILS AND ANCHORAGE.
- G. ELEVATOR GUIDERAIL SUPPORT FRAMING AND ANCHORAGE.
- H. LOVERED SCREENWALL AND ANCHORAGE.
- J. INTERIOR AND EXTERIOR COLD-FORMED METAL FRAMING.
- K. ARCHITECTURAL SUSPENDED CEILING SYSTEM.
- L. CURTAIN WALL SYSTEM.
- M. STRUCTURED CEILING GRID SYSTEM.
- N. FIRE SPRINKLERS SUPPORT SYSTEM AND ANCHORAGE.
- P. INSULATED PRECAST CONCRETE NON LOAD- BEARING PANELS AND CONNECTIONS.
- Q. PRE-FABRICATED GUARD BOOTH

DD3 INTERIOR MEP EQUIPMENT REQUIRES POSITIVE ATTACHMENT PER APLLICABLE BUILDING CODE OR AS REQUIRED BY MANUFACTURER (MOST STRINGENT SHALL GOVERN).

DD4 REFER TO MEP SPECIFICATION FOR ADDITIONAL INFORMATION.

DD5 WHEN SUBMITTING, INDICATE LOCATION, DIRECTION, AND MAGNITUDE OF LOADS WHICH ARE APPLIED TO PRIMARY STRUCTURE. CONNECTIONS TO PRIMARY STRUCTURE SHALL NOT IMPOSE TORSION TO THE SUPPORTING BEAM/JOISTS.

DD6 ALL POST-INSTALLED ANCHORS TO PRIMARY STRUCTURAL FRAMING SHALL HAVE ICC-ESR APPROVALS SUBMITTED TO THESTRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.

6 DELEGATED DESIGN
12" = 1'-0"

Structural plan review is limited to a general survey for code compliance. No review is implied nor was undertaken to verify structural adequacy.

Code violations that are found during inspection are required to be corrected. Permit issuance does not grant approval of a code violation.

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY. (A)

SYMBOLS LEGEND

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

TOS EL 115'-0"	LEVEL ELEVATION
EL 115'-0"	SPOT ELEVATION
	VERTICAL BRACE
	LOAD BEARING CMU WALL AND/OR SHEARWALL
	LOAD BEARING CONC WALL AND/OR SHEARWALL
	MATCHLINE
4 / S3.10 — DETAIL / SECTION DESIGNATION	DETAIL / SECTION REFERENCE CUT
4 / S3.10	PARTIAL PLAN OR PLAN DETAIL REFERENCE
1 / S-201	VERTICAL BRACE, SHEARWALL OR TRUSS ELEVATION REFERENCE
	SLOPE IN STRUCTURE, DROP IN STRUCTURE, RIDGES AND VALLEYS
RTU-1 — UNIT DESIGNATION PER MEP DRAWINGS REF SCHED FOR FULL OPERATING WEIGHT	MEP EQUIPMENT
*FACTORED TOTAL LOAD; JOIST DEPTH; DDCJ XXX / YYY / ZZZ; *FACTORED POST COMPOSITE LIVE LOAD; *FACTORED POST COMPOSITE DEAD LOAD. NOTES: 1. (*) DENOTES ALL JOIST GIRDER POINT LOADS ARE SERVICE LEVEL. 2. WHERE APPLICABLE, REFER TO DESIGN LOAD & DESIGN DATA TABLE FOR ULTIMATE ROOF WIND LOAD PRESSURES FOR COMPOSITE JOIST DESIGN.	COMPOSITE JOIST (CJ) KEY
STEEL COLUMN; STEEL BEAM; DENOTES MOMENT CONNECTION (REF TYPICAL STEEL DETAILS FOR MOMENT CONN REQUIREMENTS)	MOMENT CONNECTION
HSS8x8x1/4 BP; SPECIAL BASE PL TYPE; F1; 97'-0"; FOOTING TYPE; TOP OF FOOTING ELEVATION	COLUMN, BASE PLATE AND FOUNDATION MARK
W10X33 BP; SPECIAL BASE PL TYPE; P1; 97'-0"; PIER TYPE; TOP OF PIER ELEVATION	
NUMBER OF HEADED SHEAR CONNECTORS (IF REQD); STL BEAM SIZE; VERT BEAM REACTION IN KIPS (LRFD); BEAM CAMBER (INCHES) (IF REQD); HORIZONTAL COLLECTOR / DRAG FORCE IN KIPS ON BOTH SIDES OF COLUMN (LRFD) (IF REQD); 15K W14x22 (20) C=3/4" 15K [-6"] H=15K; TO BEAM IF DIFFERENT FROM REFERENCE EL; COLLECTOR / DRAG CONNECTION (IF REQD)	STEEL BEAM KEY

2 SYMBOLS LEGEND
1/8" = 1'-0"

EQUIPMENT WEIGHT SCHEDULE

DESIGNATION	EQUIPMENT TYPE	FULLY LOADED OPERATING WEIGHT (LB)
AHU	AIR HANDLING UNIT	50,000
GEN	GENERATOR	110,000
XFMR	TRANSFORMER	20,000
UPS	UPS PODS	145,500
VS	VISTA SWITCHES	10,000
SG MAU	SWITCHGEAR	2,100
MAU	MAKEUP AIR UNIT	4,900
RU-RTXX	ROOF TO UNIT	10,000 MAX
CF	CHILLER FILTER	3,100
CHILLER	CHILLER UNIT	55,000
THST	THERMAL STORAGE TANK	60,000 MAX

5 EQUIPMENT WEIGHT SCHEDULE
12" = 1'-0"

SHEET LIST

SHEET NUMBER	SHEET NAME
S0-00	GENERAL NOTES
S0-01	GENERAL NOTES
S0-02	GENERAL NOTES
S0-03	LOADING DIAGRAM
S0-04	LOADING DIAGRAM
S0-05	LOADING DIAGRAM
S0-06	LOADING DIAGRAM
S0-07	SNOW LOAD DIAGRAM
S0-08	SNOW LOAD DIAGRAM
S0-09	OVERALL 3D VIEW
S0-10	DEFLECTION DIAGRAM
S0-11	ROOF WIND UPLIFT DIAGRAM
S0-12	GENERAL NOTES
S1-10	OVERALL FOUNDATION PLAN
S1-10A	FOUNDATION PLAN - SEGMENT A
S1-10B	FOUNDATION PLAN - SEGMENT B
S1-10C	FOUNDATION PLAN - SEGMENT C
S1-10D	FOUNDATION PLAN - SEGMENT D
S1-10E	FOUNDATION PLAN - SEGMENT E
S1-10F	FOUNDATION PLAN - SEGMENT F
S1-10G	FOUNDATION PLAN - SEGMENT G
S1-10H	FOUNDATION PLAN - SEGMENT H
S1-10J	FOUNDATION PLAN - SEGMENT J
S1-10K	FOUNDATION PLAN - SEGMENT K
S1-10L	FOUNDATION PLAN - SEGMENT L
S1-10M	FOUNDATION PLAN - SEGMENT M
S1-10N	FOUNDATION PLAN - SEGMENT N
S1-10P	FOUNDATION PLAN - SEGMENT P
S1-10Q	FOUNDATION PLAN - SEGMENT Q
S1-10R	FOUNDATION PLAN - SEGMENT R
S1-10S	FOUNDATION PLAN - SEGMENT S
S1-10T	FOUNDATION PLAN - SEGMENT T
S1-11	PARTIAL FOUNDATION PLANS
S1-20	OVERALL SECOND FLOOR FRAMING PLAN
S1-20A	SECOND FLOOR FRAMING PLAN - SEGMENT A
S1-20B	SECOND FLOOR FRAMING PLAN - SEGMENT B
S1-20C	SECOND FLOOR FRAMING PLAN - SEGMENT C
S1-20D	SECOND FLOOR FRAMING PLAN - SEGMENT D
S1-20F	SECOND FLOOR FRAMING PLAN - SEGMENT F
S1-20Fb	LOWER SECOND FLOOR FRAMING PLAN - SEGMENT F
S1-20H	SECOND FLOOR FRAMING PLAN - SEGMENT H
S1-20Hb	LOWER SECOND FLOOR FRAMING PLAN - SEGMENT H
S1-20K	SECOND FLOOR FRAMING PLAN - SEGMENT K
S1-20Kb	LOWER SECOND FLOOR FRAMING PLAN - SEGMENT K
S1-20M	SECOND FLOOR FRAMING PLAN - SEGMENT M
S1-20Mb	LOWER SECOND FLOOR FRAMING PLAN - SEGMENT M
S1-20P	SECOND FLOOR FRAMING PLAN - SEGMENT P
S1-20Pb	LOWER SECOND FLOOR FRAMING PLAN - SEGMENT P
S1-20R	SECOND FLOOR FRAMING PLAN - SEGMENT R
S1-20Rb	LOWER SECOND FLOOR FRAMING PLAN - SEGMENT R
S1-20S	SECOND FLOOR FRAMING PLAN - SEGMENT S
S1-20T	SECOND FLOOR FRAMING PLAN - SEGMENT T
S1-21	SECOND FLOOR CHORD REINFORCING DIAGRAM
S1-30	OVERALL ROOF FRAMING PLAN
S1-30A	ROOF FRAMING PLAN - SEGMENT A
S1-30B	ROOF FRAMING PLAN - SEGMENT B
S1-30C	ROOF FRAMING PLAN - SEGMENT C
S1-30D	ROOF FRAMING PLAN - SEGMENT D
S1-30F	ROOF FRAMING PLAN - SEGMENT F
S1-30H	ROOF FRAMING PLAN - SEGMENT H
S1-30K	ROOF FRAMING PLAN - SEGMENT K
S1-30M	ROOF FRAMING PLAN - SEGMENT M
S1-30P	ROOF FRAMING PLAN - SEGMENT P
S1-30R	ROOF FRAMING PLAN - SEGMENT R
S1-30S	ROOF FRAMING PLAN - SEGMENT S
S1-30T	ROOF FRAMING PLAN - SEGMENT T
S1-31	ROOF CHORD REINFORCING DIAGRAM
S1-40	OVERALL PLATFORM FRAMING PLAN
S1-40F	PLATFORM FRAMING - SEGMENT F
S1-40H	PLATFORM FRAMING - SEGMENT H
S1-40K	PLATFORM FRAMING - SEGMENT K
S1-40M	PLATFORM FRAMING - SEGMENT M
S1-40P	PLATFORM FRAMING - SEGMENT P
S1-40R	PLATFORM FRAMING - SEGMENT R
S1-40T	PENTHOUSE ROOF FRAMING PLAN - SEGMENT T
S1-50	OVERALL PLATFORM GIRT FRAMING PLAN
S1-50F	PLATFORM GIRT FRAMING PLAN - SEGMENT F
S1-50H	PLATFORM GIRT FRAMING PLAN - SEGMENT H
S1-50K	PLATFORM GIRT FRAMING PLAN - SEGMENT K
S1-50M	PLATFORM GIRT FRAMING PLAN - SEGMENT M
S1-50P	PLATFORM GIRT FRAMING PLAN - SEGMENT P
S1-50R	PLATFORM GIRT FRAMING PLAN - SEGMENT R
S2-00	FRAME ELEVATIONS
S2-01	FRAME ELEVATIONS
S2-02	FRAME ELEVATIONS
S2-03	FRAME ELEVATIONS
S2-04	FRAME ELEVATIONS
S2-05	FRAME ELEVATIONS
S2-06	FRAME ELEVATIONS
S2-07	FRAME ELEVATIONS
S2-08	FRAME ELEVATIONS
S2-09	FRAME ELEVATIONS
S2-10	FRAME ELEVATIONS
S2-11	FRAME ELEVATIONS
S2-12	FRAME ELEVATIONS
S2-13	FRAME ELEVATIONS
S2-20	EXT STAIR FRAMING ELEVATIONS
S2-30	TYPICAL BRACING DETAILS
S2-31	TYPICAL BRACING DETAILS
S3-00	PIER, GRADE BEAM SCHEDULE AND TYPICAL FOUNDATION DETAILS
S3-01	TYPICAL FOUNDATION DETAILS
S3-10	FOUNDATION DETAILS
S3-11	FOUNDATION DETAILS
S3-12	FOUNDATION DETAILS
S3-20	FOUNDATION DETAILS
S3-21	FOUNDATION DETAILS
S5-00	TYPICAL STEEL COLUMN DETAILS AND BASE PLATE SCHEDULE
S5-01	TYPICAL CONNECTION DETAILS
S5-02	TYPICAL STEEL FRAMING SECTIONS AND DETAILS
S5-03	TYPICAL STEEL FRAMING SECTIONS AND DETAILS
S5-04	TYPICAL STEEL FRAMING SECTIONS AND DETAILS
S5-05	STANDARD SHEAR PLATE CONNECTIONS
S5-06	DOUBLE ANGLE BOLTED-BOLTED CONNECTION
S5-07	DOUBLE ANGLE BOLTED-WELDED CONNECTION
S5-08	DOUBLE ANGLE WELDED-BOLTED CONNECTIONS
S5-09	EXTENDED SHEAR PLATE
S5-10	STEEL FLOOR FRAMING DETAILS
S5-11	STEEL FLOOR FRAMING DETAILS
S5-12	STEEL FLOOR FRAMING DETAILS
S5-13	STEEL FLOOR FRAMING DETAILS
S5-14	STEEL FLOOR FRAMING DETAILS
S5-20	STEEL ROOF FRAMING SECTIONS AND DETAILS
S5-21	STEEL ROOF FRAMING SECTIONS AND DETAILS
S5-22	STEEL ROOF FRAMING SECTIONS AND DETAILS
S5-23	STEEL ROOF FRAMING SECTIONS AND DETAILS
S5-24	STEEL ROOF FRAMING SECTIONS AND DETAILS
S5-30	PLATFORM FRAMING SECTION AND DETAILS
S5-31	PLATFORM FRAMING SECTION AND DETAILS

1 SHEET LIST
12" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
TLD DESIGN CONSULTING LLC

COLORADO LICENSED
NICHOLAS W. RIVIO
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7

GENERAL:

G1 STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE PROTECTED BY U.S.A. COPYRIGHT LAWS. DO NOT USE THESE STRUCTURAL DRAWINGS FOR ANY PURPOSE OTHER THAN CONSTRUCTION OF STRUCTURE FOR BUILDING DESCRIBED IN ARCHITECTURAL DRAWINGS AT LOCATION INDICATED.

G2 UNLESS NOTED OTHERWISE, GENERAL NOTES AND DETAILS IN DRAWINGS DESIGNATED AS "TYPICAL DETAILS," ARE APPLICABLE TO CONSTRUCTION IN ALL LOCATIONS WHERE CONDITIONS ARE SIMILAR.

G3 NOTES PROVIDED ON STRUCTURAL DRAWINGS DO NOT REPLACE CONTRACT SPECIFICATIONS. IN EVENT OF A CONFLICT BETWEEN THE TWO, THE MORE STRINGENT INTERPRETATION BASED ON SOLE DISCRETION OF STRUCTURAL ENGINEER SHALL APPLY.

G4 CONSTRUCTION SHALL CONFORM TO PROVISIONS OF SPECIFIED BUILDING CODE AND ALL STANDARDS REFERENCED THEREIN IN THEIR ENTIRETY, WITH ALL LOCALLY ADOPTED AMENDMENTS, REFERENCED THEREIN. IN EVENT OF A CONFLICT BETWEEN BUILDING CODE AND STRUCTURAL DRAWINGS AND SPECIFICATIONS, THE MORE STRINGENT INTERPRETATION BASED ON SOLE DISCRETION OF STRUCTURAL ENGINEER SHALL APPLY.

G5 COMPLY WITH ALL APPLICABLE BUILDING CODES, REGULATION AND SAFETY REQUIREMENTS. CONTRACTOR IS SOLELY RESPONSIBLE FOR SAFETY ON JOBSITE DURING CONSTRUCTION.

G6 UNLESS ACCOMPANIED BY A FORMAL CHANGE ORDER, RESPONSES TO QUESTIONS AND RFI'S, COMMENTS MADE DURING REVIEW OF SUBMITTALS, AND DIRECTIVES PROVIDED IN ANY FORM, BY STRUCTURAL ENGINEER TO CONTRACTOR DURING CONSTRUCTION PROCESS ARE INTENDED TO BE CLARIFICATIONS OF CONTRACT DOCUMENTS OR CORRECTIONS TO PERCEIVED INTERPRETATION OF INTENT OF CONTRACT DOCUMENTS BY CONTRACTOR. SUCH CLARIFICATIONS AND CORRECTIONS ARE NOT INTENDED TO REPRESENT A CHANGE IN COST OF PROJECT TO OWNER AND ARE CONSIDERED TO BE INFERABLE FROM CONTENT OF CONTRACT DRAWINGS OR CONSISTENT WITH INDUSTRY STANDARDS OF CONSTRUCTION. IF CONTRACTOR DETERMINES THAT SUCH CLARIFICATIONS AND CORRECTIONS HAVE AN IMPACT ON COST OF PROJECT TO OWNER, SUBMIT A CHANGE ORDER REQUEST WITH DETAILED PRICING INFORMATION TO ARCHITECT BEFORE PURCHASING, DETAILING, FABRICATING OR INSTALLING ANY COMPONENT RELATED TO SUCH CLARIFICATIONS AND CORRECTIONS.

G7 ROOF STRUCTURE AND ITS SUPPORTING ELEMENTS HAVE BEEN DESIGNED WITH THE ASSUMPTION THAT ADEQUATE AND SUFFICIENT DRAINAGE HAS BEEN PROVIDED TO PREVENT PONDING OF WATER.

G8 INFORM ARCHITECT IN WRITING OF ANY DISCREPANCIES OR OMISSIONS NOTED ON DRAWINGS OR IN SPECIFICATIONS. UPON RECEIPT OF SUCH INFORMATION, ARCHITECT WILL SEND WRITTEN CLARIFICATIONS TO ALL CONCERNED.

G9 DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, AND FORMWORK, AS REQUIRED FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION OF BUILDING, WHICH IS SOLE RESPONSIBILITY OF CONTRACTOR.

G10 CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES.

G11 STRUCTURAL DRAWINGS ARE NOT TO BE VIEWED AS STAND ALONE DRAWINGS WITH RESPECT TO PROJECT DIMENSIONS OR ANY OTHER COMPONENT OF CONSTRUCTION THAT CAN AND MAY BE IDENTIFIED IN OTHER PARTS OF CONTRACT DOCUMENTS. REDUNDANT DIMENSIONS AND OTHER INFORMATION INDICATED ELSEWHERE IN CONTRACT DOCUMENTS MAY OR MAY NOT BE PROVIDED IN STRUCTURAL DRAWINGS. THE ENTIRE SET OF CONTRACT DOCUMENTS IS REQUIRED TO PROPERLY CONSTRUCT THE STRUCTURE AS WELL AS OTHER COMPONENTS OF BUILDING. ASSEMBLE AND COORDINATE REQUIREMENTS OF ALL COMPONENTS OF CONTRACT DOCUMENTS IN ORDER TO PROPERLY IMPLEMENT REQUIREMENTS OF CONTRACT. IF THERE IS SPECIFIC DIMENSIONAL OR OTHER INFORMATION THAT CONTRACTOR FEELS CAN NOT BE DETERMINED FROM CONTRACT DOCUMENTS AS A WHOLE, THEN CONTRACTOR SHALL BRING IT TO ATTENTION OF ARCHITECT.

G12 STRUCTURAL DRAWINGS ARE NOT TO BE VIEWED AS DETAILED SHOP OR ERECTION DRAWINGS. DETERMINE VARIOUS ANCILLARY DIMENSIONS REQUIRED FOR PROPER FIT-UP OF VARIOUS COMPONENTS OF STRUCTURE FROM INFORMATION PROVIDED IN CONTRACT DOCUMENTS. IT IS CONTRACTOR'S RESPONSIBILITY TO ESTABLISH WHERE SUCH ANCILLARY DIMENSIONS ARE NECESSARY AND TO CALCULATE AND VERIFY THESE DIMENSIONS AS REQUIRED TO ACHIEVE PROPER FIT-UP OF MATERIALS AND TO ACHIEVE COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS AND CONSTRUCTION TOLERANCES.

G13 ANCHORS REQUIRED FOR ANCHORING MEP EQUIPMENT AND/OR PIPING ARE NOT INDICATED ON STRUCTURAL DRAWINGS. DETERMINE AND COORDINATE REQUIREMENTS FROM OTHER DISCIPLINES AND PROVIDE APPROPRIATE ALLOWANCES INTO BID. REFERENCE ARCHITECTURAL, ELECTRICAL, AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF PIPES, VENTS, CHASES, DUCTS AND OTHER OPENINGS AND DETAILS NOT INDICATED ON STRUCTURAL DRAWINGS. CHECK AND VERIFY DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

G14 DRAWINGS AND SPECIFICATIONS REPRESENT FINISHED STRUCTURE. UNLESS SO STATED OR NOTED, THEY DO NOT INDICATE COMPONENTS THAT ARE NECESSARY FOR SUPPORTING AND STABILIZING WORK DURING CONSTRUCTION OR MEANS AND METHODS OF CONSTRUCTION, ALL OF WHICH ARE RESPONSIBILITY OF CONTRACTOR. TEMPORARY BRACING, PROPERLY DESIGNED UNDER SUPERVISION OF A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN STATE IN WHICH PROJECT IS LOCATED, SHALL BE PROVIDED AS REQUIRED TO HOLD ALL COMPONENTS OF STRUCTURE IN PLACE UNTIL FINISHED STRUCTURE IS COMPLETE.

G15 INSPECTIONS, SPECIAL OR OTHERWISE, THAT ARE REQUIRED BY BUILDING CODES, LOCAL BUILDING DEPARTMENTS, OR CONTRACT DOCUMENTS, SHALL BE PROVIDED BY AN INDEPENDENT INSPECTION COMPANY.

G16 VARIOUS OPENINGS, DEPRESSIONS, RECESSES, SLOPES, BLOCKOUTS, CURBS, AND EMBEDMENTS NOT INDICATED IN STRUCTURAL DOCUMENTS MAY BE REQUIRED IN COMPLETED STRUCTURE BY OTHER DISCIPLINE CONTRACT DOCUMENTS. INCORPORATE AND COORDINATE SUCH ITEMS AND BRING CONSTRUCTION OR INSTALLATION ISSUE TO ATTENTION OF ARCHITECT. COORDINATE ATTACHMENT AND EMBEDMENTS REQUIRED BY MANUFACTURERS AND INSTALLERS OF VARIOUS EQUIPMENT AND FINISHES. BRING CONSTRUCTION OR INSTALLATION ISSUES TO ATTENTION OF ARCHITECT PRIOR TO FABRICATION OF MATERIALS OR INSTALLATION. INCLUDE AN ALLOWANCE IN BID PRICE SUFFICIENT TO ADEQUATELY COVER STRUCTURAL REQUIREMENTS FOR SUCH ITEMS WITHOUT NEED FOR A FUTURE CHANGE TO BID PRICE.

G17 ELEVATIONS PROVIDED IN STRUCTURAL DRAWINGS ARE RELATIVE ELEVATIONS AND ARE NOT INTENDED TO ESTABLISH ACTUAL SEA LEVEL ELEVATION OF STRUCTURE. REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR ACTUAL SEA LEVEL ELEVATIONS OF VARIOUS ELEMENTS OF BUILDING.

G18 LOCATIONS AND LOADINGS FOR MECHANICAL EQUIPMENT ARE BASED ON UNITS INDICATED ON MECHANICAL DRAWINGS AND IN EQUIPMENT SCHEDULE. REPORT CHANGES IN LOCATION, TYPE, SIZE, WEIGHT, OR NUMBER OF PIECES OF EQUIPMENT TO ARCHITECT FOR VERIFICATION OF ADEQUACY OF SUPPORTING MEMBERS PRIOR TO PLACEMENT OF SUCH EQUIPMENT.

G19 SUBMIT, IN WRITING WITH REQUIRED BACKUP DATA, PROPOSED SUBSTITUTIONS OF MATERIALS, PRODUCTS OR DETAILS DEPICTED IN CONTRACT DOCUMENTS. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "NOTICE IN WRITING." PROPOSED SUBSTITUTIONS OR DEVIATIONS WILL NOT BE REVIEWED OR RESPONDED TO WITHOUT REQUIRED BACKUP DATA. CONTRACTOR SHALL REMAIN LIABLE FOR ANY SUBSTITUTION OR DEVIATION UNLESS ACKNOWLEDGMENT AND ACCEPTANCE IS PROVIDED BY ARCHITECT IN WRITING.

G20 COORDINATE GRADES WITH CIVIL ENGINEER'S GRADING PLAN AND LANDSCAPE ARCHITECT'S PLAN. NOTIFY ARCHITECT OF ANY DISCREPANCIES.

G21 DO NOT SCALE DRAWINGS IN STRUCTURAL DOCUMENTS FOR ANY PURPOSE, INCLUDING DETERMINATION OF QUANTITIES AND FIT UP OF MATERIALS.

G22 OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA AND SUBMIT SUCH DOCUMENTS TO SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF STRUCTURAL MEMBERS, AND CONSTRUCTION.

G23 UNLESS SPECIFICALLY INDICATED ON STRUCTURAL DRAWINGS, LOCATION OF EXISTING SUBGRADE UTILITIES OR OTHER OBSTRUCTIONS IS UNKNOWN. NOTIFY ARCHITECT OF CONFLICT OF DISCOVERED ITEMS WITH CONSTRUCTION OF NEW FOUNDATION ELEMENTS.

G24 UNLESS NOTED OTHERWISE, NO PROVISION HAS BEEN MADE IN STRUCTURAL DESIGN FOR FUTURE VERTICAL EXPANSION.

SHOP DRAWINGS:

SDR1 REVIEW ALL SUBMITTALS FOR COMPLIANCE WITH CONTRACT DOCUMENTS AND CERTIFY REVIEW BY A STAMP NOTING THAT SUBMITTAL HAS BEEN "APPROVED" AND WHICH BEARS SIGNATURE OF AN AUTHORIZED REPRESENTATIVE OF CONTRACTOR AND DATE. SUBMITTALS WHICH DO NOT REFLECT CONTRACTOR'S APPROVAL, SIGNATURE AND DATE; OR DO NOT APPEAR TO HAVE BEEN REVIEWED BY CONTRACTOR, WILL BE RETURNED WITHOUT REVIEW.

SDR2 REVIEW OF SUBMITTALS BY STRUCTURAL ENGINEER IS ONLY FOR GENERAL CONFORMANCE WITH DESIGN CONCEPT AND INFORMATION GIVEN IN STRUCTURAL CONTRACT DOCUMENTS. CORRECTIONS OR COMMENTS MADE ON SUBMITTALS DURING THIS REVIEW DO NOT RELIEVE CONTRACTOR FROM COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS. APPROVAL OF A SPECIFIC ITEM DOES NOT IMPLY OR INCLUDE APPROVAL OF AN ASSEMBLY OF WHICH ITEM IS A COMPONENT. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS TO BE CONFIRMED AND CORRELATED AT JOBSITE; INFORMATION THAT PERTAINS SOLELY TO FABRICATION PROCESSES OR TO MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION; COORDINATION OF WORK WITH THAT OF OTHER TRADES AND PERFORMING WORK IN A SAFE AND SATISFACTORY MANNER.

SDR3 DO NOT USE REPRODUCTIONS OR ELECTRONIC FILES OF CONTRACT DOCUMENTS FOR PREPARATION OF SHOP DRAWINGS.

SDR4 OMISSION FROM SHOP DRAWINGS OF ANY REQUIREMENTS OF CONTRACT DOCUMENTS SHALL NOT RELIEVE CONTRACTOR OF RESPONSIBILITY OF COMPLYING WITH OMITTED REQUIREMENTS, EVEN IF SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED.

SDR5 ARCHITECT/ENGINEER SHALL NOT BE RESPONSIBLE FOR DELAYS CAUSED BY REJECTION OF INADEQUATE OR INCORRECT SHOP DRAWINGS.

SDR6 SHOP DRAWINGS THAT ARE NOT SPECIFICALLY REQUIRED BY CONTRACT DOCUMENTS WILL NOT BE REVIEWED OR RETURNED.

SDR7 SIGNED AND SEALED CALCULATIONS FOR A PARTICULAR SYSTEM SUBMITTED ARE REVIEWED FOR LOAD CRITERIA AND GENERAL CONFORMANCE WITH CONTRACT DOCUMENTS. CALCULATION REVIEW AND COMMENTS DO NOT INFER A DETAILED CHECK OF CALCULATIONS, NOR DOES REVIEW RELIEVE SUBMITTING STRUCTURAL ENGINEER OR CONTRACTOR OF RESPONSIBILITY.

GEOTECHNICAL REPORT:

GR1 FOUNDATION DESIGN AND SUBSURFACE INFORMATION IS BASED ON GEOTECHNICAL INVESTIGATION REPORT AS FOLLOWS:

GEOTECHNICAL CONSULTANT:	TERRACON CONSULTANTS, INC.
GEOTECHNICAL REPORT NUMBER:	25245150
GEOTECHNICAL REPORT NAME:	QTS DATA CENTER - DC2 AND DC3 GEOTECHNICAL REPORT
GEOTECHNICAL REPORT LOCATION:	1120 NORTH GUN CLUB ROAD, AURORA, COLORADO
GEOTECHNICAL REPORT DATE:	OCTOBER 4, 2024

GR2 GEOTECHNICAL REPORT IS AVAILABLE TO CONTRACTOR AS A REFERENCE DOCUMENT UPON REQUEST TO OWNER.

GR3 ARCHITECT AND ENGINEER ARE NOT RESPONSIBLE OR LIABLE FOR ACCURACY OR APPLICABILITY OF INFORMATION PRESENTED IN GEOTECHNICAL REPORT.

SUBGRADE PREPARATION:

SP1 EXAMINE SITE THOROUGHLY AND CONTENTS OF GEOTECHNICAL REPORT AND ITS RECOMMENDATIONS. PERFORM ALL SUBGRADE PREPARATION, FILL, FILL PLACEMENT, AND FOUNDATION CONSTRUCTION IN STRICT COMPLIANCE WITH CONTRACT DOCUMENTS AND GEOTECHNICAL REPORT, AND DO NOT PROCEED WITH FOUNDATION CONSTRUCTION UNTIL SUBGRADE HAS BEEN OBSERVED, TESTED, AND APPROVED BY PROJECT'S GEOTECHNICAL-ENGINEER-OF-RECORD.

CONCRETE SLAB-ON-GRADE:

SOG1 INSTALL VAPOR RETARDER WHERE INDICATED ON DRAWINGS IN ACCORDANCE WITH SPECIFICATIONS AND MANUFACTURER'S RECOMMENDATIONS.

SOG2 REMOVE ANY STANDING WATER ON SURFACE OF VAPOR RETARDER PRIOR TO CONCRETE PLACEMENT.

SOG3 REFERENCE PLANS FOR SLAB REINFORCING. PROVIDE CLASS B TENSION LAP SPLICE FOR SLAB REINFORCING. PROVIDE ADDITIONAL #4X5'-0" AT RE-ENTRANT CORNERS, INCLUDING RE-ENTRANT CORNERS AROUND PERIMETER OF BUILDINGS, FLOOR RECESSES, AND OPENINGS.

SOG4 PROVIDE CONTROL JOINTS ALONG COLUMN LINES AND AT 15'-0" ON CENTER, EACH WAY, MAXIMUM, UNLESS NOTED OTHERWISE. PROVIDE ADDITIONAL CONTROL JOINTS AT RE-ENTRANT CORNERS.

SOG5 DO NOT PLACE HEAVY EQUIPMENT OR CONSTRUCTION STORAGE LOADS ON SLAB UNLESS CALCULATIONS, PREPARED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN STATE IN WHICH PROJECT IS LOCATED, HAVE BEEN SUBMITTED VERIFYING ADEQUACY OF SLAB.

DRILLED CONCRETE PIERS:

P1 STRAIGHT SHAFT (SS) DRILLED PIER DESIGNS ARE BASED UPON INFORMATION LISTED BELOW.

ALLOWABLE END BEARING PRESSURE:	25,000 PSF
ALLOWABLE COMPRESSION SKIN FRICTION RESISTANCE:	2,000 PSF
UPLIFT FROM SOIL	85 x PIER DIA
ALLOWABLE UPLIFT SKIN FRICTION RESISTANCE:	2,000 PSF
MINIMUM PIER CLEAR SPACING FOR FULL CAPACITY:	3 SHAFT DIA. OF THE LARGER PIER
BEARING STRATUM:	CLAYSTONE BEDROCK, OR AS APPROVED BY GER REPRESENTATIVE
BEARING STRATUM ELEVATION (FOR ESTIMATING PURPOSE ONLY):	20 FT

P2 CONSTRUCT PIERS AND PLACE CONCRETE IN PIERS IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT AND AS PER THE CURRENT VERSION OF ACI 336. FILL PIER EXCAVATIONS WITH CONCRETE AS SOON AS PRACTICAL BEFORE END OF DAY, BUT NO LONGER THAN 8 HOURS AFTER START OF DRILLING IN PENETRATION OR BEARING ZONE. REFER TO GEOTECHNICAL REPORT FOR CASING REQUIREMENTS.

P3 PLACE PIERS NOT SPECIFICALLY LOCATED ON DRAWINGS ON CENTERLINE OF COLUMN ABOVE. WHERE NO COLUMN OCCURS, LOCATE PIER ON CENTERLINE OF WALL OR BEAM.

P4 HOLD REINFORCING CAGE SECURELY AWAY FROM EARTH AT SIDE AND BOTTOM BY SETS OF 3 SPACERS AT A MAXIMUM SPACING OF 8 FEET ALONG LENGTH OF CAGE AND 12-INCHES FROM BOTTOM.

P5 VERIFY DEPTHS OF PIERS BEFORE PIER STEEL IS CUT. PIER STEEL MAY BE DELIVERED TO JOBSITE IN STANDARD LENGTHS AND CUT AS REQUIRED.

P6 BELLING AT TOP OF PIERS IS PROHIBITED. FORM TOP OF PIER IF REQUIRED TO MAINTAIN SPECIFIED DIAMETER. REMOVE ANY CONCRETE EXTENDING BEYOND SPECIFIED DIAMETER.

P7 PROVIDE CASING, IF UTILIZED ON THIS PROJECT, WITH AN INSIDE DIAMETER EQUAL TO PIER DIAMETER INDICATED IN PIER SCHEDULE.

P8 WHERE CLEAR DISTANCE BETWEEN TWO PIERS IS LESS THAN PIER DIAMETER OF LARGER PIER, WAIT AT LEAST 48 HOURS AFTER CASTING FIRST PIER BEFORE DRILLING SECOND PIER.

P9 USE TEMPORARY TEMPLATES TO SET DOWELS AND ANCHOR BOLTS IN PIERS. PLACE DOWEL AND ANCHOR BOLTS PRIOR TO PIER CONCRETE ACHIEVING ITS INITIAL SET.

P10 FOR ESTIMATING PURPOSES ONLY, 20' IS DISTANCE FROM EXISTING GRADE TO REQUIRED BEARING STRATUM INDICATED IN TYPICAL PIER DETAIL.

P11 LOCATION TOLERANCE AT CUT-OFF SHALL BE NO GREATER THAN 1/24 OF SPECIFIED SHAFT DIAMETER OR 3 IN., WHICHEVER IS GREATER. OUT-OF-PLUMBNESS OF PIERS SHALL NOT EXCEED 1.5%.

P12 NOTIFY ARCHITECT/ENGINEER 48 HOURS PRIOR TO DRILLED PIER PLACEMENT.

MASS CONCRETE:

MC1 CONSTRUCT ELEMENTS OF CONCRETE THAT HAVE A LEAST DIMENSION OF GREATER THAN 59.9 INCHES, OR ARE SPECIFICALLY NOTED ON DRAWINGS PER ACI 207.1. MASS CONCRETE REQUIREMENTS DO NOT APPLY TO PORTIONS OF DRILLED PIERS BELOW GRADE. PROVIDE MASS CONCRETE COMPLYING WITH FOLLOWING REQUIREMENTS:
- A. MAXIMUM INTERNAL TEMPERATURE OF PLACED CONCRETE SHALL NOT EXCEED 160 DEGREES F.
- B. MAXIMUM TEMPERATURE DIFFERENCE BETWEEN CENTER AND SURFACE MASS OF CONCRETE PLACEMENTS SHALL NOT EXCEED 35 DEGREES. LOCATIONS OF SENSORS FOR MEASUREMENT OF MAXIMUM AND MINIMUM TEMPERATURES IN PLACEMENT SHALL BE STATED IN THERMAL CONTROL PLAN. PROVIDE A WRITTEN THERMAL CONTROL PLAN THAT ADDRESSES NEED FOR ITEMS SUCH AS INSULATING BLANKETS, COOLED MATERIALS AND DEVICES FOR INTERNAL CONCRETE CORE TEMPERATURE MEASUREMENT.
- C. USE OF 56 DAY BREAKS IN LIEU OF 28 DAY BREAKS FOR DETERMINATION OF STRENGTHS IS PERMITTED.

CONCRETE MIX:

CM1 PROVIDE CONCRETE MEETING FOLLOWING:

CONCRETE MIX DESIGN SCHEDULE

USE	28-DAY STRENGTH (PSI)	MAX. WATER CEMENT RATIO	MAX. CURED DENSITY (PCF)	MAX. AGGREGATE SIZE (IN)	CEMENT TYPE (REF CM13)	MAX. ALLOWABLE % FLY ASH (REF CM5)	% AIR ENTRAINMENT	EXPOSURE CATEGORY
DRILLED PIERS	4,000	0.55	150	1	I / II	40	-	F0,S0
PEDESTALS, PILASTERS	4,500	0.45	150	1	I / II	20	6	F1,S0
GRADE BEAMS	4,500	0.45	150	1	I / II	20	6	F1,S0
PRECAST WALLS	4,500	0.45	150	1	I / II	30	6	F1,S0
NORMAL WEIGHT SLAB ON COMPOSITE METAL DECK (REF NOTE CM8)	4,000	0.45	150	1	I / II	20	-	F0,S0
SLAB FOR EQUIP PADS AND EXTERIOR SLAB-ON-GRADE	4,500	0.45	150	1	I / II	20	6	F1,S0
SLAB-ON-GRADE INTERIOR	4,000	0.50	150	1	I / II	20	-	F1,S0
MASS CONCRETE (ALSO REFER TO MASS CONCRETE NOTES)	4,500 AT 56 DAYS	0.40	150	1	II	40	-	F0,S0

CM2 CHANGING APPROVED MIX DESIGN REQUIRES A NEW MIX DESIGN SUBMITTAL. USE ADMIXTURES PLACED IN CONCRETE MIX IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND INCLUDE IN SUBMITTED CONCRETE MIX DESIGN FOR APPROVAL.

CM3 DO NOT PLACE CONCRETE WHEN AMBIENT AIR TEMPERATURE IS OUTSIDE LIMITS PROVIDED ON APPROVED MIX DESIGNS. USE OF CHLORIDES IN ANY ADMIXTURE IS NOT PERMITTED.

CM4 PROVIDE PORTLAND CEMENT CONFORMING TO ASTM C150, OF TYPES NOTED IN SCHEDULE. PROVIDE NORMAL WEIGHT AGGREGATE CONFORMING TO ASTM C33. PROVIDE LIGHT WEIGHT AGGREGATE CONFORMING TO ASTM C330.

CM5 DO NOT USE FLY ASH AS A CONSTITUENT IN ARCHITECTURALLY EXPOSED CONCRETE. FLY ASH IS ALLOWED IN ALL OTHER NON-ARCHITECTURALLY EXPOSED CONCRETE, UP TO MAXIMUM PERCENTAGE LISTED ABOVE. ADD WEIGHT OF FLY ASH TO WEIGHT OF CEMENT IN CALCULATION OF WATER CEMENT RATIO. CONFIRM IN MIX DESIGNSUBMITTAL THAT USE OF FLY ASH WILL NOT INTERFERE WITH PERFORMANCE OF OTHER PRODUCTS AND MATERIALS THAT WILL BE IN CONTACT WITH CONCRETE.

CM6 AIR ENTRAINMENT IS REQUIRED ONLY IN NORMAL WEIGHT CONCRETE PERMANENTLY EXPOSED TO WEATHER CONDITIONS. THE TOLERANCE FOR AIR CONTENT AS DELIVERED IS +/- 1.5%. DO NOT AIR-ENTRAIN INTERIOR FLOOR SLABS THAT RECEIVE HARD TROWEL FINISH.

CM7 MIX FOR TOPPING SLABS THAT ARE LESS THAN TWO INCHES THICK SHALL CONTAIN SIKA FIBERMESH - 150 FIBERS OR EQUIVALENT WITH A DOSAGE OF 1.5 POUNDS PER CUBIC YARD.

CM8 PROVIDE CONCRETE IN ELEVATED SLABS AND BEAMS WITH A SHRINKAGE LIMIT OF 0.02% AT 28 DAYS MEASURED IN ACCORDANCE WITH ASTM C157. SUBMIT LABORATORY TEST RESULTS TO ARCHITECT PRIOR TO CONSTRUCTION.

CM9 NON-CHLORIDE ACCELERATING ADMIXTURE MAY BE USED IN CONCRETE SLABS PLACED AT AMBIENT TEMPERATURES BELOW 50 DEGREES FAHRENHEIT AT CONTRACTOR'S OPTION.

CM11 SUBMIT MIX DESIGN SUBMITTALS TO PROJECT'S TESTING LABORATORY PRIOR TO SUBMITTING TO ARCHITECT. A SUBMITTAL WITHOUT TESTING LABORATORY'S APPROVAL WILL BE REJECTED.

CM12 PROPORTION CONCRETE MIXES WITH SLUMPS SUITABLE TO PROPERLY PLACE, CONSOLIDATE, AND FINISH CONCRETE IN ACCORDANCE WITH ACI 318 AND CONTRACT DOCUMENTS.

CM13 IF TYPE 1L CEMENT IS BEING SUBSTITUTED FOR TYPE I / II CEMENT IN THE CONCRETE MIX DESIGN FOR THE PROJECT, THE CONTRACTOR'S CONCRETE MIX SUPPLIER SHALL PROVIDE GUIDANCE FOR USE OF 1L CEMENT FOR ALL FLOOR SLABS ON THE PROJECT, INCLUDING DIRECTIONS FOR PLACEMENT, FINISHING, CURING AND POSSIBLE USE OF SURFACE DENSIFIER APPROPRIATE TO THE CHEMICAL COMPOSITION OF THE SPECIFIC TYPE 1L CEMENT TO BE UTILIZED. SUGGESTED COMPONENTS OF THE JUSTIFICATION FOR USE OF TYPE 1L WOULD INCLUDE:
1. A MOCKUP 20 FEET BY 20 FEET, WHICH IS NOT PART OF THE COMPLETED PROJECT WORK SHALL BE CAST, FINISHED, CURED, AND AS NEEDED DENSIFIED, FOR REVIEW BY THE DESIGN TEAM AND OWNER.
2. ALL FLOOR SLABS SHALL BE WET-CURED FOR A FULL 28-DAYS OR LESS IF DEMONSTRATED THAT A SHORTER WET-CURE PERIOD CAN ACHIEVE THE REQUIRED CONCRETE STRENGTH AND SLAB FINISH.
3. AS AN ALTERNATIVE TO WET-CURING, ADDITIONAL WATER MAY BE ADDED TO MIXTURE WITHIN CODE LIMITS AND DEMONSTRATED TO BE COMPATIBLE WITH CURING METHODS PROPOSED TO PREVENT FLOOR SLAB SURFACE DEFECTS.
4. APPLIED DENSIFIERS SHALL BE CONSIDERED FOR APPLICATION TO FLOOR SLABS PROVIDED DENSIFIER IS COMPATIBLE WITH TYPE 1L CEMENT AND WITHOUT THE RISK OF INTERFERING WITH FLOORING ADHESIVES AND VOIDING FLOOR FINISH WARRANTIES.

COLD-FORMED METAL FRAMING:

CF1 PROVIDE COLD-FORMED METAL FRAMING DESIGNED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN STATE IN WHICH PROJECT IS LOCATED, PER SPECIFICATIONS AND PER LOADS INDICATED IN STRUCTURAL CONTRACT DOCUMENTS AND MEETING FOLLOWING REQUIREMENTS AS A MINIMUM.
- A. MEET MINIMUM REQUIREMENTS OF AISI STANDARDS, LATEST EDITION.
- B. DESIGN IN ACCORDANCE WITH AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, LATEST EDITION.
- C. PROVIDE STRUCTURAL MEMBERS FORMED FROM CORROSION-RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A653.
- D. PROVIDE STRUCTURAL MEMBERS WITH ZINC-COATED MEETING ASTM A924.
- E. ANCHOR TOP AND BOTTOM RUNNER TRACKS OF EXTERIOR AND LOAD-BEARING INTERIOR WALLS TO STRUCTURE OVERHEAD AND FLOOR STRUCTURE BELOW BY WELDING, NAILING, OR BOLTING AS APPLICABLE.

EMBEDDED HEADED-STUD CONNECTORS & DEFORMED BAR ANCHORS (DBA):

SC1 PROVIDE HEADED-STUD CONNECTORS (SC) CONFORMING TO ASTM A108, AISI C-1015 THROUGH C-1020, HEADED STUD TYPE, COLD FINISHED CARBON STEEL; AWS D1.1, TYPE B (MINIMUM TENSILE STRENGTH 65,000 PSI). AUTOMATICALLY END WELD STUDS IN SHOP OR IN FIELD.

SC2 PROVIDE DEFORMED BAR ANCHORS (DBA) CONFORMING TO ASTM A1064-18 AND AWS D1.1, TYPE C (MINIMUM TENSILE STRENGTH 80,000PSI). AUTOMATICALLY END WELD DBA IN SHOP OR FIELD.

BLAST-RESISTANT DESIGN REPORT:

BRD 1 BLAST LOADS RESISTED BY EXTERIOR ELEMENTS OF THE BUILDING ENVELOPE ARE BASED ON BLAST-RESISTANT DESIGN REPORT AS FOLLOWS:

BLAST CONSULTANT: PROTECTION ENGINEERING CONSULTANTS
BLAST REPORT NO: 24-117
BLAST REPORT NAME: QTS DENVER DATA CENTER 2 (DEN1-DC2)
BLAST REPORT LOCATION: AURORA, COLORADO
BLAST REPORT DATE: 5/30/2025

BRD 2 BLAST-RESISTANT DESIGN REPORT IS AVAILABLE TO CONTRACTOR AS A REFERENCE DOCUMENT UPON REQUEST TO OWNER.

BRD 3 ARCHITECT AND ENGINEER ARE NOT RESPONSIBLE OR LIABLE FOR ACCURACY OR APPLICABILITY OF INFORMATION PRESENTED IN BLAST-RESISTANT DESIGN REPORT.

BRD 4 CONTRACTOR SHALL REFER TO BLAST REPORT FOR DESIGN RECOMMENDATIONS APPLICABLE TO EXTERIOR CLADDING ELEMENTS OF THIS STRUCTURE. SPECIFIC ELEMENTS WITHIN BLAST LOADING SCOPE INCLUDE EXTERIOR GLAZING, EXTERIOR COLD-FORMED METAL FRAMING, EXTERIOR PRECAST WALL PANELS, ROOF STRUCTURAL FRAME, AND STRUCTURAL COMPONENTS SUPPORTING THESE ELEMENTS, IN ADDITION TO OTHER ELEMENTS SPECIFICALLY ADDRESSED IN BLAST REPORT.

CONCRETE REINFORCING:

CR1 PROVIDE REINFORCING STEEL CONFORMING TO ASTM A615 USING NEW OR RECYCLED DOMESTIC BILLET STEEL OF A DOMESTIC MANUFACTURE. PROVIDE GRADES AS FOLLOWS:
- A. #3 THROUGH #18.....GRADE 60

CR2 PROVIDE REINFORCING STEEL SPECIFIED TO BE WELDED CONFORMING TO ASTM A706, WITH GRADES AS INDICATED ON DRAWINGS OR AS PERMITTED. WELD IN ACCORDANCE WITH AWS D1.4. WELDING OF REINFORCING STEEL FOR USE IN STRUCTURAL CONCRETE OR STRUCTURAL MASONRY IS PERMITTED ONLY WHERE SPECIFICALLY DESIGNATED ON DRAWINGS.

CR3 PROVIDE ELECTRICALLY-WELDED WIRE REINFORCEMENT (WWR) OF COLD-DRAWN WIRE (70,000 PSI YIELD) CONFORMING TO ASTM A185, OF SIZE INDICATED ON DRAWINGS.

CR4 WELD HEADED STUDS AND DEFORMED BAR ANCHORS (DBA) BY FULL-FUSION PROCESS, AS FURNISHED BY TRW NELSON STUD WELDING DIVISION OR EQUIVALENT.

CR5 DETAIL CONCRETE REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI 315.

CR6 PROVIDE CLEAR CONCRETE COVER MEASURED TO OUTERMOST BAR, STIRRUP OR TIE AS FOLLOWS:

EXPOSURE CONDITION	BAR SIZE OR USE	MINIMUM COVER
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH (WITH OR WITHOUT VAPOR RETARDER):	ALL	3"
EXPOSED TO EARTH OR WEATHER	#5 BAR AND SMALLER	1 1/2"
	#6 BAR AND LARGER	2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND: (TYPICAL INTERIOR CONDITIONS, CRAWL SPACES, AND UNDERSIDE OF PARKING FLOORS ARE IN THIS CATEGORY)	SLABS AND WALLS	1"
	WIDE MODUL

POST-INSTALLED CONCRETE ANCHORS:

PA1 USE POST-INSTALLED ANCHORS ONLY WHERE SPECIFIED ON CONSTRUCTION DOCUMENTS. OBTAIN APPROVAL FROM ARCHITECT PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.

PA2 DRILL AND CLEAN HOLES IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS.

PA3 PROVIDE POST INSTALLED CONCRETE ANCHORS OF SIZE, TYPE, AND QUANTITY AS NOTED ON DETAILS AS MANUFACTURED BY HILTI OR EQUIVALENT.

PA4 USE ONLY ANCHORS FROM ONE MANUFACTURER ON THIS PROJECT.

PA5 PROVIDE HOLES IN CONNECTION PLATES FOR EXPANSION BOLTS NOT MORE THAN 1/16" LARGER THAN BOLT DIAMETER. IF FIELD CONDITIONS OR TOLERANCES REQUIRE LARGER HOLES, PROVIDE PLATE WASHERS WITH BOLTS HOLES 1/16" LARGER THAN BOLT DIAMETER THAT ARE WELDED TO CONNECTION PLATE. SIZE PLATE WASHERS AND WELDS TO TRANSFER SHEAR AND TENSION CAPACITY OF EXPANSION BOLT.

PA6 INSTALL POST INSTALLED BOLTS IN ACCORDANCE WITH MANUFACTURER'S WRITTEN RECOMMENDATIONS.

PA7 PRIOR TO DRILLING FOR ANCHORS, USE A PACHOMETER OR ANOTHER APPROVED METHOD TO LOCATE EXISTING REINFORCING TO ENSURE THERE IS NO CONFLICT. IF A CONFLICT EXISTS, ANCHOR POSITION CAN BE ADJUSTED BY NO MORE THAN 1 INCH. IF A LARGER DEVIATION IS NECESSARY, CONTACT ARCHITECT. FILL ABANDONED OR UNUSED HOLES WITH EPOXY GROUT.

PA8 ONCE FINAL POSITION OF THE ANCHORS IS SET, USE TEMPLATES TO TRANSFER FINAL LOCATION OF BOLT GROUP ON TO CONNECTION PLATE.

PA9 CONSTRUCTION OF POST INSTALLED ANCHORS REQUIRES CONTINUOUS INSPECTION BY TESTING AGENCY TO ENSURE PROPER EMBEDMENT AND INSTALLATION PER MANUFACTURER'S SPECIFICATIONS.

PA10 USE ONLY PERSONNEL CERTIFIED BY ACI/CRSI ADHESIVE ANCHOR INSTALLER PROGRAM TO INSTALL ANCHORAGE SYSTEM IN A HORIZONTAL OR UPWARDLY INCLINED ORIENTATION AND WHERE SUPPORTING SUSTAINED TENSION LOADS.

STRUCTURAL STEEL FRAMING:

SS1 PROVIDE STRUCTURAL STEEL MEETING FOLLOWING MINIMUM YIELD STRENGTHS AND ASTM SPECIFICATIONS:

STRUCTURAL STEEL TYPE	YIELD STRESS, Fy	TENSILE STRESS, Fu	ASTM SPECIFICATION
WIDE FLANGE AND WT SHAPES	50 KSI	65 KSI	A992
	65 KSI	80 KSI	A913, GRADE 65
HOLLOW STRUCTURAL SECTIONS	46 KSI ROUND 50 KSI RECTANGULAR	62 KSI	A500, GRADE C
PIPES	35 KSI	60 KSI	A53 TYPE E, GRADE B
ANGLES	36 KSI	58 KSI	A36
CHANNELS	36 KSI	58 KSI	A36
BARS	36 KSI	58 KSI	A36
PLATES	36 KSI	58 KSI	A36
	50 KSI	65 KSI	A572, GRADE 50
BOLTS (HEAVY-HEX)	92 KSI	120 KSI	A325
NUTS (HEAVY-HEX)	92 KSI	120 KSI	A563, GRADE DH
WASHERS	36 KSI	58 KSI	F436
ANCHOR RODS	36 KSI	58 KSI	F1554, GRADE 36
	55 KSI	75 KSI	F1554, GRADE 55, S1
	105 KSI	125 KSI	F1554, GRADE 105
SHEAR-STUD CONNECTORS	50 KSI	65 KSI	A108
RAISED-PATTERN FLOOR PLATE	-	-	A786, COMMERCIAL GRADE

SS2 DRAWINGS INDICATE VARIOUS NON-PRIMARY STRUCTURAL FRAME ELEMENTS SUCH AS SHELF ANGLES, LINTELS, SUPPORT MEMBERS FOR CURTAIN WALLS OR MASONRY, AND EDGE ANGLES FOR OPENINGS AND PERIMETER CONDITIONS WHICH ARE INTENDED TO SUPPORT OR BE COORDINATED WITH MATERIALS FURNISHED BY OTHER TRADES. WHERE THESE NON-PRIMARY STRUCTURAL FRAME ELEMENTS ARE INDICATED TO BE WELDED IN DRAWINGS, CONTRACTOR SHALL HAVE FINAL RESPONSIBILITY FOR DETERMINING IF WELDS SHOULD BE MADE IN SHOP OR FIELD IN ORDER TO ACHIEVE PROPER FIT-UP OF VARIOUS COMPONENTS OF FRAMING AND FOR MEETING TOLERANCES REQUIRED BY OTHER TRADES.

SS3 CONTRACTOR IS RESPONSIBLE FOR PROPER FIT-UP OF VARIOUS COMPONENTS OF FRAMING AND FOR MEETING TOLERANCES REQUIRED BY OTHER TRADES, WHICH MAY BE MORE STRINGENT THAN A.I.S.C. TOLERANCES FOR STRUCTURAL STEEL.

SS4 USE 3/4" DIAMETER ASTM A 325-N HIGH-STRENGTH BOLTS FOR STEEL BEAM AND COLUMN, UNO. ALL BOLTED CONNECTIONS ARE BEARING TYPE, UNLESS NOTED OTHERWISE.

SS5 PROVIDE WELDING IN ACCORDANCE WITH ANSI/AWS D1.1 STRUCTURAL WELDING CODE USING 70 KSI LOW HYDROGEN ELECTRODES.

SS6 STEEL CONNECTION NOTES
- A. DESIGN OF STRUCTURAL STEEL CONNECTIONS, WHICH ARE NOT FULLY DETAILED OR SCHEDULED WITHIN CONTRACT DOCUMENTS IS RESPONSIBILITY OF CONTRACTOR. PROVIDE CONNECTION DESIGNS USING LRFD DESIGN PRINCIPLES PREPARED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN STATE IN WHICH THE PROJECT IS LOCATED (ENGINEER OF CONTRACTOR) IN ACCORDANCE WITH LATEST ADOPTED EDITION OF AISC 303, "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- B. ENGINEER OF CONTRACTOR SHALL REVIEW AND CONFIRM IN WRITING THAT SHOP AND ERECTION DRAWINGS PROPERLY INCORPORATE CONNECTION DESIGNS PREPARED BY ENGINEER OF CONTRACTOR AND PROPERLY INCORPORATE DESIGNS SPECIFICALLY DETAILED OR SCHEDULED WITHIN CONTRACT DOCUMENTS PRIOR TO SUBMITTING CONNECTION CALCULATIONS AND SHOP DRAWINGS FOR REVIEW.
- C. EACH CONNECTION ON SHOP DRAWINGS AND ERECTION DRAWINGS TO IDENTIFY SPECIFIC CONNECTION DESIGN THAT WAS UTILIZED, WHETHER DESIGNED BY ENGINEER OF CONTRACTOR OR WHETHER SPECIFICALLY DETAILED OR SCHEDULED WITHIN CONTRACT DOCUMENTS.
- D. CONNECTION DESIGN FORCES ARE INDICATED WITHIN CONTRACT DOCUMENTS AND ARE IN UNITS OF KIPS AND FEET. UNLESS NOTED OTHERWISE, SPECIFIED CONNECTION DESIGN FORCES ARE ULTIMATE LOADS (LRFD LOADS).
- E. WHERE HORIZONTAL FORCES ARE INDICATED ON PLAN AS "H= .K" (LRFD), SIZE CONNECTION TO TRANSFER HORIZONTAL FORCE IN ADDITION TO REQUIRED VERTICAL REACTION. SLOTTED HOLES ARE NOT PERMITTED AT CONNECTIONS WITH HORIZONTAL FORCES.
- F. UNLESS NOTED OTHERWISE, CONNECT MISCELLANEOUS STEEL MEMBERS WITH FILLET WELDS AS REQUIRED TO DEVELOP FULL TENSION CAPACITY OF SMALLEST MEMBER BEING JOINED.
- G. UNLESS NOTED OTHERWISE, DETAIL MOMENT CONNECTIONS WITH COMPLETE JOINT PENETRATION WELD AND DESIGN TO DEVELOP FULL FLEXURAL CAPACITY OF MEMBER.
- H. PROVIDE WELDED SPACER PLATES AT THREE FEET ON CENTER AT BACK TO BACK CHANNELS OR DOUBLE ANGLES.
- I. PROVIDE MINIMUM 1/4" THICK CLOSURE PLATES AT ENDS OF HOLLOW STRUCTURAL STEEL SECTIONS UNLESS SPECIFICALLY DETAILED WITH A CAP OR BASE PLATE. MATCH INSIDE DIMENSION OF HOLLOW SECTION AND WELD ALL AROUND WITH GROOVE WELD.
- J. TIGHTEN BOLTS IN CONNECTIONS TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC UNLESS NOTED OTHERWISE. TENSION CONTROL BOLTS MAY BE USED IN LIEU OF CONVENTIONAL HIGH-STRENGTH BOLTS, UNO. TENSION CONTROL BOLTS ARE NOT PERMITTED IN CRAWLSPACE CONNECTIONS.

SS7 PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ELEVATION. GROUT UNDER BEARING AND BASEPLATES IMMEDIATELY AFTER ANY REQUIRED PLUMBING OF COLUMN, BEAM OR FRAME AND PRIOR TO APPLICATION OF LOADS. PROVIDE HIGH STRENGTH, NON-SHRINK, NON-METALLIC GROUT BELOW BASE PLATES WITH A MINIMUM COMPRESSIVE STRENGTH OF 10,000 PSI.

SS8 PAINT STRUCTURAL STEEL, EXCEPT EMBEDDED ITEMS, WITH ONE SHOP COAT OF RUST INHIBITIVE PAINT UNLESS OTHERWISE NOTED. DO NOT PAINT TOP FLANGE OF COMPOSITE STEEL BEAMS OR SURFACE OF COLUMNS TO WHICH SHEAR STUD CONNECTORS WILL BE WELDED. DO NOT PAINT SURFACES TO RECEIVE APPLIED FIREPROOFING.

SS9 HOT DIP GALVANIZE STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO EARTH OR WEATHER OR LOCATED IN CRAWLSPACE WITH G-90 COATING. HOT DIP GALVANIZE COOLING TOWER GRILLAGE WITH G-135 COATING. TOUCH UP ANY DAMAGE TO GALVANIC MATERIAL DURING WELDING WITH GALVANIZING REPAIR PAINT. HIGH-ZINC-DUST-CONTENT PAINT FOR REGALVANIZING WELDS AND REPAIR PAINTING GALVANIZED STEEL, WITH DRY FILM CONTAINING NOT LESS THAN 93 PERCENT ZINC DUST BY WEIGHT, AND COMPLYING WITH DOD-P-21035A OR SSPC-PAINT 20.

SS10 DESIGN, FURNISH, AND INSTALL TEMPORARY SUPPORTS, BRACES, FALSE WORK, SHORES, CRIBBING OR OTHER SUPPORTS REQUIRED DURING ERECTION TO KEEP STRUCTURAL STEEL SECURE, SUPPORTED, PLUMB, AND IN ALIGNMENT.

SS11 WHERE FILLET WELD SIZE IS NOT INDICATED ON DRAWINGS, PROVIDE MINIMUM WELD PER AISC.

SS12 USE SLIP CRITICAL BOLTS OR WELDED CONNECTIONS AT BEAMS WITH LATERAL BRACING ATTACHED AND WEB CONNECTIONS OF MOMENT-CONNECTED BEAMS.

SS13 PERFORM WELDING BY WELDERS CERTIFIED FOR WELDS TO BE MADE.

SS14 PROVIDE ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS), WHERE INDICATED ON CONTRACT DOCUMENTS, CONFORMING TO AISC 303 SECTION 10.

SS15 INSTALL ANGLES FOR FASCIA BRICK SUPPORT SUCH THAT SWEEP OR CAMBER DOES NOT EXCEED 1/8 INCH DEVIATION FROM STRAIGHT LINE.

SS16 ANY SUPPORTING ELEMENTS SUSPENDED FROM STEEL BEAM FRAMING SHALL BE ATTACHED CONCENTRICALLY TO THE BEAM CENTERLINE.

METAL ROOF DECKING:

MD1 COMPLY WITH CURRENT STEEL DECK INSTITUTE SPECIFICATIONS. METAL ROOF DECKING SIZES ARE INDICATED ON STRUCTURAL DRAWINGS AND WITH FOLLOWING MINIMUM PROPERTIES:

METAL ROOF DECK SCHEDULE									
TYPE	GAUGE	PROFILE	DEPTH (IN)	MOMENT OF INERTIA (+) (IN4)	MOMENT OF INERTIA (-) (IN4)	SECTION MODULUS (+) (IN3)	SECTION MODULUS (-) (IN3)	YIELD STRENGTH (KSI)	NRC
1.5WR	18	WIDE RIB	1.5	0.289	0.295	0.318	0.327	50	-

MD2 REFER TO ROOF DIAPHRAGM FASTENER DIAGRAM AND SCHEDULE FOR REQUIRED ATTACHMENT PATTERN.

MD3 METAL ROOF DECK IS DESIGNED TO BE CONTINUOUS OVER THREE OR MORE SPANS UNLESS FEWER THAN THREE SPANS ARE INDICATED ON STRUCTURAL DRAWINGS.

MD4 PROVIDE METAL ROOF DECKING: GALVANIZED WITH G60 COATING CONFORMING TO ASTM A924.

MD5 COORDINATE METAL DECK LENGTHS WITH FINAL JOIST AND BEAM LAYOUT. FINAL JOIST AND BEAM LAYOUT CAN BE DIFFERENT THAN THAT INDICATED IN STRUCTURAL DRAWINGS DEPENDING ON WHETHER JOIST BEARING SEATS ARE BUTTED OR STAGGERED. JOIST LOCATIONS INDICATED IN STRUCTURAL DRAWINGS DO NOT ACCOUNT FOR SMALL DIFFERENCE IN JOIST LOCATION DUE TO VARIOUS JOIST BEARING CONDITIONS THAT COULD EXIST IN THE FIELD.

MD6 DO NOT SUSPEND APPURTENANCES SUCH AS LIGHT FIXTURES, DUCTS AND OTHER UTILITIES FROM METAL ROOF DECK.

COMPOSITE STEEL CONSTRUCTION:

CSC1 PROVIDE COMPOSITE METAL FLOOR DECK WITH TOP SURFACE PHOSPHATIZED AND UNDERSIDE SURFACE SHOP PRIMED WITH MANUFACTURERS' STANDARD GRAY OR WHITE PRIMER AND WITH FOLLOWING MINIMUM PROPERTIES:

METAL FLOOR DECK SCHEDULE									
TYPE	GAUGE	PROFILE	DEPTH (IN)	MOMENT OF INERTIA (+) (IN4)	MOMENT OF INERTIA (-) (IN4)	SECTION MODULUS (+) (IN3)	SECTION MODULUS (-) (IN3)	YIELD STRENGTH (KSI)	NRC
3.0CI	18	COMPOSITE INTERLOCKING	3.0	1.252	1.252	0.761	0.794	50	-

CSC2 COMPOSITE STEEL FRAMING IS DESIGNED AS NON-SHORED CONSTRUCTION. DO NOT SHORE BEAMS DURING CONCRETE PLACEMENT OPERATION.

CSC3 COMPOSITE METAL DECK, SLAB AND SLAB REINFORCING
- A. PROVIDE COMPOSITE METAL DECK AND SLAB INDICATED ON STRUCTURAL DRAWINGS.
- B. INSTALL DECK IN THREE SPAN LENGTHS WITHOUT SHORING UNLESS FEWER THAN THREE SPANS ARE INDICATED ON STRUCTURAL DRAWINGS.
- C. UNLESS SPECIFIED OTHERWISE ON STRUCTURAL DRAWINGS, REINFORCE SLAB OVER METAL DECK WITH WELDED WIRE MESH, PROVIDED IN FLAT SHEETS AS FOLLOWS:
  1. 2" TO 3 1/4" SLAB THICKNESS OVER TOP OF DECK - 6X6-W1.4XW1.4
  2. 3 1/2" TO 4 1/2" SLAB THICKNESS OVER TOP OF DECK - 6X6-W2.1XW2.1
  3. GREATER THAN 4 1/2" OVER TOP OF DECK - TWO LAYERS OF 6X6-W2.1XW2.1
  4. PROVIDE A 6'-0" WIDE EXTRA LAYER OF WELDED WIRE MESH IN TOP OF SLAB OVER ALL GIRDERS THAT ARE PARALLEL TO DECK SPAN TYPICAL, UNLESS NOTED OTHERWISE.
  5. LAP EDGES AND ENDS OF ADJOINING SHEETS. PROVIDE LENGTH OF LAP MEASURED BETWEEN OUTER MOST CROSS WIRES OF FABRIC SHEET AND NOT LESS THAN ONE SPACING OF CROSS WIRES PLUS 2 INCHES. LACE OVERLAPS WITH WIRE.
  6. SUPPORT WELDED WIRE MESH ON CHAIRS WITH A 3/4" COVER FROM TOP OF SLAB.
- D. PROVIDE 14 GAUGE COLD FORMED STEEL POUR STOPS WELDED TO STEEL BEAM FLANGE WITH 1" FILLET WELDS AT 12" OC. UNLESS NOTED OTHERWISE.

CSC4 STEEL BEAM CAMBER
- A. DURING SHIPMENT AND HANDLING SUPPORT CAMBERED MEMBERS IN A MANNER THAT WILL MINIMIZE LOSS OF CAMBER.
- B. ERECT MEMBERS WITH CAMBER UP.

CSC5 COMPOSITE STEEL DECK CONNECTIONS
- A. INSTALL DECK WITH ENDS ALIGNED OVER SUPPORTING FRAME WITH A MINIMUM END BEARING LENGTH OF 1.5" WITH END JOINTS BUTTED WITH A MAXIMUM GAP OF 1". DO NOT OVERLAP.
- B. ATTACH COMPOSITE STEEL DECK TO SUPPORTING STEEL AT 12" OC MAX. ATTACH STEEL DECK TO SUPPORTING STEEL BEAMS BY WELDING SPECIFIED SHEAR CONNECTORS THROUGH DECK BY THE FULL-FUSION PROCESS, AS FURNISHED BY TRW NELSON STUD WELDING DIVISION OR EQUIVALENT. WHERE SPECIFIED SHEAR CONNECTOR SPACING EXCEEDS 12" OC, PROVIDE 3/4" PUDDLE WELDS BETWEEN SHEAR STUD CONNECTORS TO MAINTAIN A MAXIMUM DECK CONNECTION SPACING OF 12" OC. WHERE DECK UNITS ABUT SIDE TO SIDE OR END TO END OVER A STEEL BEAM OR ABUT TO A GIRDER FILLER, PROVIDE 3/4" PUDDLE WELDS AT 12" OC ON EACH DECK UNIT.
- C. WHERE COMPOSITE STEEL DECK IS SUPPORTED BY A STEEL LEDGE ANGLE ATTACHED TO A CONCRETE OR MASONRY WALL, ATTACH DECK TO LEDGE ANGLE WITH 3/4" PUDDLE WELDS AT 12" OC.
- D. WELD GIRDER FILLERS TO STEEL BEAM WITH 2" LONG FILLET WELDS AT A MAXIMUM SPACING OF 12 INCHES.
- E. CONNECT SIDES OF ADJACENT UNITS WITH 1 1/2" LONG WELDS AT 3'-0" OC MAX.

CSC6 CONCRETE PLACEMENT
- A. DO NOT PLACE EXCESSIVE AMOUNTS OF CONCRETE IN ANY ONE LOCATION ON METAL DECK OR ANY ONE BEAM DURING CONCRETE PLACEMENT OPERATION. MONITOR DEFLECTION OF STEEL BEAMS, SLAB THICKNESS AND PLACING OF CONCRETE TO PREVENT EXCESSIVE LOAD ON ANY BEAM.
- B. CONCRETE PLACEMENT IN RESPONSE TO SUPPORTING STEEL DEFLECTIONS.
  1. PLACE FLOOR ELEVATIONS, UNLESS SPECIFICALLY NOTED OTHERWISE, WITHIN SPECIFIED TOLERANCES. HOWEVER, DECK AND SUPPORTING STEEL WILL DEFLECT AT VARIOUS LOCATIONS DURING CONCRETE PLACEMENT WHICH WILL HAVE IMPACT ON THICKNESS OF SLAB.
  2. STEEL BEAMS WILL NOT DEFLECT AT SUPPORT LOCATIONS SUCH AS COLUMNS, WALLS, PERIMETER GRADE BEAMS, ETC. MAINTAIN SPECIFIED SLAB THICKNESS AND SPECIFIED FINISHED FLOOR ELEVATION AT BEAM SUPPORT LOCATIONS.
  3. DECK WILL DEFLECT BETWEEN SUPPORTS. THEREFORE, SLAB THICKNESS AT MIDSPAN OF DECK WILL BE GREATER THAN SLAB THICKNESS AT DECK SUPPORT LOCATIONS. MAXIMUM ANTICIPATED DECK DEFLECTION IS **3/8"**.
  4. BEAMS WITHOUT A SPECIFIED CAMBER WILL DEFLECT. THEREFORE, SLAB THICKNESS AT THESE BEAMS AND BEAMS THEY SUPPORT, IF ANY, WILL INCREASE OVER SPECIFIED THICKNESS. ANTICIPATED MAXIMUM DEFLECTION OF MEMBERS WITHOUT A SPECIFIED CAMBER IS **3/4"**.
  5. BEAMS WITH SPECIFIED CAMBERS ARE SIZED TO DEFLECT AN AMOUNT EQUAL TO OR SLIGHTLY GREATER THAN SPECIFIED CAMBER. THEREFORE, IF ACTUAL CAMBER IS EQUAL TO SPECIFIED CAMBER, SLAB THICKNESS AT THESE BEAMS AND BEAMS THEY SUPPORT, IF ANY, WILL BE EQUAL TO OR SLIGHTLY THICKER THAN SPECIFIED THICKNESS.
  6. SLAB THICKNESS TOLERANCES:
     - a. AT LOCATIONS WHERE SUPPORTING STEEL AND DECK DO NOT DEFLECT, PLUS SLAB THICKNESS TOLERANCE IS +1/4". AT LOCATIONS WHERE SUPPORTING STEEL AND DECK DO DEFLECT, A PLUS SLAB THICKNESS TOLERANCE IS NOT APPLICABLE SINCE THE SLAB CAN BE THICKER THAN SPECIFIED THICKNESS AT SOME LOCATIONS DUE TO THE DEFLECTION OF THE SUPPORTING STRUCTURE DURING CONCRETE PLACEMENT. HOWEVER, NOTIFY ARCHITECT IMMEDIATELY IF ATTEMPTS TO PROVIDE A SPECIFIED FINAL FLOOR ELEVATION RESULT IN A SLAB THICKNESS GREATER THAN SPECIFIED SLAB THICKNESS BY MORE THAN **1 1/2"**.
     - b. MINUS SLAB THICKNESS TOLERANCE IS -1/4". HOWEVER, NOTIFY ARCHITECT IMMEDIATELY IF ATTEMPTS TO PROVIDE A SPECIFIED FINAL FLOOR ELEVATION RESULTS IN A SLAB THICKNESS THAT IS LESS THAN SPECIFIED THICKNESS, WITH TOLERANCE, DUE TO RESIDUAL CAMBER IN THE BEAMS.
  7. ADJUSTMENTS TO CONTRACT PRICE FOR CONCRETE WILL NOT BE MADE DUE TO EFFECTS OF DECK AND BEAM DEFLECTIONS

CSC7 SHEAR CONNECTORS: ASTM A108, AISI C-1015 THROUGH C-1020, HEADED STUD TYPE, COLD-FINISHED CARBON STEEL; AWS D1.1, TYPE B.
- A. PROVIDE SHEAR CONNECTORS IN ACCORDANCE WITH FOLLOWING:

DECK THICKNESS	TOTAL SLAB THICKNESS	SHEAR CONNECTOR SIZE
1.5" & 2"	4.5"	3/4" DIA X 4"
2", 3", 3.5"	>= 5.5"	3/4" DIA X 5"

- B. SHEAR CONNECTORS INDICATED ARE FINAL INSTALLATION LENGTHS.
- C. APPLY SHEAR CONNECTORS IN FIELD AND PLACE IN DECK FLUTES IN CONTACT WITH TOP FLANGE OF BEAM.
- D. ESTABLISH APPROPRIATE SHEAR CONNECTOR PLACEMENT PRIOR TO INSTALLATION OF SHEAR CONNECTORS.

CSC8 DO NOT LOAD COMPOSITE FLOORS UNTIL CONCRETE REACHES 75 PERCENT OF ITS SPECIFIED STRENGTH.

CSC9 NO CONDUITS ALLOWED IN SLABS.

CSC10 DO NOT HANG CONCENTRATED LOAD EXCEEDING 100LB FROM COMPOSITE METAL DECKING. SUPPORT MEP APPURTENANCES EXCEEDING 100LB BY STEEL BEAMS OR JOISTS.

BAR GRATING:

BG1 DESIGN GRATING PER SPECIFICATIONS AND PER LOADS INDICATED IN CONTRACT DOCUMENTS.

BG2 GALVANIZE GRATING AND COMPLY WITH NAAMM STANDARDS AND TOLERANCES.

BG3 WELD EVERY THIRD BEARING BAR TO SUPPORTING BEAMS WITH 3/16"x1" FILLET WELDS.

BG4 PROVIDE MINIMUM BEARING LENGTHS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS AS REQUIRED TO ACHIEVE ALLOWABLE LOADS LISTED IN MANUFACTURERS LOAD TABLES.

BG5 PROVIDE MAXIMUM DISTANCE BETWEEN END OF GRATING AND VERTICAL LEG OF EDGE ANGLES OF 1/2".

ELEVATOR FRAMING:

EF1 PROVIDE HEAVIER OR REINFORCED GUIDE RAILS WHICH DO NOT REQUIRE INTERMEDIATE STRUCTURAL SUPPORT WHERE FLOOR TO FLOOR DISTANCE IS GREATER THAN ALLOWABLE SPAN FOR ELEVATOR'S STANDARD RAILS. IF REINFORCED RAILS ARE NOT POSSIBLE, PROVIDE ALLOWANCE FOR SUPPLEMENTAL STEEL SPANNING BETWEEN FLOORS AS TYPICAL ELEVATOR RAILING SUPPORT. DESIGN THIS SUPPLEMENTAL STEEL, AS A DELEGATED DESIGN, BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN STATE IN WHICH PROJECT IS LOCATED. IF ELEVATOR RAIL SUPPORT FRAMING APPEARS IN STRUCTURAL DRAWINGS, IT'S INTENT IS DIAGRAMATIC ONLY. REFER TO ELEVATOR DRAWINGS FOR EXACT NUMBER OF ELEVATOR RAILS.

EF2 COORDINATE ELEVATOR PIT DIMENSIONS AND ROUGH FLOOR OPENINGS AND CLEAR HEIGHTS FOR ELEVATORS WITH FINAL FRAMING SHOP DRAWINGS. CLEARLY INDICATE CHANGES TO DIMENSIONS INDICATED ON STRUCTURAL DRAWINGS ON FRAMING SHOP DRAWINGS.

EF3 CONFIRM ELEVATOR REACTIONS INDICATED ON STRUCTURAL DRAWINGS WITH FINAL ELEVATOR SHOP DRAWINGS. CLEARLY INDICATE ON SHOP DRAWINGS ANY CHANGES TO REACTIONS INDICATED ON STRUCTURAL DRAWINGS.

EF4 COORDINATE REQUIRED ELEVATOR EMBEDS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.

PRECAST CONCRETE WALL PANELS:

PCW1 DESIGN PRECAST IN ACCORDANCE WITH PROVISIONS OF ACI AND PRECAST CONCRETE INSTITUTE TO PROVIDE POSITIVE LOAD PATH TO FOUNDATIONS FOR OUT OF PLANE AND IN PLANE LATERAL LOADS NOTED IN CONTRACT DOCUMENTS IN ADDITION TO THEIR SELF WEIGHT AND OTHER GRAVITY LOADING FROM FLOOR OR ROOF. WHERE PANELS DO NOT RECEIVE GRAVITY LOADING FROM FLOORS OR ROOF, PROVIDE FOR AT LEAST 1.50" OF VERTICAL MOVEMENT IN FLOOR AND AT LEAST 1.50" OF VERTICAL DOWNWARD MOVEMENT, 0.50" OF VERTICAL UPWARD MOVEMENT IN ROOF FRAMING CONNECTIONS TO PRECAST PANELS.

PCW2 ONLY WALLS INTENDED TO BE PART OF LATERAL FORCE RESISTING SYSTEM, PROVIDE CONNECTIONS BETWEEN PRECAST WALLS AND FLOOR AND ROOF FRAMING TO TRANSMIT OUT-OF-PLANE WIND AND SEISMIC FORCES BUT ARE CONFIGURED TO PREVENT THE TRANSFER OF IN-PLANE FORCES. CONFIGURE CONNECTIONS FOR IN-PLANE LATERAL MOVEMENT OF 2.50 IN. AT LEVEL 2 AND 4.00 IN. AT THE ROOF IN EITHER DIRECTION. ALLOW FOR 2.00 IN. INTERSTORY DRIFT BETWEEN LEVEL 2 AND THE ROOF. ADDITIONALLY, DESIGN CONNECTIONS FOR FORCES THAT MAY RESULT FROM OUT-OF-PLANE DISPLACEMENTS OF 3.50 IN. AT LEVEL 2 AND 6.50 IN. AT THE ROOF WITH MAXIMUM INTERSTORY DRIFT BETWEEN THE 2ND LEVEL AND THE ROOF OF 3.00 IN.

PCW4 PROVIDE COUPLERS AT FOUNDATION (OR AT JOINTS ALONG HEIGHT OF PANELS) CAPABLE OF RESISTING 150% OF YIELD STRENGTH OF REINFORCEMENT BEING COUPLED.

PCW6 FURNISH AND INSTALL INSERTS, BRACES, STRONG-BACKS, AND OTHER REQUIRED LIFTING ACCESSORIES AND LOCATE IN ACCORDANCE WITH ACCEPTED PRACTICES. DESIGN LOCATIONS AND SIZES OF INSERTS, BRACES, OR STRONG-BACKS TO PREVENT CRACKING DUE TO ERECTION STRESSES. SHOULD PROPER INSERTS OR BRACES NOT BE PROVIDED TO MEET THE ABOVE CRITERIA, PROVIDE SPECIAL REINFORCING, AS MAY BE REQUIRED, TO PREVENT FLEXURAL CRACKS FROM OCCURRING IN PANELS DURING LIFTING AND HANDLING OPERATIONS.

PCW7 TEMPORARILY BRACE PANELS SECURELY IN ERECTED POSITION UNTIL THEY CAN BE PERMANENTLY ANCHORED AND BRACED AT FLOOR AND ROOF. DO NOT REMOVE PANEL BRACES UNTIL ROOF STRUCTURE AND DECK ARE COMPLETELY INSTALLED AND ADJOINING FLOOR SLAB IS PLACED AND CURED FOR A MINIMUM OF 7 DAYS.

PCW10 INSULATED PRECAST PANELS:
- A. REFERENCE ARCHITECTURAL DRAWINGS FOR PANEL FINISHES, THICKNESS, AND REVEAL PATTERNS. PROVIDE MAXIMUM 3/4" DEPTH REVEALS
- B. REFERENCE ARCHITECTURAL DRAWINGS FOR DETAILS OF INSULATION OUTSIDE STRUCTURAL PORTION OF PANELS.

STATEMENT OF SPECIAL INSPECTIONS:

SI1 THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE WITH LOCAL AMENDMENTS. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTIONS SERVICES APPLICABLE TO THIS PROJECT.

SI2 THE OWNER SHALL EMPLOY ONE OR MORE QUALIFIED SPECIAL INSPECTORS, APPROVED BY THE BUILDING OFFICIAL, TO PERFORM SPECIAL INSPECTIONS AND TESTS DURING CONSTRUCTION IN ACCORDANCE WITH BUILDING CODE. SPECIAL INSPECTORS SHALL PERFORM ALL DUTIES AND RESPONSIBILITIES AS REQUIRED BY THE BUILDING CODE. JOB SITE VISITS BY THE STRUCTURAL ENGINEER DO NOT CONSTITUTE AND ARE NOT A SUBSTITUTE FOR SPECIAL INSPECTIONS. ALL INSPECTORS SHALL BE QUALIFIED BY TRAINING AND EXPERIENCE FOR THE REQUIRED INSPECTIONS AND TEST PROCEDURES.

SI3 THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF ALL SPECIAL INSPECTIONS AND TESTS AND SHALL SUBMIT REPORTS OF INSPECTIONS AND TESTS TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATED THAT WORK INSPECTED OR TESTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF THE WORK. INTERIM TESTING AND INSPECTION REPORTS SHALL BE SUBMITTED ON A DAILY BASIS TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. A FINAL REPORT DOCUMENTING THE REQUIRED SPECIAL INSPECTIONS AND TESTS, AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS OR TESTS, SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON PRIOR TO THE START OF WORK BY THE OWNER OR THE OWNER'S AUTHORIZED AGENT TO THE BUILDING OFFICIAL.

SI4 IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO ENSURE THE REQUIRED SPECIAL INSPECTIONS AND TESTS ARE PERFORMED IN ACCORDANCE WITH THE BUILDING CODE AND CONSTRUCTION DOCUMENTS. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO COMPLY WITH THE CONTRACT DOCUMENTS. JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

SI5 ARCHITECTURAL, MECHANICAL AND ELECTRICAL COMPONENTS REQUIRING SPECIAL INSPECTIONS AND TESTING PER THE BUILDING CODE ARE NOT LISTED HERE. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND SPECIFICATIONS FOR SPECIAL INSPECTION REQUIREMENTS FOR THESE COMPONENTS.

SI6 THE FOLLOWING SPECIAL INSPECTIONS AND TESTS SCHEDULE CONTAINS A LIST OF THE MATERIALS, SYSTEMS, COMPONENTS AND WORK REQUIRED TO HAVE SPECIAL INSPECTIONS AND/OR TESTS BY THE SPECIAL INSPECTOR RESPONSIBLE FOR EACH PORTION OF THE WORK. REFER TO THE GENERAL NOTES AND PROJECT SPECIFICATIONS FOR ADDITIONAL INSPECTION AND TESTING REQUIREMENTS. WHERE CONFLICTS OCCUR, THE MOST STRINGENT REQUIREMENT SHALL CONTROL. (2)

| SPECIAL INSPECTIONS AND TESTS SCHEDULE PER IBC 2

FIRST FLOOR LOADING LEGEND

DESIGN DEAD LOADS
SLAB ON GRADE.....SELF WT

DESIGN CABINETS STORAGE LOADS
FLOOR LOADS.....SINGLE ARRAY OF SINGLE 6,000LBS (4'-6" x 2'-0" RACK DIMENSIONS) SERVER CABINETS SEPARATED BY 6' HOT AISLES AND 6' COLD AISLES 3,000 LB (CONCENTRATED)

DESIGN LIVE LOADS
FLOOR LIVE (TYP).....40 PSF IN HOT AISLES, 100 PSF IN COLD AISLES

DESIGN DEAD LOADS
SLAB ON GRADE.....SELF WT

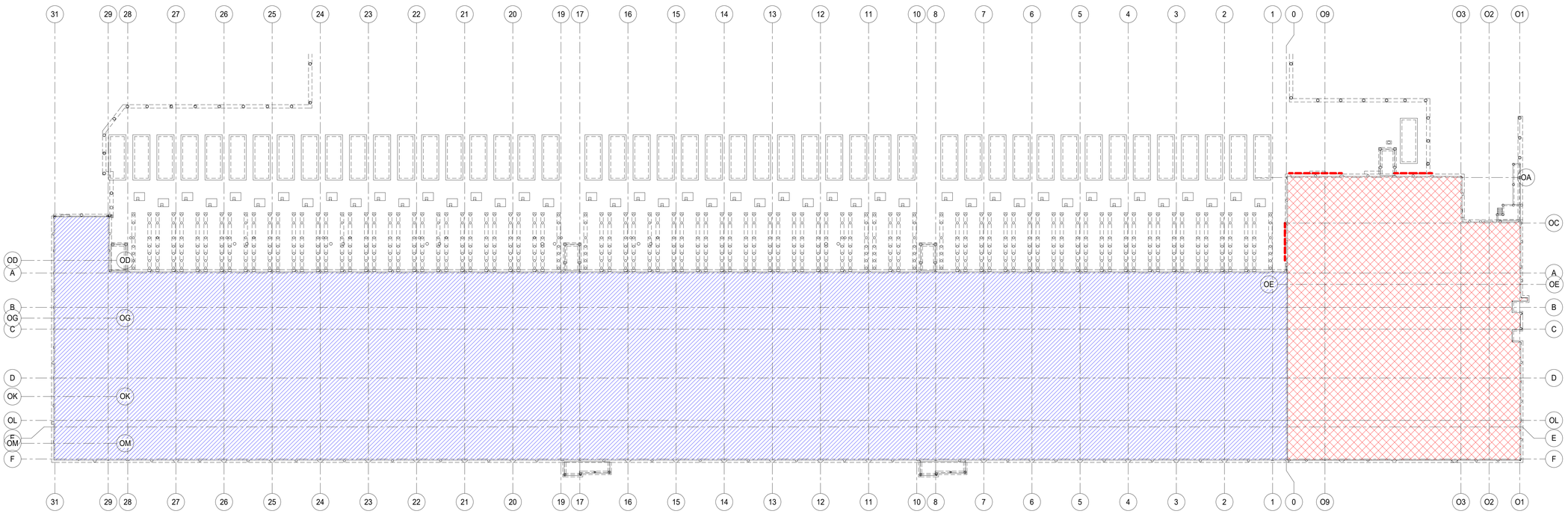
DESIGN LIVE LOADS
FLOOR LIVE LOADS.....250 PSF (REDUCIBLE) 2,000 LB (CONCENTRATED)
MECHANICAL/ELECTRICAL/STORAGE.....250 PSF (UNREDUCIBLE) 2,000 LB (CONCENTRATED)

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2021 INTERNATIONAL CODES & 2023 NEC



2 FIRST FLOOR LOADING DIAGRAM
1" = 40'-0"

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9		
10		

	REVISIONS	
1	03.28.25	ADDENDUM 1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

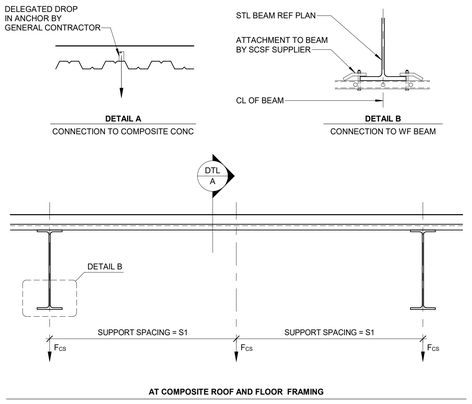
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JOB 24464.0000
DATE 09.10.2025
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S0-03

9/10/2025 5:00:53 PM S0-03 LOADING DIAGRAM



STRUCTURED CEILING SUPPORT NOTES:

1. REFER TO THE LOADING DIAGRAMS FOR COLLATERAL DEAD LOADS HANGING BELOW THE FLOORS AND ROOF FOR WHICH THE STRUCTURED CEILING SUPPORT FRAMING (SCSF) SHALL BE DESIGNED FOR.
2. DESIGN OF ALL SCSF AND ITS APPURTENANCES IS CONSIDERED DELEGATED DESIGN AND SHALL BE PERFORMED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.
3. HANGERS TO SUPPORT THESE STRUCTURED CEILINGS AND/OR THE SCSF MAY BE ATTACHED DIRECTLY TO THE COMPOSITE CONCRETE SLAB OR ROOF USING THE ANCHORS AS SHOWN IN THIS DETAIL. DESIGN OF ANY ALTERNATE HANGERS ATTACHMENT METHOD TO THE COMPOSITE CONCRETE SLAB OR ROOF SHALL BE CONSIDERED PART OF THE DELEGATED DESIGN OF THE SCSF AND WILL BECOME THE RESPONSIBILITY OF THE SCSF SYSTEM'S STRUCTURAL ENGINEER.
4. THE CONTRACTOR MUST CARRY AN APPROPRIATE ALLOWANCE IN THEIR BIDS FOR ANY MISCELLANEOUS FRAMING THAT IS REQUIRED TO BYPASS ANY OBSTRUCTION SUCH AS AN EXISTING MEP APPURTENANCE OR A FRAMING MEMBER TO TRANSMIT THE HANGING LOAD TO THE ANCHORS AS SHOWN IN THIS DETAIL.
5. IF THE STRUCTURED CEILING HANGERS ARE ATTACHED DIRECTLY TO THE COMPOSITE SLAB, ANY OTHER MEP APPURTENANCES GREATER THAN 100 POUNDS MUST BE HUNG FROM FLOOR FRAMING MEMBERS, NOT FROM THE SLAB.
6. ATTACHMENT OF THE SCSF AND/OR THE CEILING HANGER TO THE METAL ROOF DECK IS NOT ALLOWED.
7. ATTACHMENTS TO FLOOR FRAMING MEMBERS SHALL BE SYMMETRICAL TO THE CENTERLINE OF THE FRAMING MEMBER, AND ANY CLAMP TYPE DEVICES THAT INDUCE TORSIONAL STRESSES IN THE FLOOR FRAMING MEMBERS ARE NOT PERMITTED.

2 STRUCTURED CEILING ATTACHMENT DETAIL
3/4" = 1'-0"

SECOND FLOOR LOADING LEGEND

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WEIGHT
MISC (CEILING, MECH, FLOORING, ETC).....15 PSF

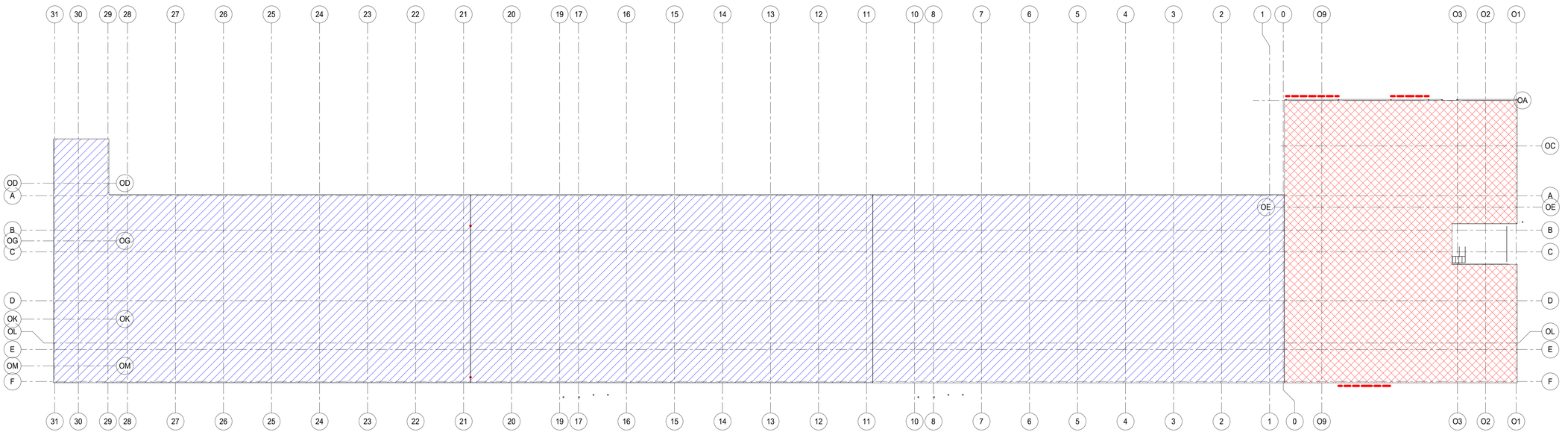
DESIGN LIVE LOADS
FLOOR LIVE (TYP).....250 PSF (UNREDUCIBLE)
250 PSF (UNREDUCIBLE AT STAIRS)
250 PSF (UNREDUCIBLE AT STORAGE ROOMS)
2,000 LB (CONCENTRATED)

DESIGN DEAD LOADS
COLLATERAL HANGING BELOW FLOOR FRAMING.....100 PSF
ATTACHMENTS TO FLOOR FRAMING MEMBERS SHALL BE SYMMETRICAL TO THE CENTERLINE OF THE FRAMING MEMBER. NO ATTACHMENT TO THE UNDERSIDE OF DECK ALLOWED. 65 PSF FOR WEIGHT OF BASE BUILDING SYSTEMS INCLUDING THE WEIGHT OF THE CEILING SYSTEM + 35 PSF HANGING LOAD AVAILABLE TO THE CUSTOMER). REF 2-S0-04 FOR ADD'L INFO.

FLOOR DEAD LOAD.....SELF WEIGHT

DESIGN SERVER CABINET STORAGE LOADS:
FLOOR LOADS.....SINGLE ARRAY OF SINGLE 6,000 LBS (4'-6" x 2'-0" RACK DIMENSIONS) SERVER CABINETS SEPARATED BY 6' HOT AISLES AND 6' COLD AISLES
3,000 LBS (CONCENTRATED)

DESIGN LIVE LOADS
FLOOR LIVE (TYP).....40 PSF IN HOT AISLES, 100 PSF IN COLD AISLES



1 SECOND FLOOR LOADING DIAGRAM
1" = 40'-0"

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City of Aurora Building Division
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Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

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	REVISIONS	

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

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S0-04

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ROOF LOADING LEGEND

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT
ROOFING AND INSULATION.....10 PSF
COLLATERAL HANGING BELOW ROOF FRAMING.....8 PSF
(INCLUDES CEILINGS, PIPING, DUCTWORK, LIGHTS, ELECTRICAL)

DESIGN LIVE LOADS
ROOF LIVE.....20 PSF (UNREDUCIBLE)
ROOF SNOW.....40 PSF + DRIFTS (UNREDUCIBLE)

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT
ROOFING INSULATION.....10 PSF
COLLATERAL HANGING BELOW FLOOR FRAMING.....100 PSF
ATTACHMENTS TO FLOOR FRAMING MEMBERS SHALL BE SYMMETRICAL TO THE CENTERLINE OF THE FRAMING MEMBER. NO ATTACHMENT TO THE UNDERSIDE OF DECK IS ALLOWED. 65 PSF FOR WEIGHT OF BASE BUILDING SYSTEMS INCLUDING THE WEIGHT OF THE CEILING SYSTEM + 35 PSF HANGING LOAD AVAILABLE TO THE CUSTOMER). REF 2-S0-04 FOR ADDTL INFO

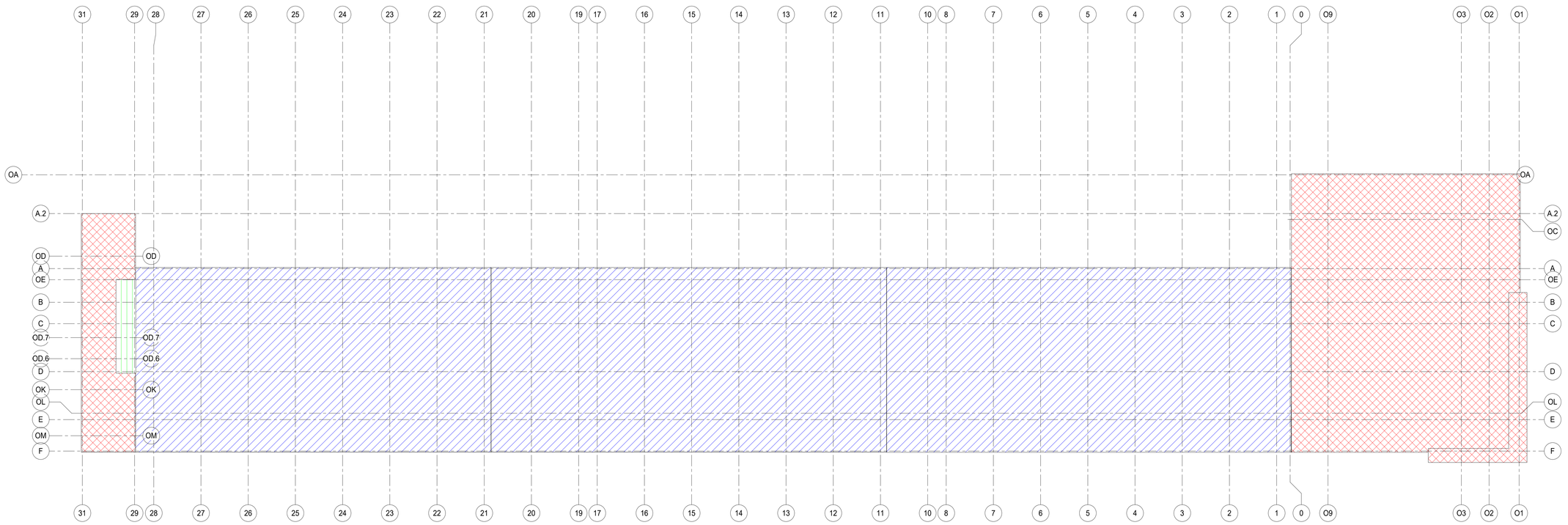
DESIGN LIVE LOADS
ROOF LIVE.....20 PSF (UNREDUCIBLE)
ROOFING INSULATION.....SEE SNOW LOADING PLAN

DESIGN LIVE LOADS
ROOF LIVE.....20 PSF (UNREDUCIBLE)
ROOF SNOW.....40 PSF + DRIFTS

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT
MISC (CEILING, MECH, FLOORING, ETC).....10 PSF

DESIGN LIVE LOADS
FLOOR LIVE.....100 PSF (UNREDUCIBLE)
100 PSF (UNREDUCIBLE AT STAIRS)
100 PSF (UNREDUCIBLE AT STORAGE ROOMS)

2,000 LBS (CONCENTRATED)



1 ROOF LOADING DIAGRAM
1" = 40'-0"

9/10/2025 5:01:00 PM S0-05 LOADING DIAGRAM

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COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

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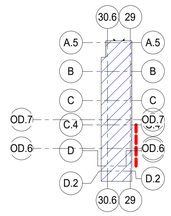
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DATE 09.10.2025
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S0-05

PENTHOUSE ROOF LOADING LEGEND

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT
ROOFING INSULATION.....10 PSF
COLLATERAL HANGING BELOW ROOF FRAMING.....8 PSF
(INCLUDES CEILINGS, PIPING, DUCTWORK, LIGHTS, ELECTRICAL)

DESIGN LIVE LOADS
ROOF LIVE.....20 PSF (UNREDUCIBLE)



PLATFORM LOADING LEGEND

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT

DESIGN LIVE LOADS
FLOOR LIVE.....60 PSF (UNREDUCIBLE)
EQUIPMENT LOADING.....SELF WT
HANGING BELOW PLATFORM FRAMING.....50 PSF (UNREDUCIBLE)

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT

DESIGN LIVE LOADS
FLOOR LIVE.....60 PSF (UNREDUCIBLE)
EQUIPMENT LOADING.....SELF WT
HANGING BELOW PLATFORM FRAMING.....100 PSF (UNREDUCIBLE)

DESIGN DEAD LOADS
FRAMING MEMBERS.....SELF WT
MISC (CEILING, MECH, FLOORING, ETC.).....15 PSF

DESIGN DEAD LOADS
FLOOR LIVE.....100 PSF (UNREDUCIBLE)
100 PSF (UNREDUCIBLE AT STAIRS)
100 PSF (UNREDUCIBLE AT STORAGE ROOM)
2,000 LB (CONCENTRATED)

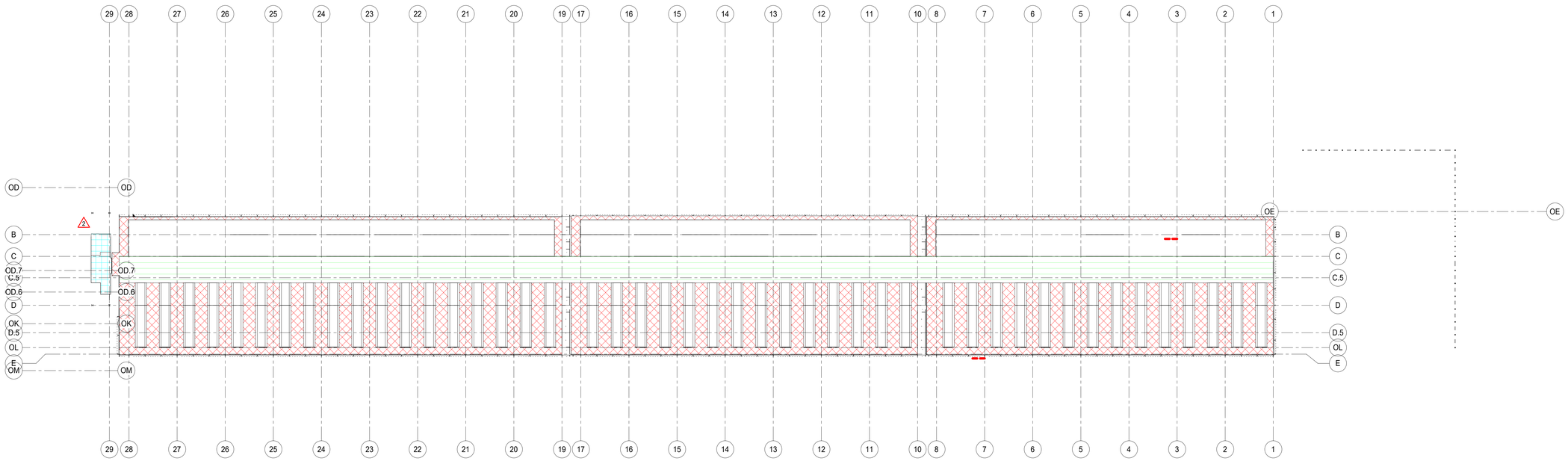
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2 PENTHOUSE ROOF LOADING DIAGRAM
1" = 40'-0"



1 PLATFORM LOADING DIAGRAM
1" = 40'-0"

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2	05.30.25	ISSUE FOR CONSTRUCTION

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S0-06

9/10/2025 5:01:01 PM S0-06 LOADING DIAGRAM

SNOW LOADING KEY

S1	55 PSF+ BALANCED SNOW LOAD, X = 17'-4"
S2	55 PSF+ BALANCED SNOW LOAD, X = 17'-4"
S3	55 PSF+ BALANCED SNOW LOAD, X = 17'-4"
S4	55 PSF+ BALANCED SNOW LOAD, X = 9'-7"
S5	55 PSF+ BALANCED SNOW LOAD, X = 17'-4"
S6	51 PSF+ BALANCED SNOW LOAD, X = 8'-0"
S7	169 PSF+ BALANCED SNOW LOAD, X = 26'-9"
S8	30 PSF+ BALANCED SNOW LOAD, X = 9'-4"
S9	96 PSF+ BALANCED SNOW LOAD, X = 15'-2"

SNOW LOADING PRESENTED FOR INFORMATION ONLY. DRIFTS HAVE BEEN CONSIDERED IN JOIST AND JOIST GIRDER SELECTIONS SHOWN ON PLANS.

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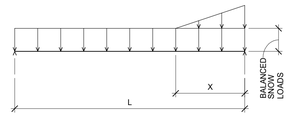
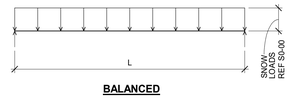
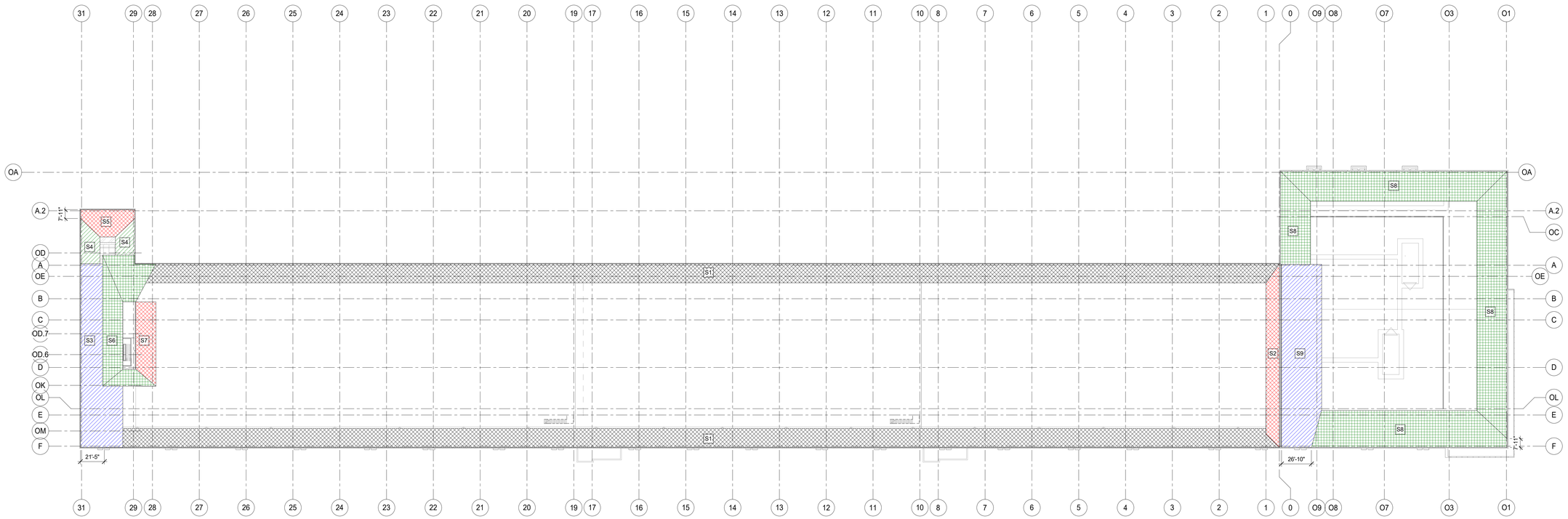


DIAGRAM 1



BALANCED



1 ROOF SNOW LOADING DIAGRAM
1" = 40'-0"

9/10/2025 5:01:02 PM S0-07 SNOW LOAD DIAGRAM

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COLORADO LICENSED
NICHOLAS W. KEVID
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PROFESSIONAL ENGINEER

A/G&E Structural Engenuity

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1140 N. GUN CLUB ROAD

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KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

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SNOW LOAD DIAGRAM

JOB 24464.0000
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S0-07

SNOW LOADING KEY

3

- 131 PSF+ BALANCED SNOW LOAD, X = 34'-3'
- 80 PSF+ BALANCED SNOW LOAD, X = 12'-8'
- 30 PSF+ BALANCED SNOW LOAD, X = 5'-0"

SNOW LOADING PRESENTED FOR INFORMATION ONLY. DRIFTS HAVE BEEN CONSIDERED IN JOIST AND JOIST GIRDER SELECTIONS SHOWN ON PLANS.

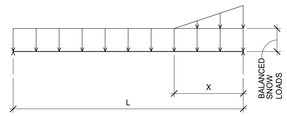
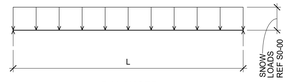


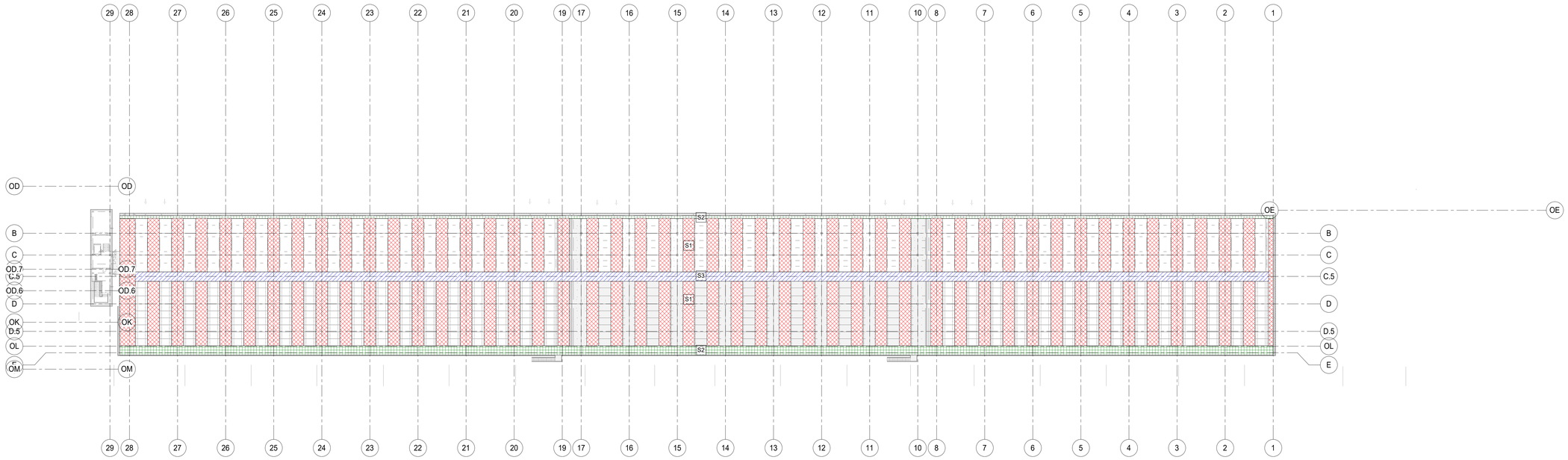
DIAGRAM 1



BALANCED

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



1 PLATFORM SNOW LOADING DIAGRAM
1" = 40'-0"

9/10/2025 5:01:21 PM S0-08 SNOW LOAD DIAGRAM

CORGAN
401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

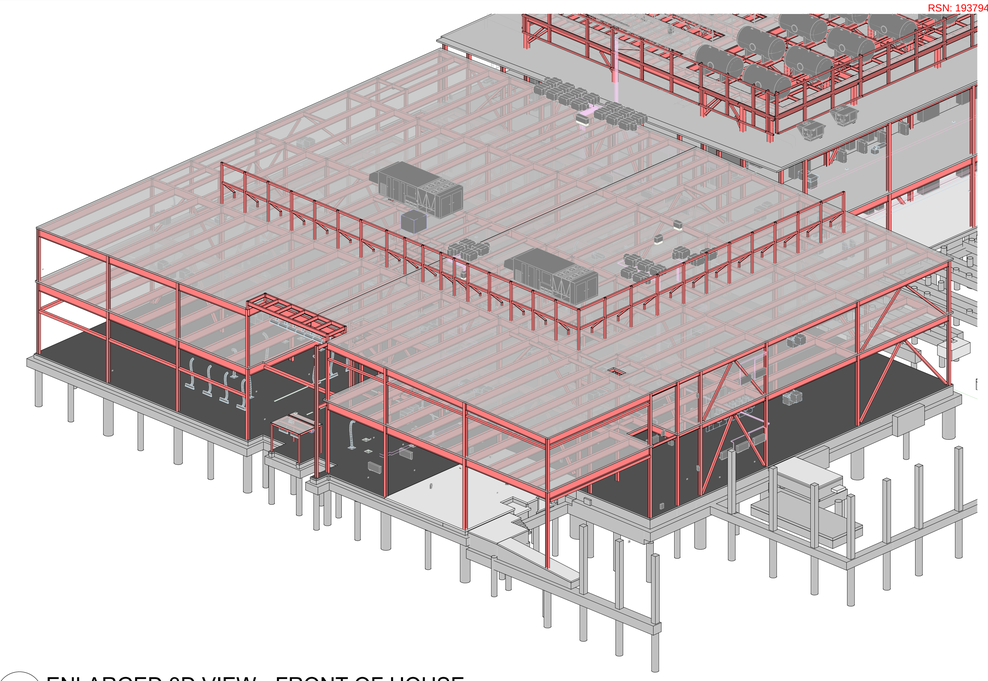
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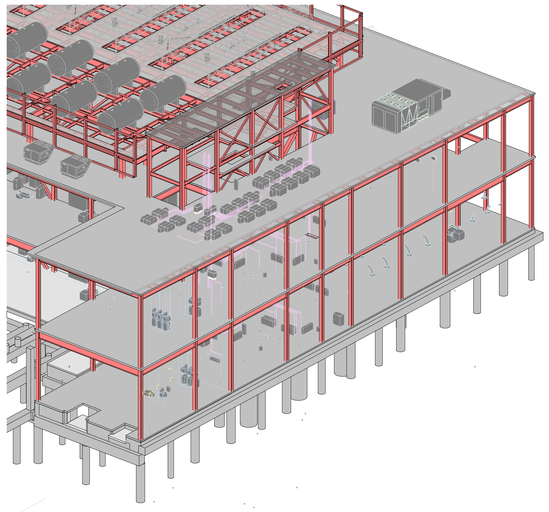
SNOW LOAD DIAGRAM

JOB 24464.0000
DATE 09.10.2025
SHEET

S0-08

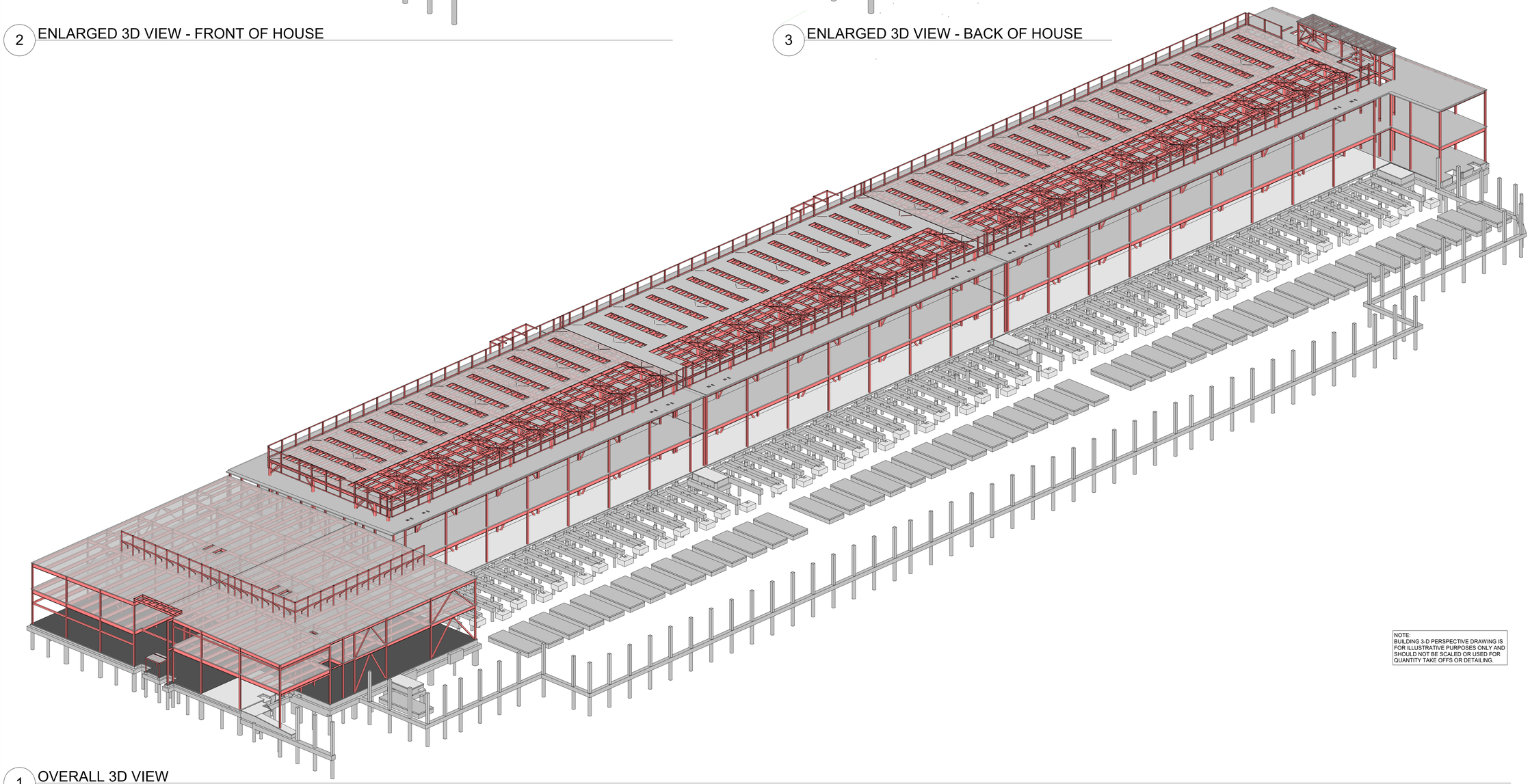


2 ENLARGED 3D VIEW - FRONT OF HOUSE



3 ENLARGED 3D VIEW - BACK OF HOUSE

This sheet of drawings has not been reviewed for code compliance.



NOTE:
BUILDING 3-D PERSPECTIVE DRAWING IS FOR ILLUSTRATIVE PURPOSES ONLY AND SHOULD NOT BE SCALED OR USED FOR QUANTITY TAKE OFFS OR DETAILING.

1 OVERALL 3D VIEW

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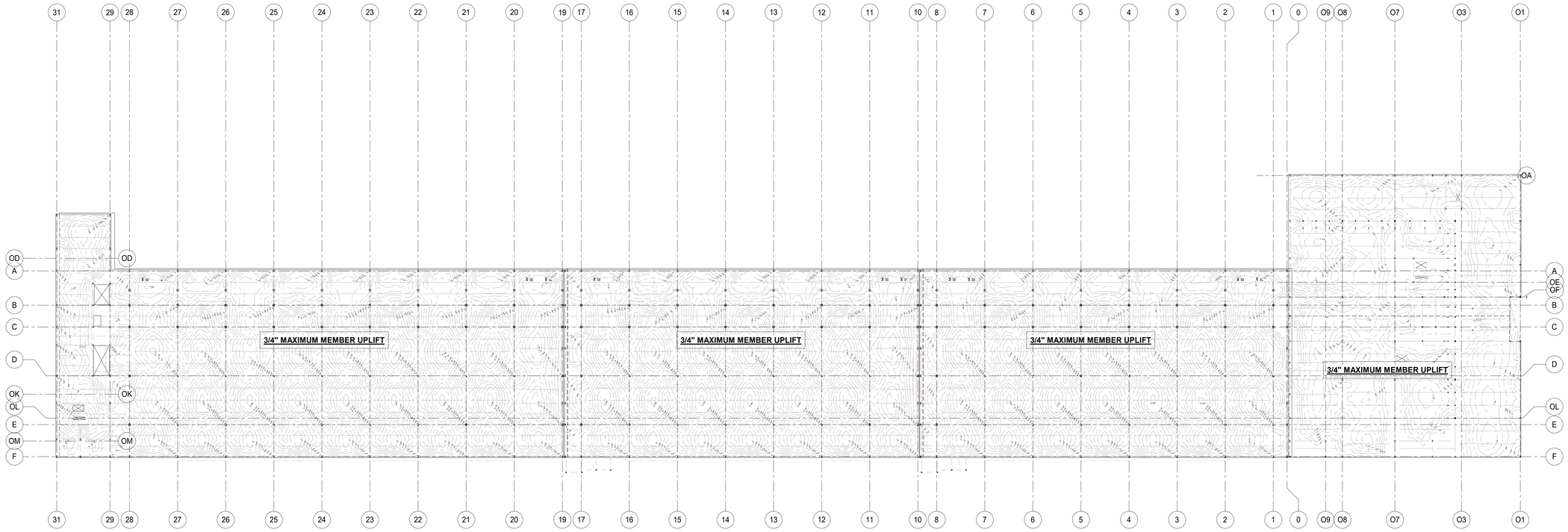
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OVERALL 3D VIEW

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DATE 09.10.2025
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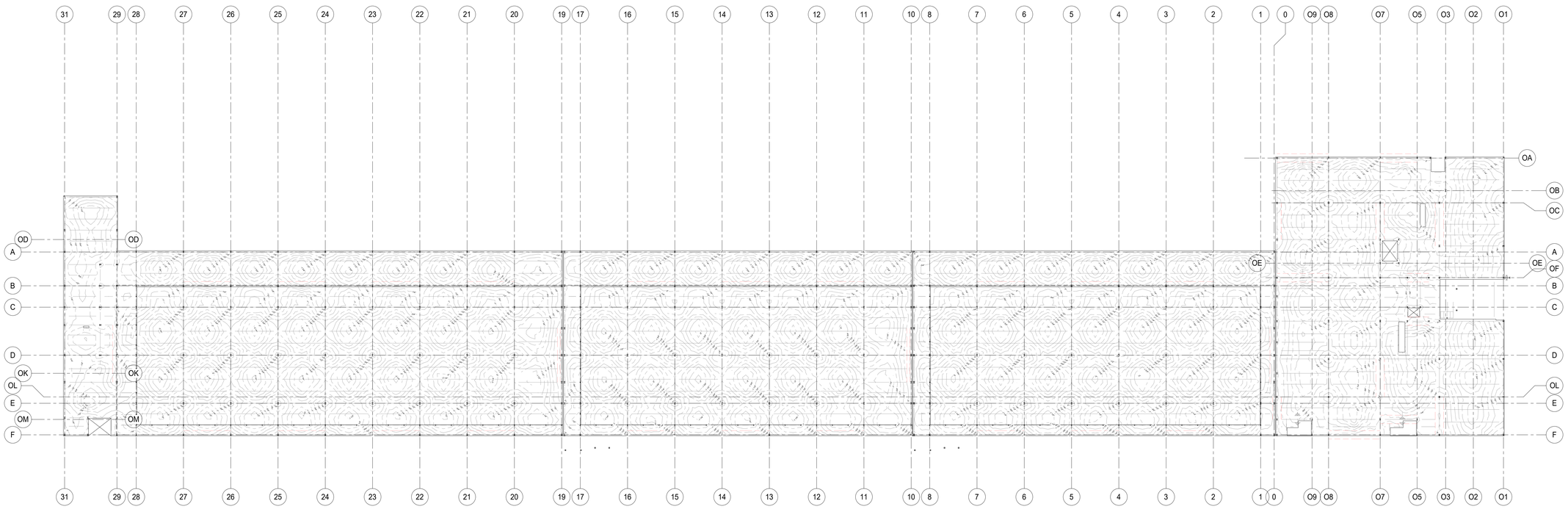
9/10/2025 5:01:33 PM S0-09 OVERALL 3D VIEW



2 ROOF DEFLECTION DIAGRAM
1" = 40'-0"

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City of Aurora Building Division
Reviewed for Code Compliance
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Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



1 SECOND FLOOR DEFLECTION DIAGRAM
1" = 40'-0"

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Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD

QTS

KEYPLAN

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DEFLECTION DIAGRAM

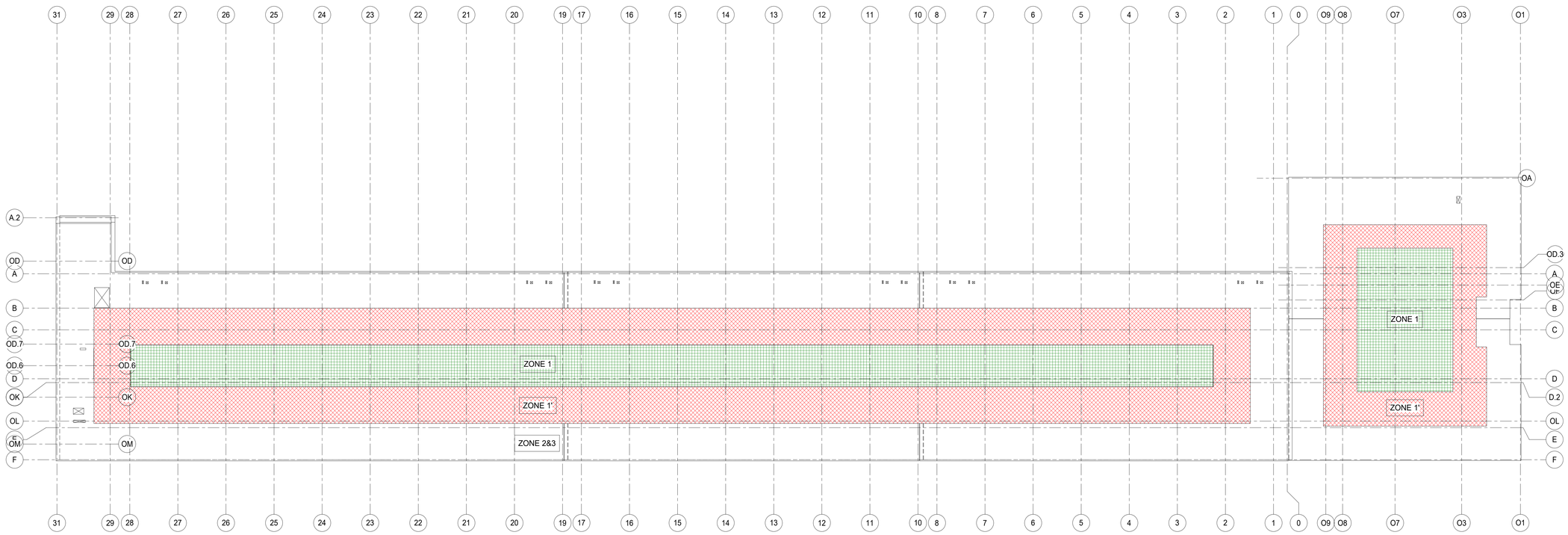
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S0-10

9/10/2025 5:01:48 PM S0-10 DEFLECTION DIAGRAM

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



1 ROOF WIND UPLIFT DIAGRAM
1" = 40'-0"

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T	R	P	M	K	H	F	D	B	

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ROOF WIND UPLIFT DIAGRAM

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DATE 09.10.2025
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S0-11

9/10/2025 5:01:49 PM S0-11 ROOF WIND UPLIFT DIAGRAM

SPECIAL INSPECTIONS

SPECIAL INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2021 IBC, CONTRACT DOCUMENTS AND APPROVED SUBMITTALS.

THESE ARE CONSIDERED MINIMUM SPECIAL INSPECTION REQUIREMENTS FOR STRUCTURAL ITEMS. CONTRACTOR TO FULFILL ADDITIONAL SPECIAL INSPECTIONS REQUIRED BY OWNER AND OTHER DISCIPLINES.

THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR DURING CONSTRUCTION ON THE FOLLOWING PHASES OF WORK:

STEEL CONSTRUCTION

THE SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STEEL CONSTRUCTION IN BUILDINGS, STRUCTURES, AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THIS SECTION.

EXCEPTION:
SPECIAL INSPECTIONS OF THE STEEL FABRICATION PROCESS SHALL NOT BE REQUIRED WHERE THE FABRICATOR DOES NOT PERFORM ANY WELDING, THERMAL CUTTING OR HEATING OPERATION OF ANY KIND AS PART OF THE FABRICATION PROCESS. IN SUCH CASES, THE FABRICATOR SHALL BE REQUIRED TO SUBMIT A DETAILED PROCEDURE FOR MATERIAL CONTROL THAT DEMONSTRATES THE FABRICATOR'S ABILITY TO MAINTAIN SUITABLE RECORDS AND PROCEDURES SUCH THAT, AT ANY TIME DURING THE FABRICATION PROCESS, THE MATERIAL SPECIFICATION AND GRADE FOR THE MAIN STRESS-CARRYING ELEMENTS ARE CAPABLE OF BEING DETERMINED. MILL TEST REPORTS SHALL BE IDENTIFIABLE TO THE MAIN STRESS-CARRYING ELEMENTS WHEN REQUIRED BY THE APPROVED CONSTRUCTION DOCUMENTS.

NOTE 1
THE SPECIAL INSPECTOR NEED NOT BE CONTINUOUSLY PRESENT DURING WELDING OF THE FOLLOWING ITEMS, PROVIDED THAT THE MATERIALS, WELDING PROCEDURES AND QUALIFICATIONS OF WELDERS ARE VERIFIED PRIOR TO THE START OF THE WORK; PERIODIC INSPECTIONS ARE MADE OF THE WORK IN PROGRESS; AND A VISUAL INSPECTION OF ALL WELDS IS MADE PRIOR TO COMPLETION OR PRIOR TO SHIPMENT OF SHOP WELDING:
- SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16 INCH (7.9 MM) IN SIZE.
- FLOOR AND ROOF DECK WELDING.
- WELDED STUDS WHERE NOT INSTALLED WITH AN AUTOMATICALLY TIMED STUD WELDING MACHINE IN ACCORDANCE WITH SECTION 7 OF AWS D1.1.
- WELDED SHEET STEEL FOR COLD-FORMED STEEL MEMBERS.
- WELDING OF STAIRS AND RAILING SYSTEMS.

FOR WELDED STUDS INSTALLED WITH AN AUTOMATICALLY TIMED STUD WELDING MACHINE AND IN ACCORDANCE WITH SECTION 7 OF AWS D1.1, THE SPECIAL INSPECTOR NEED NOT BE CONTINUOUSLY PRESENT DURING INSTALLATION OF WELDED STUDS SUBJECT TO THE FOLLOWING PROVISIONS:
THE SPECIAL INSPECTOR SHALL PERFORM A VISUAL INSPECTION OF ALL WELDED STUDS IN ACCORDANCE WITH SECTIONS 7 AND 7.8.1 OF AWS D1.1. VISUAL INSPECTION OF WELDED STUDS INSTALLED WITH AN AUTOMATICALLY TIMED STUD WELDING MACHINE MAY TAKE PLACE EITHER IN THE FABRICATION SHOP PRIOR TO COMPLETION OR PRIOR TO SHIPMENT, OR ON-SITE PRIOR TO COVERAGE.
THE FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

STRUCTURAL STEEL
SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES, AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.

EXCEPTION:
SPECIAL INSPECTION OF RAILING SYSTEMS COMPOSED OF STRUCTURAL STEEL ELEMENTS SHALL BE LIMITED TO WELDING INSPECTION OF WELDS AT THE BASE OF CANTILEVERED RAIL POSTS.

STRUCTURAL DETAILS

THE QAI SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF CONCRETE.
THE QAI SHALL INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS.

INSPECTION OF WELDING

WELDING INSPECTION TASKS SHALL BE IN ACCORDANCE WITH THE TABLES BELOW.

O - OBSERVE ON A RANDOM BASIS
P - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER
QC - QUALITY CONTROL
QCI - QUALITY CONTROL INSPECTION
QA - QUALITIY ASSURANCE
QAI - QUALITY ASSURANCE INSPECTION

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD*
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:			
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS AND SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	AISC 360, SECTION A3.3 AND APPLICABLE ASTM MATERIAL STANDARDS
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	X	-
2. INSPECTION OF HIGH-STRENGTH BOLTING:			
a. SNUG-TIGHT JOINTS.	-	X	AISC 360, SECTION M2.5 SEE ADDITIONAL REQUIREMENT IN AISC 360, SECTION N5. ITEM 6
b. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	-	X	
c. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	-	
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL:			
a. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	-	X	AISC 360, SECTION N2.1
b. FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	APPLICABLE ASTM MATERIAL STANDARDS
c. MANUFACTURER'S CERTIFIED TEST REPORTS.	-	X	-
4. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK:			
a. MANUFACTURER'S CERTIFIED TEST REPORTS.	-	X	-
5. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:			
a. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	AISC 360, SECTION A3.5 ANDAPPLICABLE AWS A5 DOCUMENTS
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	X	-
6. INSPECTION OF WELDING:			
a. STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:			
1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS.	X	-	AWS D1.1
2) MULTIPASS FILLET WELDS.	X	-	
3) SINGLE-PASS FILLET WELDS > 5/16".	X	-	
4) PLUG AND SLOT WELDS.	X	-	
5) SINGLE-PASS FILLET WELDS ≤ 5/16".	-	X	
6) FLOOR AND ROOF DECK WELDS.	-	X	AWS D1.3
b. REINFORCING STEEL:			
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706.	-	X	AWS D1.4, ACI 318 SECTION 4.2.2
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	X	-	
3) SHEAR REINFORCEMENT.	X	-	
4) OTHER REINFORCING STEEL.	-	X	
7. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE:			
a. DETAILS SUCH AS BRACING AND STIFFENING.	-	X	-
b. MEMBER LOCATION.	-	X	
c. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	-	X	

WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

SPECIAL INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2019 OSSC, CONTRACT DOCUMENTS AND APPROVED SUBMITTALS. REFER TO TABLES 1 THROUGH 2A FOR SPECIAL INSPECTION AND TABLES 6 FOR TESTING REQUIREMENTS.

INSPECTION OF OTHER STEEL ELEMENTS

OTHER STEEL ELEMENTS INSPECTION TASKS SHALL BE IN ACCORDANCE WITH THE TABLE BELOW.

O - OBSERVE ON A RANDOM BASIS
P - PERFORM THESE TASKS FOR EACH ITEM OR ELEMENT

INSPECTION OF OTHER STEEL ELEMENTS	QC	QA
1. RBS REQUIREMENTS, IF APPLICABLE - CONTOUR AND FINISH - DIMENSIONAL TOLERANCES	P	P
2. PROTECTED ZONE - NO HOLES AND UNAPPROVED ATTACHMENTS MADE BY FABRICATOR OR ERECTOR, AS APPLICABLE	P	P

INSPECTION OF COMPOSITE CONSTRUCTION

COMPOSITE CONSTRUCTION INSPECTION TASKS SHALL BE IN ACCORDANCE WITH THE TABLE BELOW.

O - OBSERVE ON A RANDOM BASIS
P - PERFORM THESE TASKS FOR EACH ITEM OR ELEMENT
QC - QUALITY CONTROL
QCI - QUALITY CONTROL INSPECTION
QA - QUALITY ASSURANCE
QAI - QUALITY ASSURANCE INSPECTION

INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT	QC	QA
1. PLACEMENT AND INSTALLATION OF STEEL DECK	P	P
2. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	P	P
3. DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS	P	P

INSPECTION OF COLD-FORMED STEEL DECK

SPECIAL INSPECTIONS AND QUALIFICATION OF WELDING SPECIAL INSPECTORS FOR COLD-FORMED STEEL FLOOR AND ROOF DECK SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF SDI QA/QC. COLD-FORMED STEEL DECK INSPECTION TASKS SHALL BE IN ACCORDANCE WITH THE TABLES BELOW.

O - OBSERVE ON A RANDOM BASIS
P - PERFORM THESE TASKS FOR EACH ITEM OR ELEMENT

INSPECTION TASKS PRIOR TO DECK PLACEMENT	QC	QA
1. VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS	P	P
2. DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	P	P

INSPECTION TASKS AFTER DECK PLACEMENT	QC	QA
1. VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	P	P
2. VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	-	P
3. DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES	P	P

INSPECTION TASKS PRIOR TO WELDING	QC	QA
1. WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	O	O
2. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	O	O
3. MATERIAL IDENTIFICATION (TYPE/GRADE)	O	O
4. CHECK WELDING EQUIPMENT	O	O

INSPECTION TASKS DURING WELDING	QC	QA
1. USE OF QUALIFIED WELDERS	O	O
2. CONTROL AND HANDLING OF WELDING CONSUMABLES	O	O
3. ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE)	O	O
4. WPS FOLLOWED	O	O

INSPECTION TASKS AFTER WELDING	QC	QA
1. VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS.	P	P
2. WELDS MEET VISUAL ACCEPTANCE CRITERIA	P	P
3. VERIFY REPAIR ACTIVITIES	P	P
4. DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	P	P

INSPECTION TASKS PRIOR TO MECHANICAL FASTENING	QC	QA
1. MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	O	O
2. PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	O	O
3. PROPER STORAGE FOR MECHANICAL FASTENERS	O	O

INSPECTION TASKS DURING MECHANICAL FASTENING	QC	QA
1. FASTENERS ARE POSITIONED AS REQUIRED	O	O
2. FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	O	O

INSPECTION TASKS AFTER MECHANICAL FASTENING	QC	QA
1. CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS	P	P
2. CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS	P	P
3. CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS	P	P
4. VERIFY REPAIR ACTIVITIES	P	P
5. DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	P	P

CONCRETE CONSTRUCTION

THE SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION SHALL BE AS REQUIRED BELOW. REFER TO OSSC 2021 IBC FOR REFERENCE STANDARDS.

WELDING OF REINFORCING BARS

SPECIAL INSPECTIONS OF WELDING AND QUALIFICATIONS OF SPECIAL INSPECTORS FOR REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.4 FOR SPECIAL INSPECTION AND OF AWS D1.4 FOR SPECIAL INSPECTOR QUALIFICATION.

MATERIAL TESTS

IN THE ABSENCE OF SUFFICIENT DATA OR DOCUMENTATION PROVIDING EVIDENCE OF CONFORMANCE TO QUALITY STANDARDS FOR MATERIALS IN CHAPTERS 19 AND 20 OF ACI 318, TESTING OF MATERIALS IS REQUIRED IN ACCORDANCE WITH THE APPROPRIATE STANDARDS AND CRITERIA FOR THE MATERIAL IN CHAPTERS 19 AND 20 OF ACI 318.

CONCRETE CONSTRUCTION	CONTINUOUS	PERIODIC
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT		X
2. REINFORCING BAR WELDING:		
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;		X
b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		X
c. INSPECT ALL OTHER WELDS.	X	
3. INSPECT ANCHORS CAST IN CONCRETE.		X
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.		
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X	
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.		X
5. VERIFY USE OF REQUIRED DESIGN MIX.		X
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.		X
9. INSPECT PRESTRESSED CONCRETE FOR:		
a. APPLICATION OF PRESTRESSING FORCES; AND	X	
b. GROUTING OF BONDED PRESTRESSING TENDONS	X	
10. INSPECTION ERECTION OF PRECAST CONCRETE MEMBERS.		X
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.		X
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		X

EXCEPTIONS:
1. ISOLATED SPREAD FOOTINGS OF BUILDINGS 3 STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK.
2. CONTINUOUS FOOTINGS SUPPORTING WALLS OF BUILDINGS 3 STORIES OF LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE:
   a. THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION
   b. THE FOOTINGS ARE DESIGNED IN ACCORDANCE WITH TABLE 1809.7.
   c. THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH F'c <= 2500 PSI, REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS OR USED IN THE FOOTING CONSTRUCTION.
3. NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE, WHERE THE EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI.
4. CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.2.
5. CONCRETE PATIOS, DRIVEWAYS AND SIDEWALKS, ON GRADE.

INSPECTION OF SOILS

SOILS INSPECTION TASKS SHALL BE IN ACCORDANCE WITH SECTION 1705.6 OF 2021 IBC AND THE TABLE BELOW.

SOILS	CONTINUOUS	PERIODIC
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.		X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		X

INSPECTION OF CAST-IN-PLACE DEEP FOUNDATIONS

CAST-IN-PLACE DEEP FOUNDATIONS TASKS SHALL BE IN ACCORDANCE WITH SECTION 1705.8 OF 2021 IBC AND THE TABLE BELOW.

SOILS	CONTINUOUS	PERIODIC
1. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE ACCURATE RECORDS FOR EACH ELEMENT.	X	
2. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGHT, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END-BEARING STRATA CAPACITY, RECORD CONCRETE OR GROUT VOLUMES.	X	
3. FOR CONCRETE ELEMENTS, PERFORM TESTS AND ADDITIONAL SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3.	IN ACCORDANCE WITH SECTION 1705.3	

EXPANSION ANCHORS

- DURING INSTALLATION OF EXPANSION BOLTS TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, DRILL-BIT TYPE, HOLE DIMENSIONS, HOLE CLEANING PROCEDURE, ANCHOR SPACING, EDGE DISTANCES, CONCRETE MEMBER THICKNESS, ANCHOR EMBEDMENT, TIGHTENING TORQUE AND ADHERENCE TO THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS.

EPOXY ANCHORS AND DOWELS

- DURING INSTALLATION OF EPOXY ANCHORS AND DOWELS TO VERIFY ANCHOR/DOWEL TYPE, ANCHOR/DOWEL DIMENSIONS, CONCRETE TYPE, ADHESIVE IDENTIFICATION AND EXPIRATION DATE, DRILL-BIT TYPE, HOLE DIMENSIONS, HOLE CLEANING PROCEDURE, ANCHOR/DOWEL SPACING, EDGE DISTANCES, CONCRETE MEMBER THICKNESS, ANCHOR/DOWEL EMBEDMENT, TIGHTENING TORQUE AND ADHERENCE TO THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS.
- SPECIAL INSPECTOR SHALL VERIFY INITIAL INSTALLATION OF EACH TYPE AND SIZE OF ANCHOR/DOWEL BY CONSTRUCTION PERSONNEL ON SITE.
- ANY CHANGE IN ANCHOR/DOWEL PRODUCT BEING INSTALLED OR PERSONNEL PERFORMING THE INSTALLATION MUST REQUIRE AN INITIAL INSPECTION.
- FOR ONGOING INSTALLATIONS OVER A PERIOD OF TIME, SPECIAL INSPECTOR SHALL PERFORM REGULAR INSPECTIONS TO CONFIRM CORRECT HANDLING AND INSTALLATION OF PRODUCT.

NON-SHRINK GROUT

- DURING TIME FRESH GROUT IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS AND AS RECOMMENDED BY MANUFACTURER.
- INSPECTION OF GROUT PLACEMENT FOR PROPER APPLICATION TECHNIQUES AND TO VERIFY FULL BEARING BELOW BASE PLATE.

BOLTS INSTALLED IN CONCRETE

- DURING INSTALLATION OF BOLTS AND PLACING OF CONCRETE AROUND SUCH BOLTS.

WIND-RESISTING COMPONENTS

PERIODIC SPECIAL INSPECTION IS REQUIRED FOR FASTENING OF THE FOLLOWING SYSTEMS AND COMPONENTS:
1. ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS.
2. EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING.

ARCHITECTURAL, MEP COMPONENTS

ARCHITECTURAL, MECHANICAL AND ELECTRICAL COMPONENTS REQUIRING SPECIAL INSPECTIONS AND TESTING PER THE BUILDING CODE ARE NOT LISTED HERE. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND SPECIFICATIONS FOR SPECIAL INSPECTION REQUIREMENTS FOR THESE COMPONENTS.

SPECIAL INSPECTOR

THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE HIS COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF A PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR

- THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.
- THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE ENGINEER OR ARCHITECT OF RECORD, AND OTHER DESIGNATED PERSONS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE PROPER DESIGN AUTHORITY.
- THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

CONTRACTOR RESPONSIBILITY

EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND OR SEISMIC FORCE RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND OR SEISMIC COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:
1. ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.
2. ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
3. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS, AND
4. IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

STRUCTURAL OBSERVATION REQUIREMENT & STATEMENT

1. PER SECTION 1704.6, THE OWNER OR THE OWNER AUTHORIZED AGENT SHALL EMPLOY THE REGISTERED DESIGN PROFESSIONAL TO PERFORM STRUCTURAL OBSERVATIONS. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS OR THE SPECIAL INSPECTIONS.
2. THE STRUCTURAL OBSERVATION WILL BE PERFORMED IN ACCORDANCE TO SECTIONS 1704.6.1 AND 1704.6.2 TO REVIEW THE CONSTRUCTION OF THE LATERAL RESISTING SYSTEM. THE STRUCTURAL OBSERVATION WILL BE LIMITED TO VISUAL OBSERVATIONS OF THE CONSTRUCTION AND TO VERIFY GENERAL CONFORMANCE TO THE APPROVED STRUCTURAL DRAWINGS AND SPECIFICATIONS AND DOES NOT SUBSTITUTE FOR ANY SPECIAL INSPECTIONS REQUIRED BY CODE AND/OR THE BUILDING DEPARTMENT.
3. AFTER EACH STRUCTURAL OBSERVATION, A REPORT WILL BE SUBMITTED TO THE OWNER AND BUILDING OFFICIALS WHICH INCLUDES A SUMMARY OF THE ELEMENTS/ CONNECTIONS OBSERVED AND ANY DEFICIENCIES IDENTIFIED.
4. AT THE CONCLUSION OF THE WORK INCLUDED IN PERMIT, THE STRUCTURAL OBSERVER SHALL SUBMIT A WRITTEN STATEMENT TO THE OWNER AND BUILDING OFFICIALS THAT THE SITE VISITS HAVE BEEN MADE AND IDENTIFY ANY REPORTED DEFICIENCIES THAT, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN SOLVED.
5. STRUCTURAL OBSERVATIONS SHALL BE PERFORMED AT THE FOLLOWING STAGES OF CONSTRUCTION AS A MINIMUM. GENERAL CONTRACTOR TO PROVIDE MINIMUM 3 DAYS NOTIFICATIONS TO THE STRUCTURAL ENGINEER PRIOR TO THESE STAGES.
   a. PRIOR TO PLACEMENT OF CONCRETE AND AFTER PLACEMENT OF STEEL REINFORCEMENT FOR THE FIRST FOUNDATION POUR (I.E., PILES, PILE CAPS, GRADE BEAMS, PEDESTALS, AND SLABS)
   b. DURING STRUCTURAL STEEL ERECTION AT ALL FLOORS.
   c. AFTER ERECTION OF THE FIRST SEISMIC RESISTING SYSTEM ELEMENTS (I.E., BRACED FRAMES, COLLECTORS AND THEIR CONNECTIONS)
   d. PRIOR TO THE PLACEMENT OF CONCRETE IN METAL DECK (AND AFTER PLACEMENT OF METAL DECK, REINFORCEMENTS AND SHEAR STUDS).
   e. FINAL PUNCH WALK AFTER COMPLETION OF ALL STRUCTURAL COMPONENTS AND PRIOR TO ARCHITECTURAL FINISHES.

Code violations that are found during inspection are required to be corrected. Permit issuance does not grant approval of a code violation.

Structural plan review is limited to a general survey for code compliance. No review is implied nor was undertaken to verify structural adequacy.

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

SPECIAL INSPECTIONS REQUIREMENTS - 2021 IBC 1704
Prior to the start of construction, the owner or owner's authorized agent shall employ one or more approved agencies to provide special inspections and tests during construction as required by the structural engineer in their statement of special inspections. The special inspector's qualifications and written reports shall be reviewed by the engineer of record and made available to the building inspector on the job site. The building division will only issue a certificate of occupancy after all special inspection reports, and the final special inspection report, have been submitted and accepted.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 STATEMENT OF SPECIAL INSPECTIONS CONT.
12" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
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SYSKA HENNESSY GROUP

DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
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REVISIONS

A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED DOUGLAS W. HAYES 65236 PROFESSIONAL ENGINEER

AG&E Structural Engeneuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

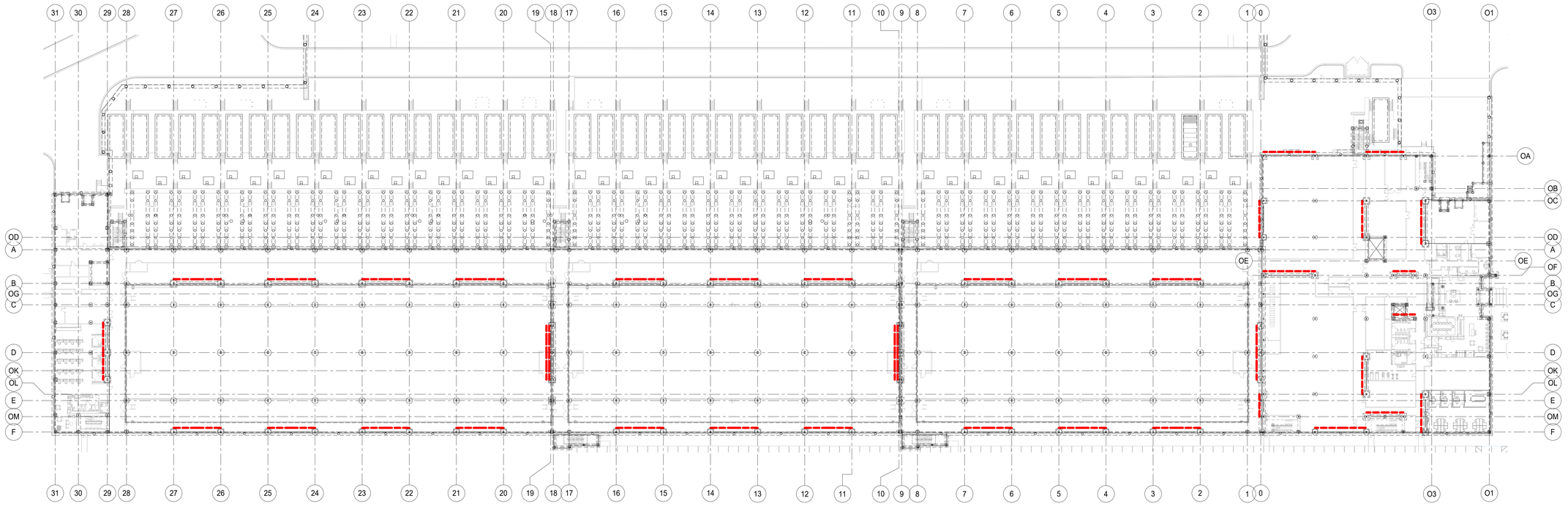
GENERAL NOTES

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DATE 09.10.2025
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9/10/2025 5:01:50 PM S0-12 GENERAL NOTES

ONLY HIGHLITED OR CLOUDED REVISIONS
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1 OVERALL FOUNDATION PLAN
1" = 40'-0"

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COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD

KEYPLAN

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T	R	P	M	K	H	F	D	B	

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OVERALL FOUNDATION PLAN

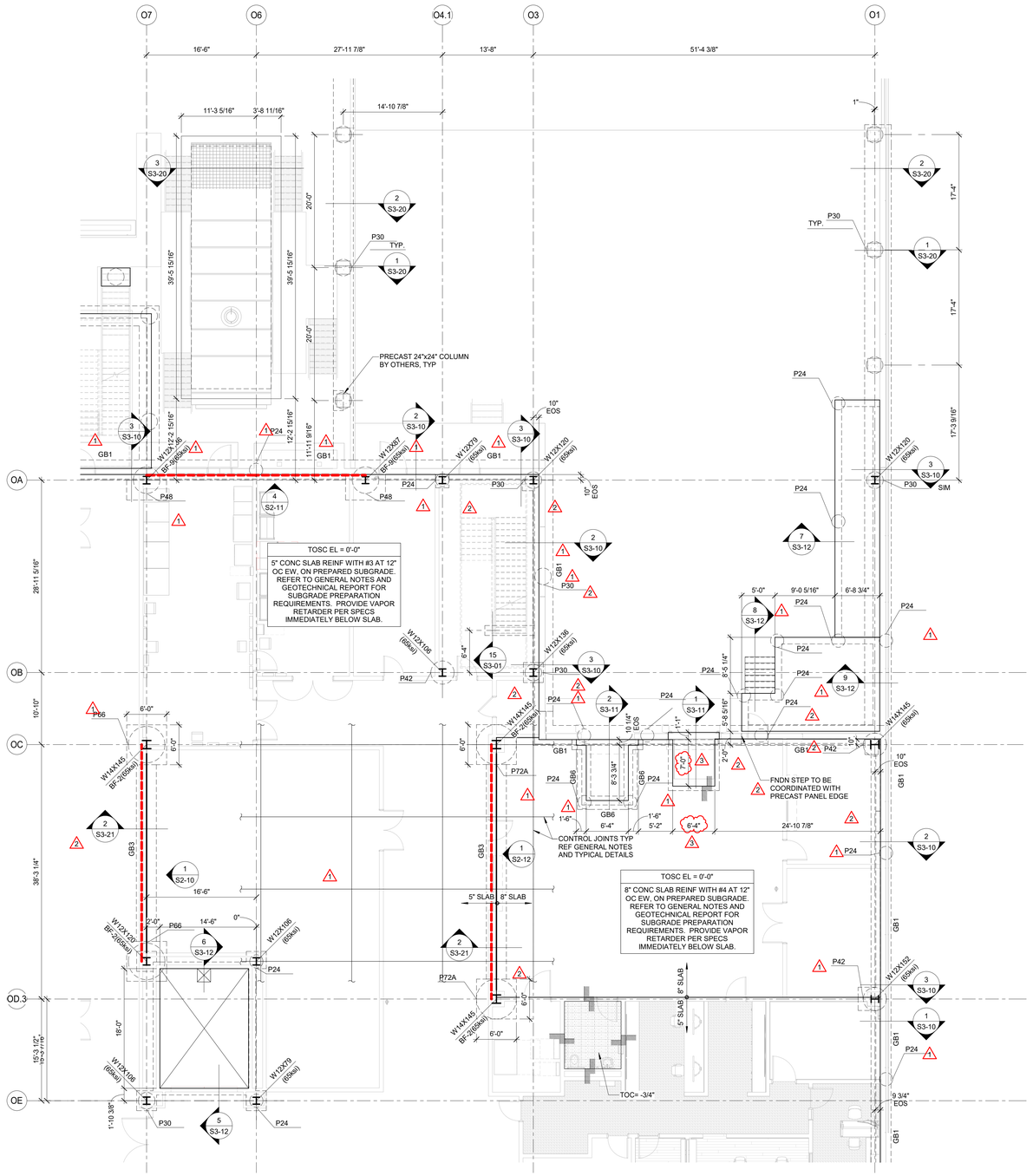
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DATE 09.10.2025
SHEET

S1-10

9/10/2025 5:01:57 PM S1-10 OVERALL FOUNDATION PLAN

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
2. VERIFY ALL WALL OPENING DIMENSIONS AND LOCATIONS WITH ARCH DRAWINGS.
3. DEVIATIONS FROM CONST JOINT/CONTROL JOINT PATTERN SHOWN MUST BE APPROVED BY THE ENGINEER OF RECORD.
4. REFER TO SHEET S3-00 FOR PIER SCHEDULE.
5. REFER TO SHEET S5-00 FOR COLUMN AND BASE PLATE SCHEDULE.
6. REFER TO S2-00 FOR BRACE FRAME ELEVATIONS.
7. PIERS WITHOUT CENTERLINES SHOWN ON PLANS, SECTIONS AND/OR DETAILS SHALL BE LOCATED AS FOLLOWS:
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9. VERIFY ALL PAD OPENING DIMENSIONS AND LOCATIONS WITH ARCH/MEP DRAWINGS. VERIFY ALL PAD DIMENSIONS WITH EQUIPMENT MANUFACTURER PRIOR TO FABRICATING REINFORCING AND INSTALLATION.
10. METAL STAIR AND CANOPY BY STAIR MANUF.



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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: **Matt A.**
Date: **Nov 06, 2025**
2021 INTERNATIONAL CODES & 2023 NEC

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SYSKA HENNESSY GROUP

DESIGN CONSULTING LLC

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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KNYID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
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FOUNDATION PLAN - SEGMENT A

JOB 24464.0000
DATE 09.10.2025
SHEET

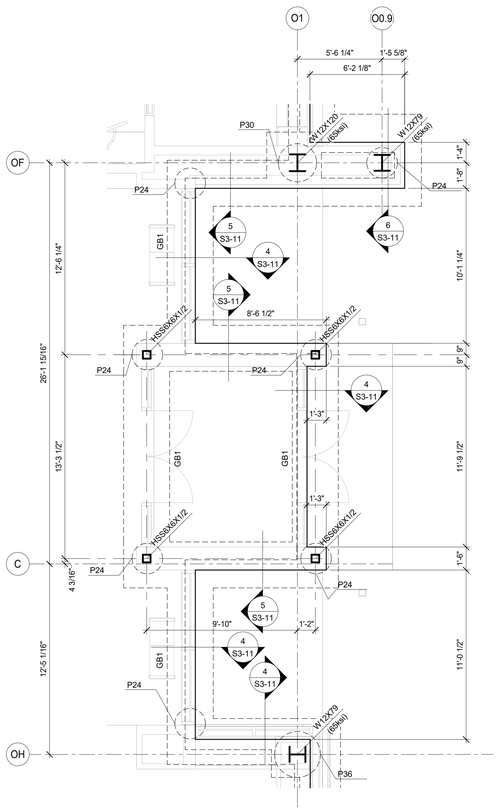
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1 FOUNDATION PLAN - SEGMENT A
1/8" = 1'-0"

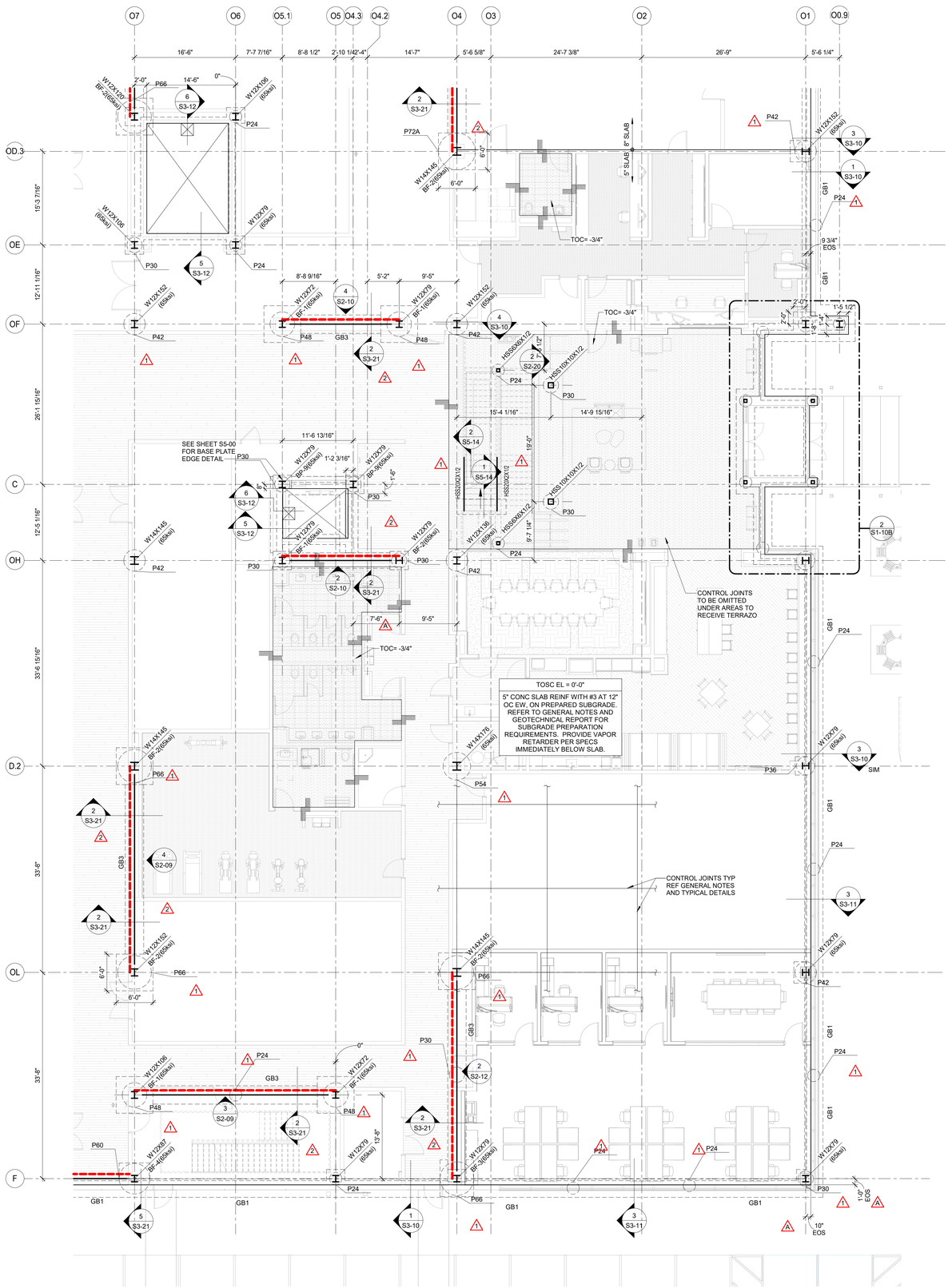
9/10/2025 5:02:00 PM S1-10A FOUNDATION PLAN - SEGMENT A

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
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9. VERIFY ALL PAD OPENING DIMENSIONS AND LOCATIONS WITH ARCH/MEP DRAWINGS. VERIFY ALL PAD DIMENSIONS WITH EQUIPMENT MANUFACTURER PRIOR TO FABRICATING REINFORCING AND INSTALLATION.
10. METAL STAIR AND CANOPY BY STAIR MANUF.



2 VESTIBULE FOUNDATION PLAN
1/4" = 1'-0"



1 FOUNDATION PLAN - SEGMENT B
1/8" = 1'-0"

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REVISIONS

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2	05.30.25	ISSUE FOR CONSTRUCTION
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEVIO
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

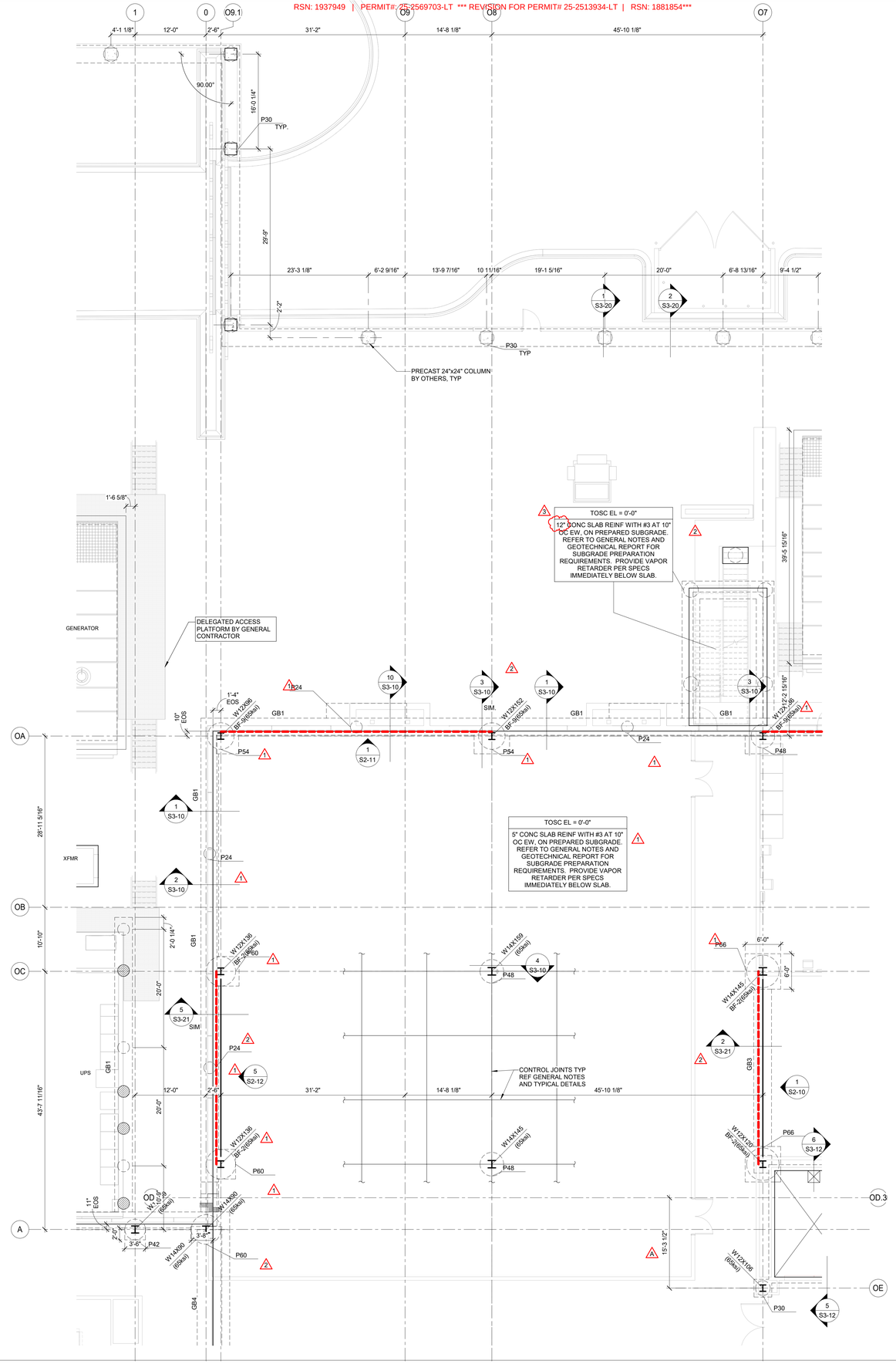
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FOUNDATION PLAN - SEGMENT B

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10B



1 FOUNDATION PLAN - SEGMENT C
1/8" = 1'-0"

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
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City of Aurora Building Division
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A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
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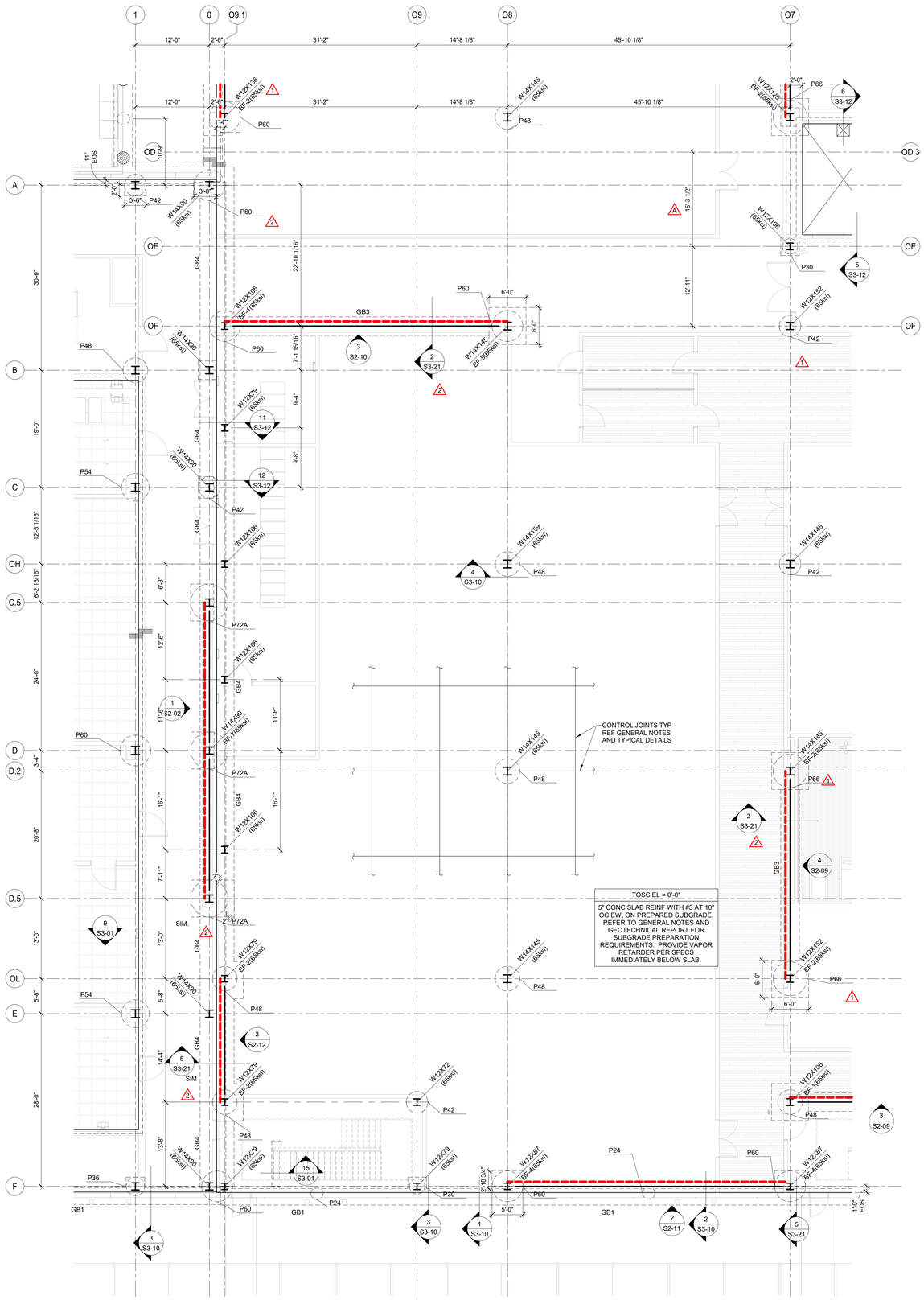
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FOUNDATION PLAN - SEGMENT C

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10C

9/10/2025 5:02:07 PM S1-10C FOUNDATION PLAN - SEGMENT C



SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
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10. METAL STAIR AND CANOPY BY STAIR MANUF.

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 FOUNDATION PLAN - SEGMENT D
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
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6	09.10.25	IFC.1
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△	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

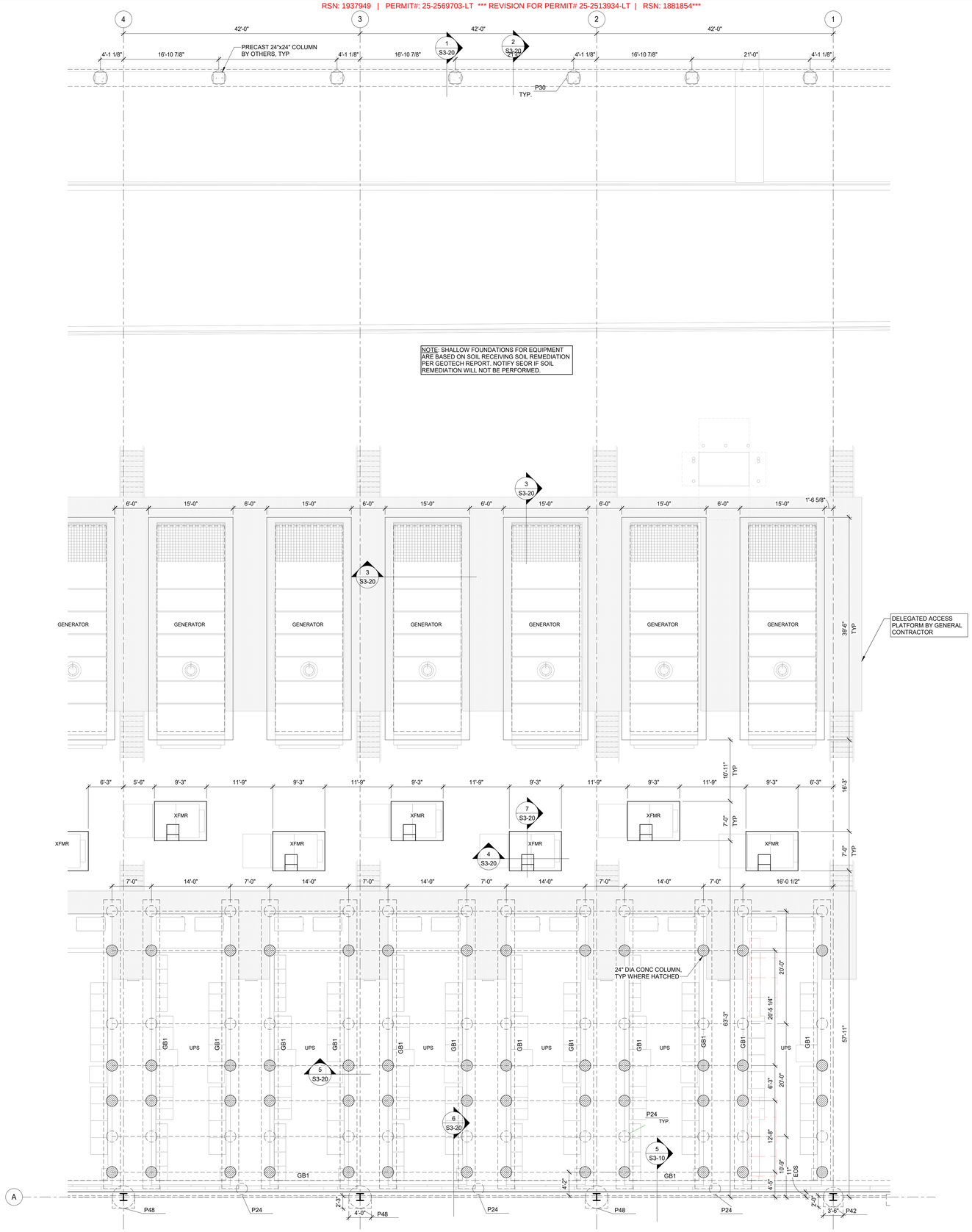
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FOUNDATION PLAN - SEGMENT D

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10D

9/10/2025 5:02:10 PM S1-10D FOUNDATION PLAN - SEGMENT D



1 FOUNDATION PLAN - SEGMENT E
1/8" = 1'-0"

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
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REVISIONS		
1	03.28.25	ADDENDUM 1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

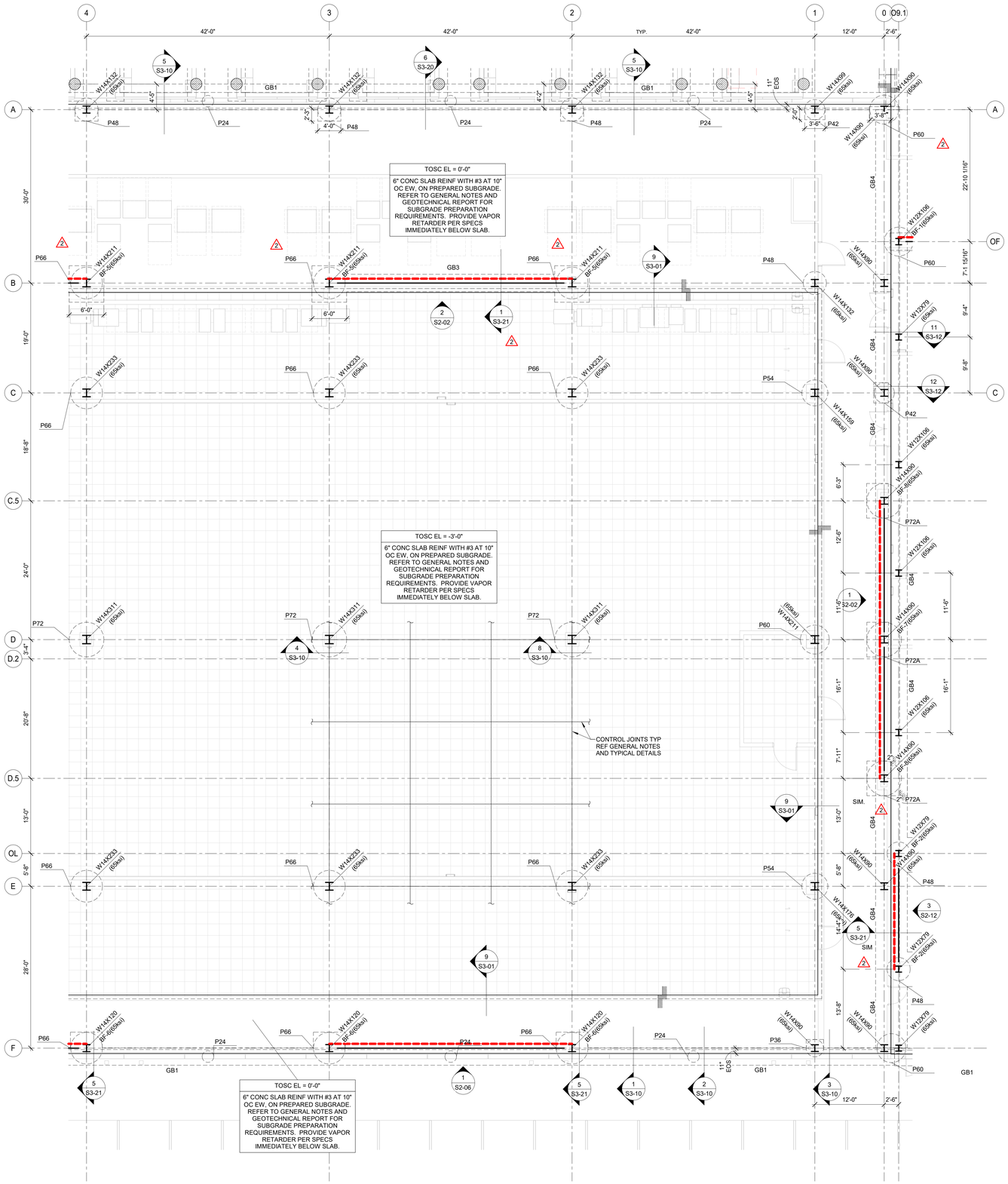
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FOUNDATION PLAN - SEGMENT E

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10E

9/10/2025 5:02:13 PM S1-10E FOUNDATION PLAN - SEGMENT E



SHEET NOTES

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1 FOUNDATION PLAN - SEGMENT F
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

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REVISIONS

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COLORADO LICENSED
NICHOLAS W. NEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

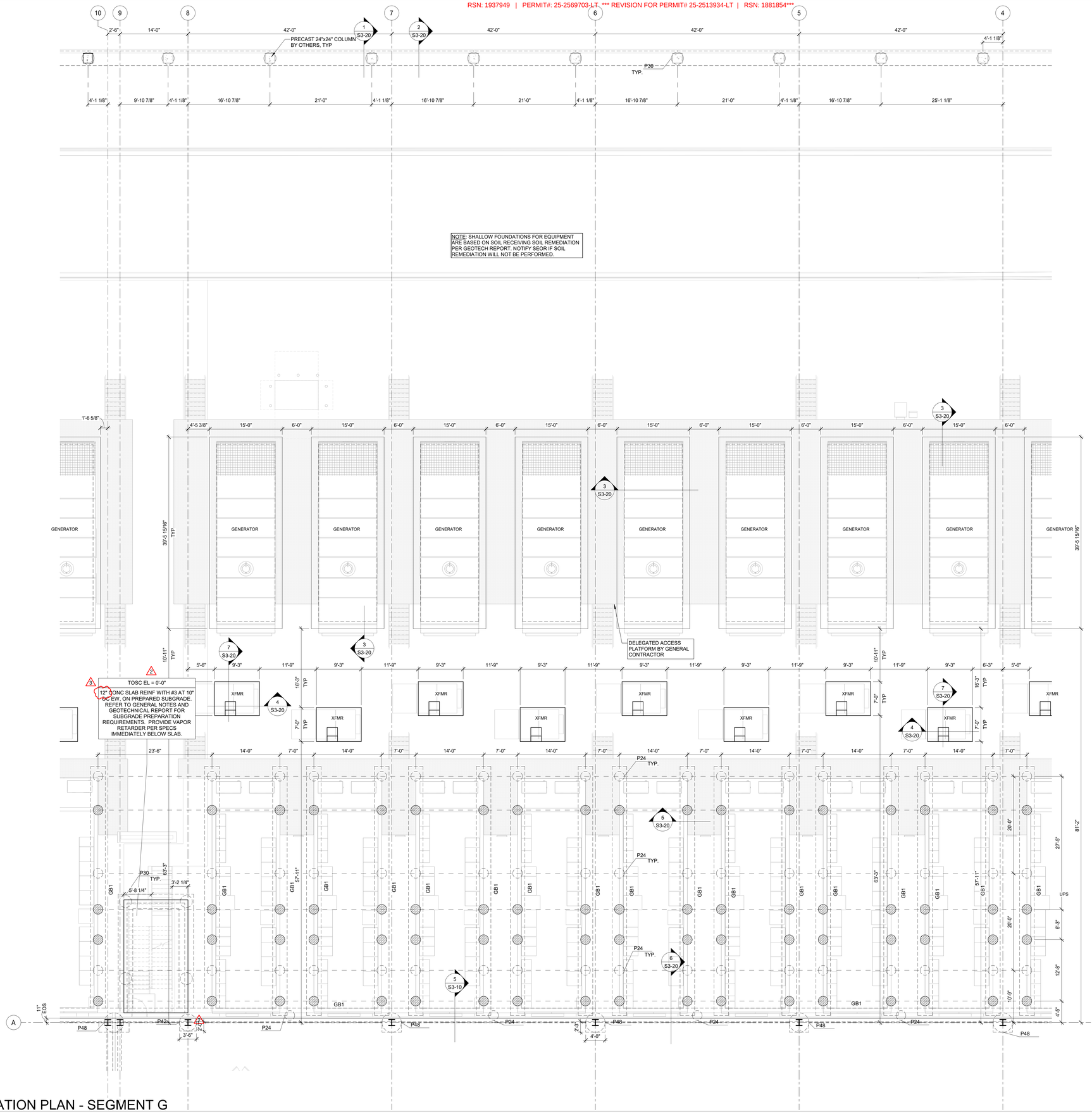
S Q N L J G E C A
T R P M K H F D B
N

FOUNDATION PLAN - SEGMENT F

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10F

9/10/2025 5:02:16 PM S1-10F FOUNDATION PLAN - SEGMENT F



1 FOUNDATION PLAN - SEGMENT G
1/8" = 1'-0"

SHEET NOTES

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AG/E

SYSKA HENNESSY GROUP

TCD DESIGN CONSULTING LLC

ISSUES		
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COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

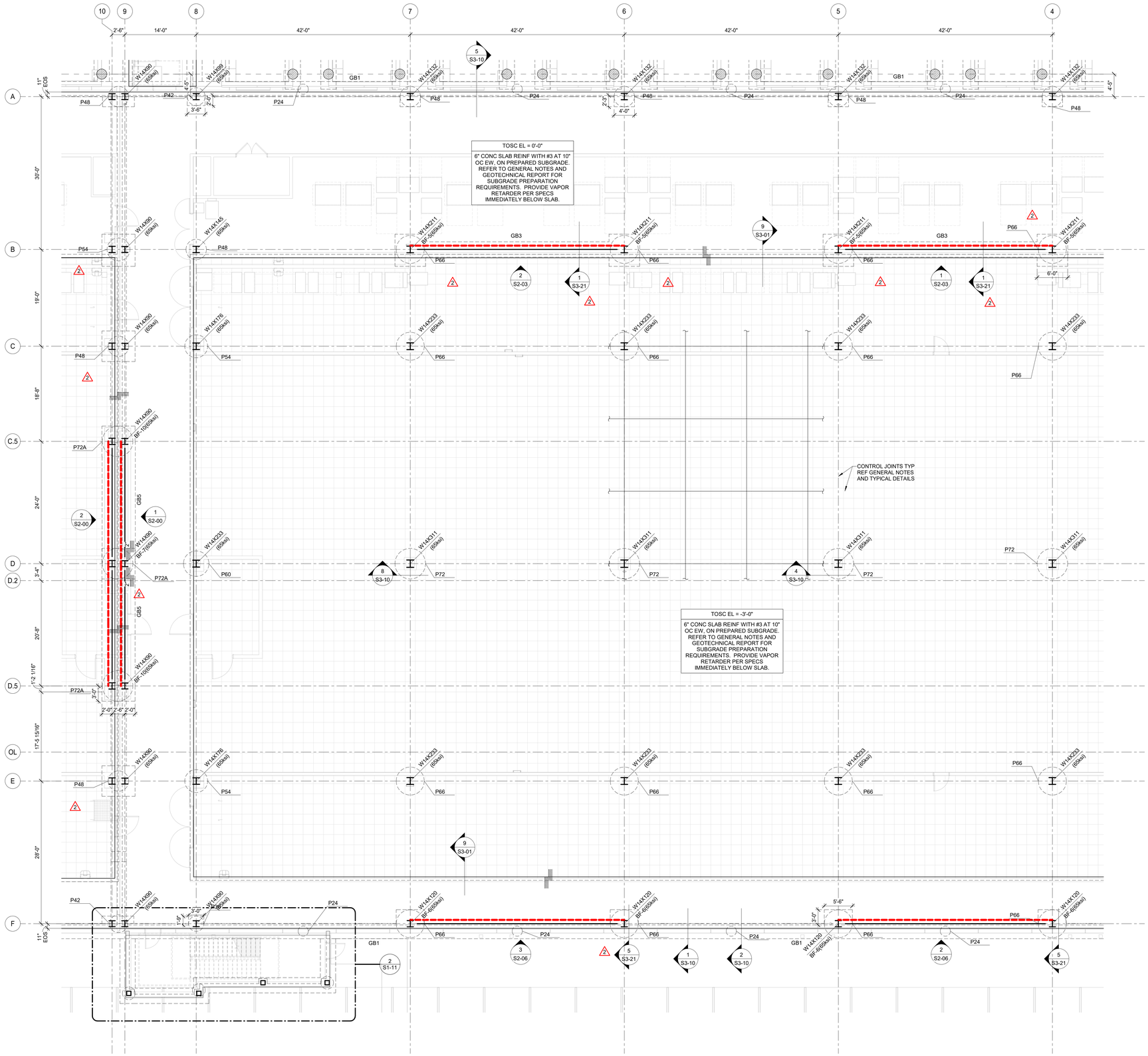
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FOUNDATION PLAN - SEGMENT G

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10G

9/10/2025 5:02:19 PM S1-10G FOUNDATION PLAN - SEGMENT G



1 FOUNDATION PLAN - SEGMENT H
1/8" = 1'-0"

SHEET NOTES

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REVISIONS		
1	03.28.25	ADDENDUM 1
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COLORADO LICENSED
NICHOLAS W. HAYES
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

KEYPLAN

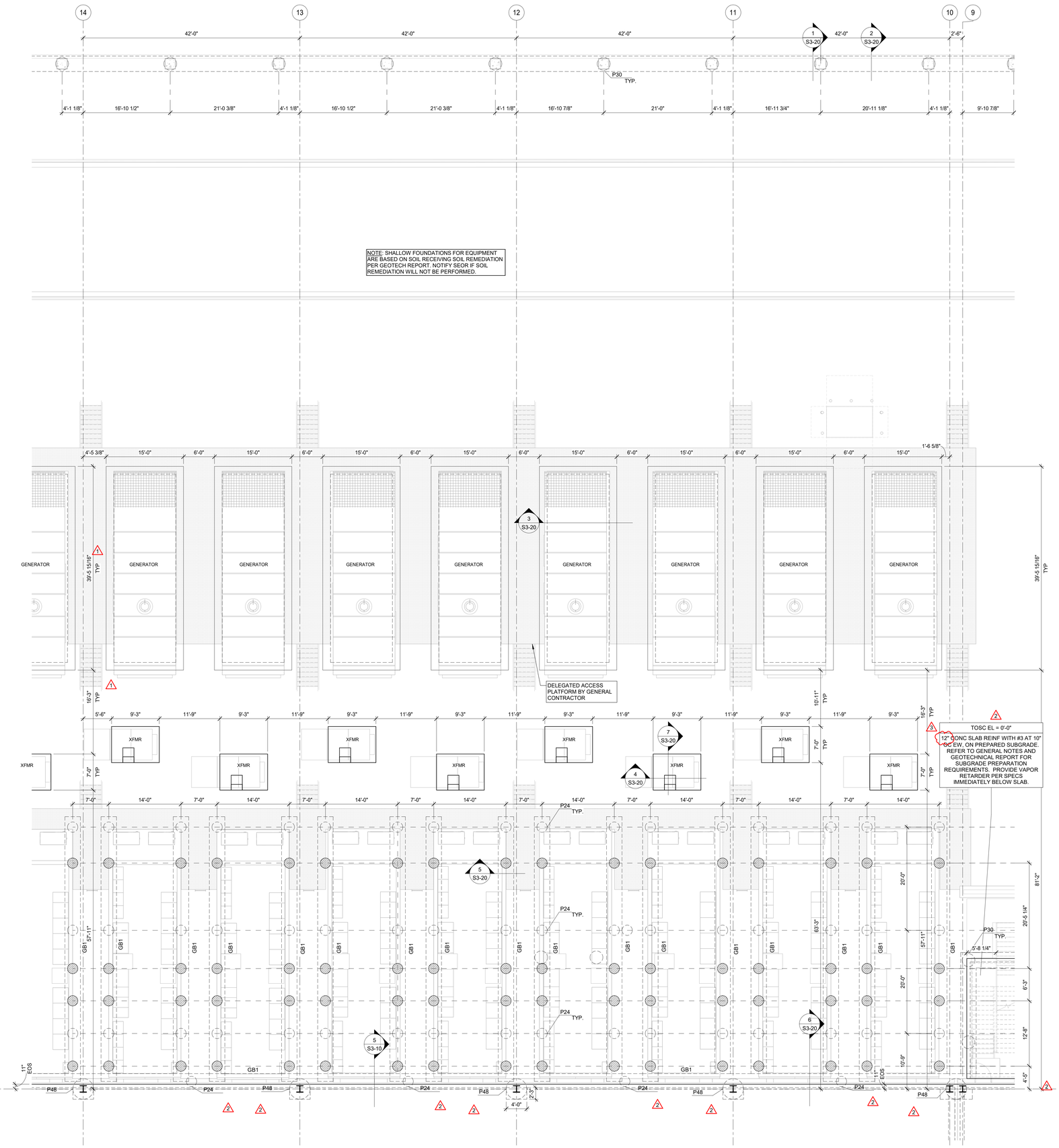
S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	N

FOUNDATION PLAN - SEGMENT H

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10H

9/10/2025 5:02:22 PM S1-10H FOUNDATION PLAN - SEGMENT H



1 FOUNDATION PLAN - SEGMENT J
1/8" = 1'-0"

SHEET NOTES

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COLORADO LICENSED
NICHOLAS W. KEVID
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AG&E Structural Engenuity

Date of issue:
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QTS DEN1 DC2
1140 N. GUN CLUB ROAD
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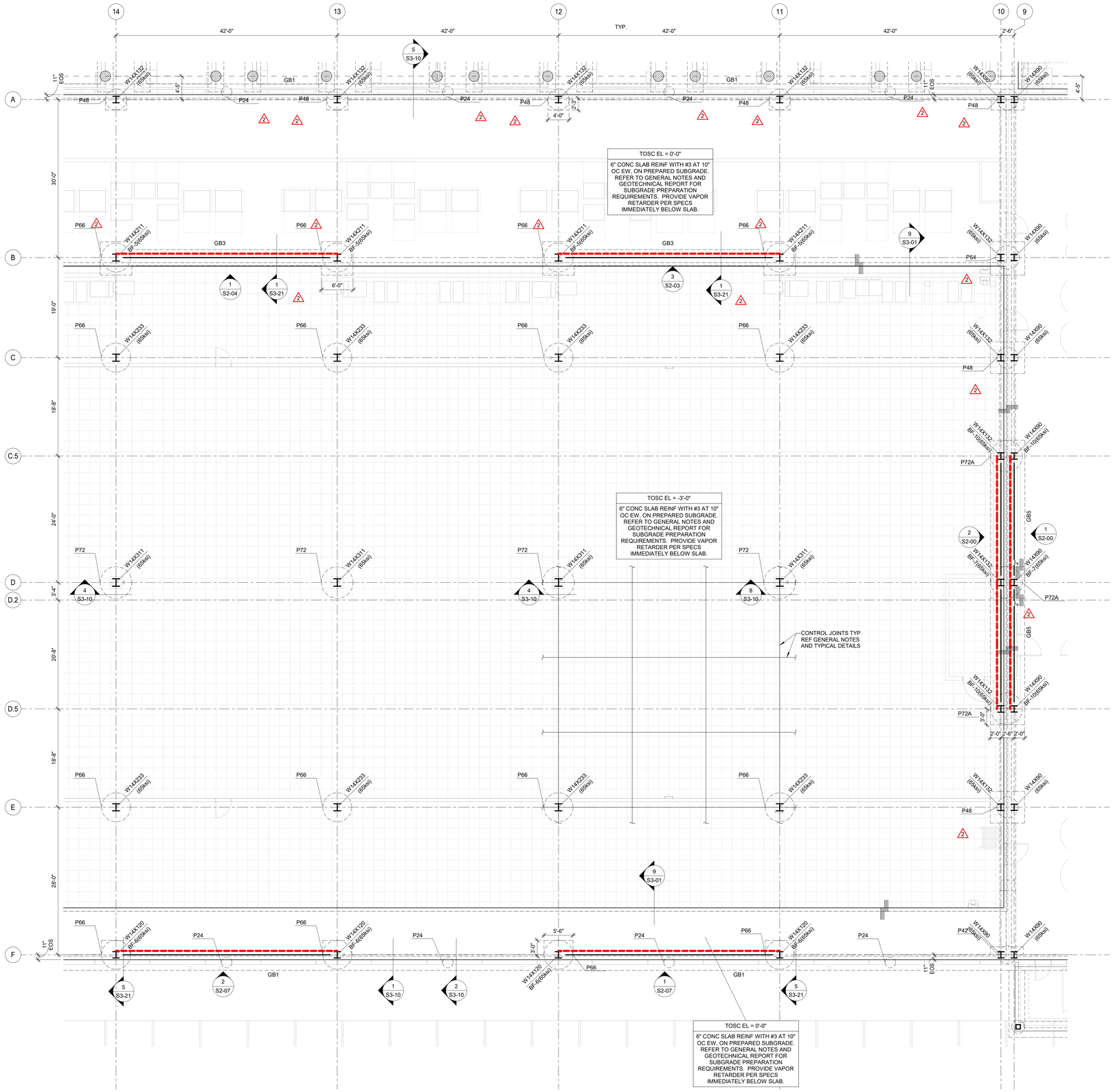
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FOUNDATION PLAN - SEGMENT J

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10J

9/10/2025 5:02:25 PM S1-10J FOUNDATION PLAN - SEGMENT J



1 FOUNDATION PLAN - SEGMENT K
1/8" = 1'-0"

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REVISIONS

1	03.28.25	ADDENDUM 1
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COLORADO LICENSED
NICHOLAS W. HAYID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

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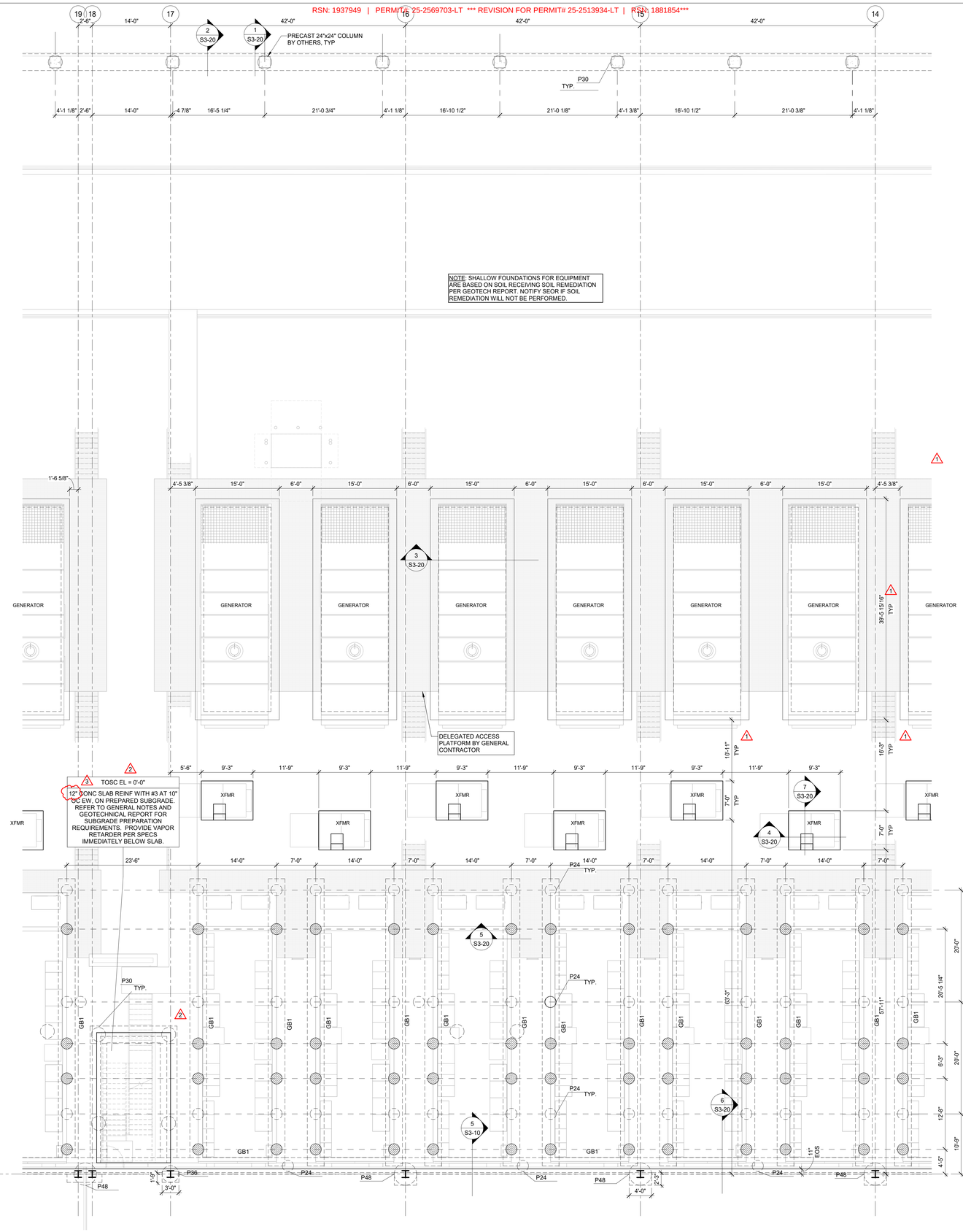
FOUNDATION PLAN - SEGMENT K

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10K

9/10/2025 5:02:27 PM S1-10K FOUNDATION PLAN - SEGMENT K

RSN: 1937949 | PERMIT# 25-2569703-LT *** REVISION FOR PERMIT# 25-2513934-LT | RSN: 1881854***



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1 FOUNDATION PLAN - SEGMENT L
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

ISSUES

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7		
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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

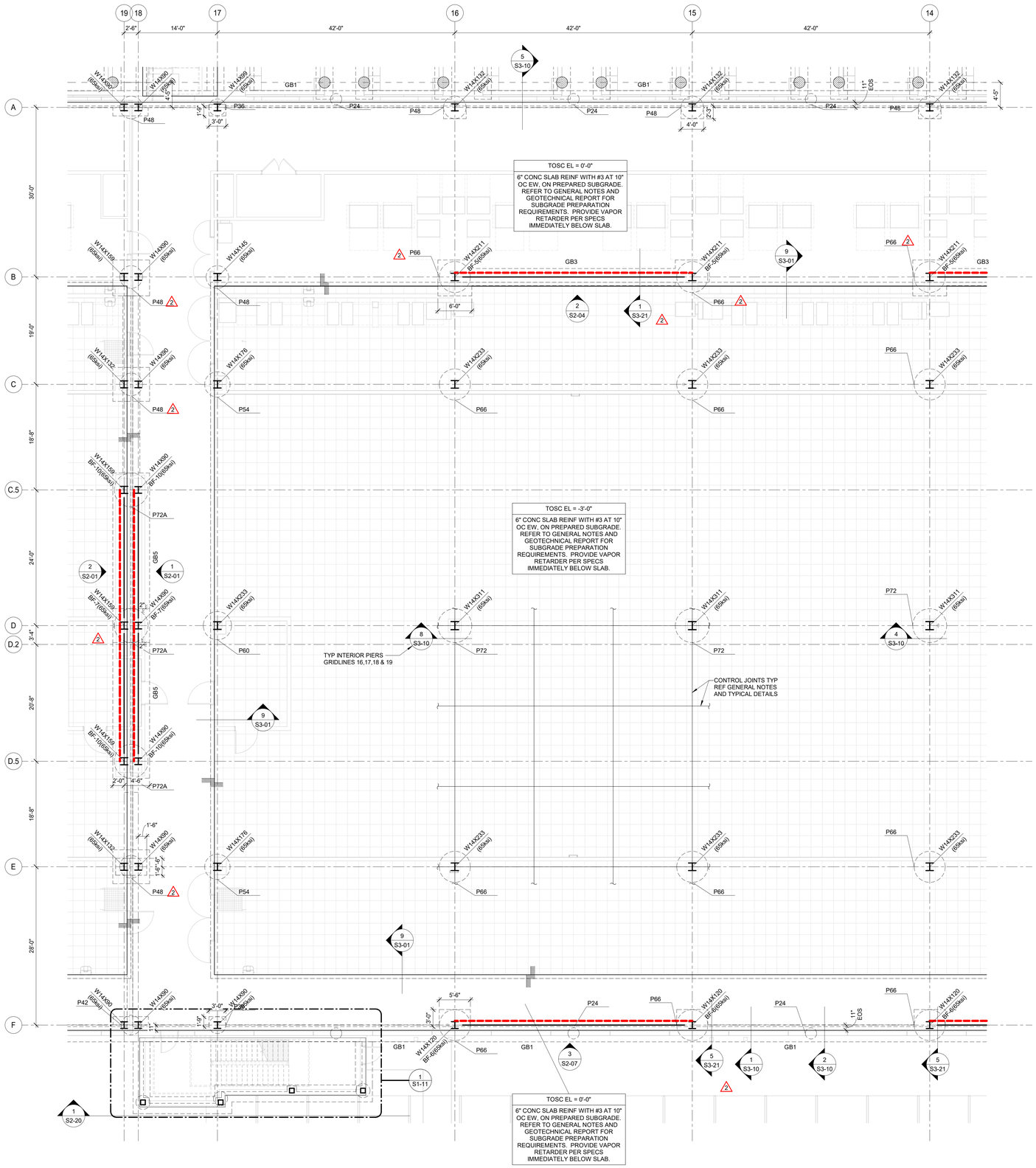
N

FOUNDATION PLAN - SEGMENT L

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10L

9/10/2025 5:02:30 PM S1-10L FOUNDATION PLAN - SEGMENT L



1 FOUNDATION PLAN - SEGMENT M
1/8" = 1'-0"

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
2. VERIFY ALL WALL OPENING DIMENSIONS AND LOCATIONS WITH ARCH DRAWINGS.
3. DEVIATIONS FROM CONST JOINT/CONTROL JOINT PATTERN SHOWN MUST BE APPROVED BY THE ENGINEER OF RECORD.
4. REFER TO SHEET S3-00 FOR PIER SCHEDULE.
5. REFER TO SHEET S5-00 FOR COLUMN AND BASE PLATE SCHEDULE.
6. REFER TO S2-00 FOR BRACE FRAME ELEVATIONS.
7. PIERS WITHOUT CENTERLINES SHOWN ON PLANS, SECTIONS AND/OR DETAILS SHALL BE LOCATED AS FOLLOWS:
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   B. GRADE BEAMS AND WALLS: CENTERLINE OF THE GRADE BEAM OR WALL.
   C. ALONG THE LENGTH OF GRADE BEAMS AND WALLS: INTERMEDIATE PIERS SHALL BE SPACED EQUALLY BETWEEN PIERS THAT ARE DIMENSIONALLY SET ON PLAN OR AS NOTED ABOVE.
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9. VERIFY ALL PAD OPENING DIMENSIONS AND LOCATIONS WITH ARCH/MEP DRAWINGS. VERIFY ALL PAD DIMENSIONS WITH EQUIPMENT MANUFACTURER PRIOR TO FABRICATING REINFORCING AND INSTALLATION.
10. METAL STAIR AND CANOPY BY STAIR MANUF.

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

N

FOUNDATION PLAN - SEGMENT M

JOB 24464.0000
DATE 09.10.2025
SHEET S1-10M



1 FOUNDATION PLAN - SEGMENT N
1/8" = 1'-0"

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
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SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

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REVISIONS

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2	05.30.25	ISSUE FOR CONSTRUCTION
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A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED NICHOLAS W. REVID 65236 PROFESSIONAL ENGINEER

AG&E Structural Engeneuity

Date of issue: 09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

FOUNDATION PLAN - SEGMENT N

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10N

9/10/2025 5:02:36 PM S1-10N FOUNDATION PLAN - SEGMENT N

SHEET NOTES

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TOSC EL = 0'-0"
6" CONC SLAB REINF WITH #3 AT 10" OC EW, ON PREPARED SUBGRADE. REFER TO GENERAL NOTES AND GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION REQUIREMENTS. PROVIDE VAPOR RETARDER PER SPECS IMMEDIATELY BELOW SLAB.

TOSC EL = -3'-0"
6" CONC SLAB REINF WITH #3 AT 10" OC EW, ON PREPARED SUBGRADE. REFER TO GENERAL NOTES AND GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION REQUIREMENTS. PROVIDE VAPOR RETARDER PER SPECS IMMEDIATELY BELOW SLAB.

CONTROL JOINTS TYP REF GENERAL NOTES AND TYPICAL DETAILS

TYP INTERIOR PIERS GRIDLINE 20

1 FOUNDATION PLAN - SEGMENT P
1/8" = 1'-0"

9/10/2025 5:02:39 PM S1-10P FOUNDATION PLAN - SEGMENT P

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DESIGN CONSULTING LLC

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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. ARVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

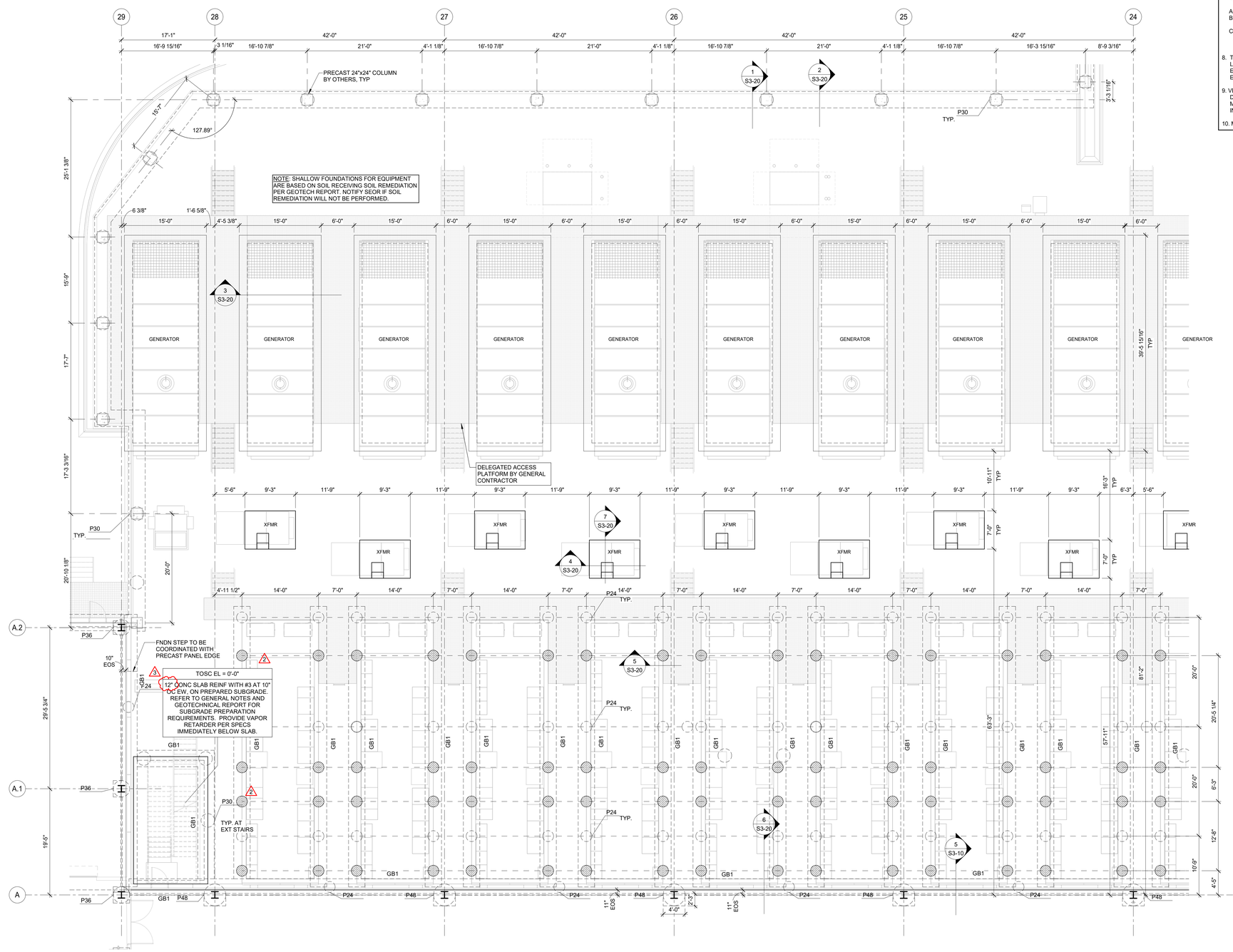
S Q N L J G E C A
T R P M K H F D B

N

FOUNDATION PLAN - SEGMENT P

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10P



SHEET NOTES

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Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 FOUNDATION PLAN - SEGMENT Q
1/8" = 1'-0"

9/10/2025 5:02:41 PM S1-10Q FOUNDATION PLAN - SEGMENT Q

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401 N. Houston Street Dallas, TX 75202
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DESIGN CONSULTING LLC

ISSUES		
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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED NICHOLAS W. ATWOOD 65236 PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

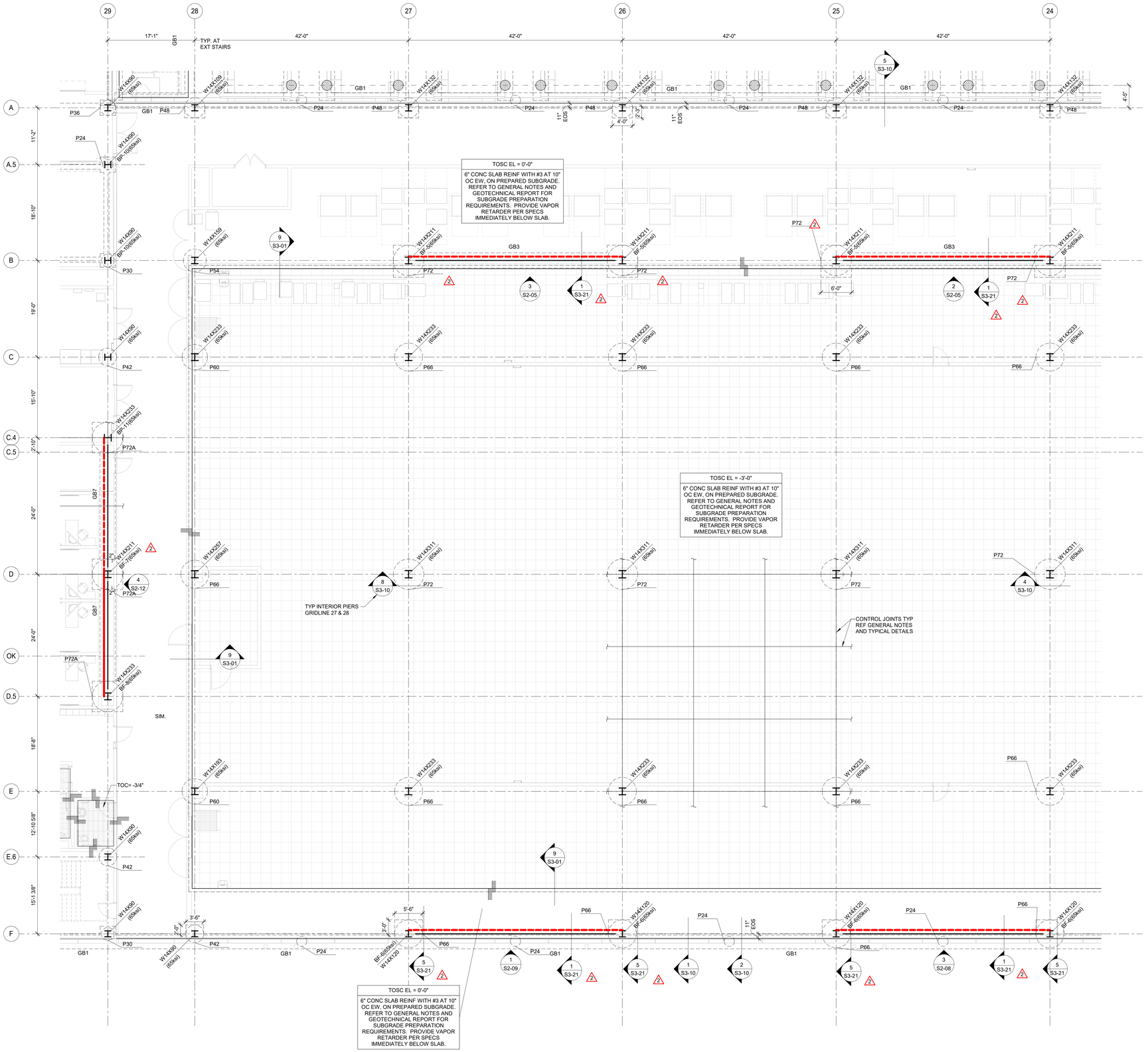
KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

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FOUNDATION PLAN - SEGMENT Q

JOB 24464.0000
DATE 09.10.2025
SHEET S1-10Q



1 FOUNDATION PLAN - SEGMENT R
1/8" = 1'-0"

SHEET NOTES

1. ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KIVIO
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

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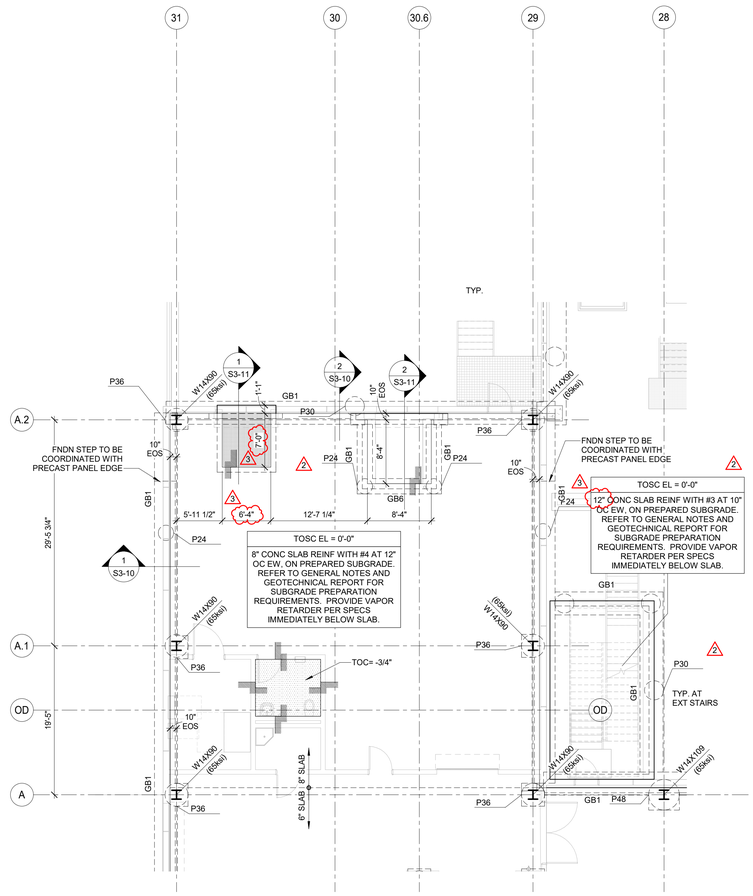
FOUNDATION PLAN - SEGMENT R

JOB 24464.0000
DATE 09.10.2025
SHEET S1-10R

9/10/2025 5:02:45 PM S1-10R FOUNDATION PLAN - SEGMENT R

SHEET NOTES

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1 FOUNDATION PLAN - SEGMENT S
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
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SYSKA HENNESSY GROUP

DESIGN CONSULTING LLC

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COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

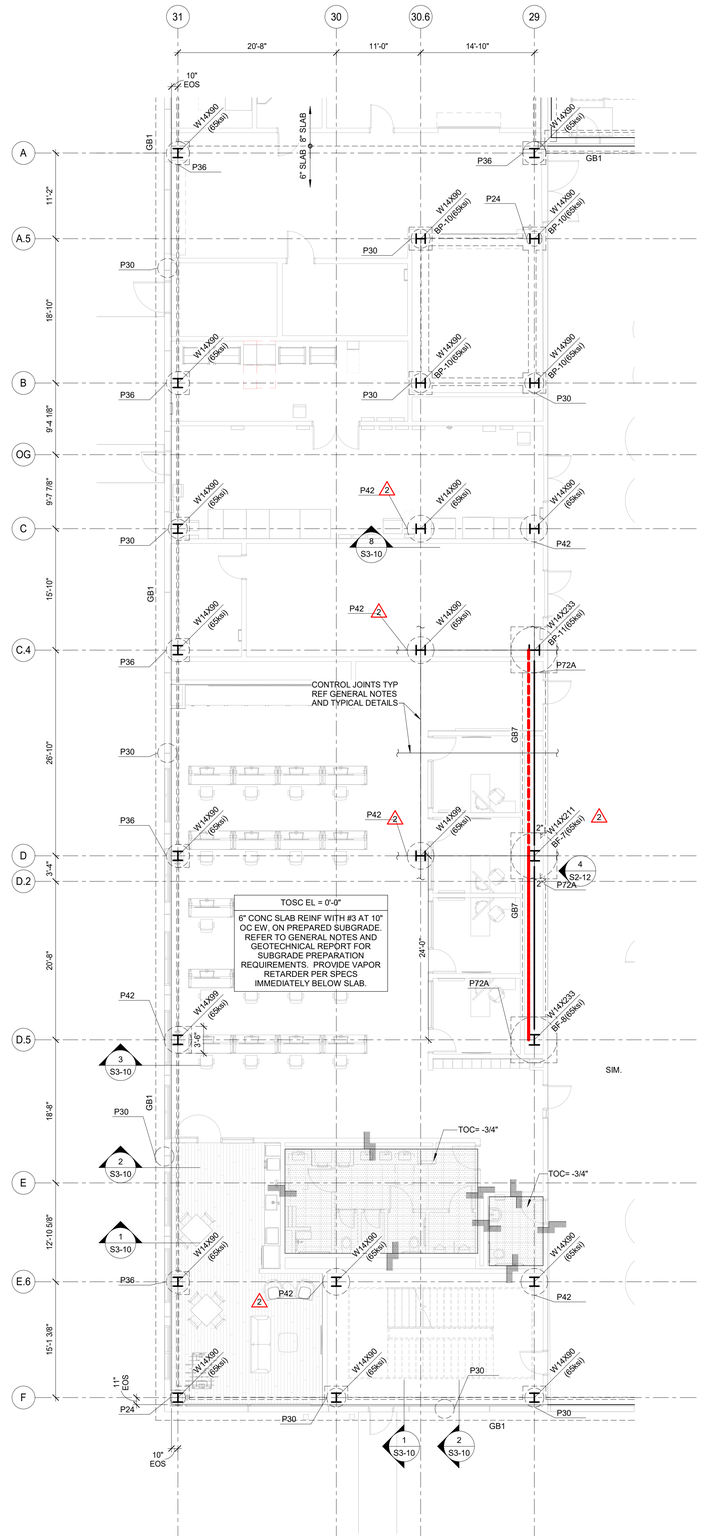
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FOUNDATION PLAN - SEGMENT S

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10S

9/10/2025 5:02:47 PM S1-10S FOUNDATION PLAN - SEGMENT S



SHEET NOTES

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1 FOUNDATION PLAN - SEGMENT T
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

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DESIGN CONSULTING LLC

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COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

FOUNDATION PLAN - SEGMENT T

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-10T

9/10/2025 5:02:50 PM S1-10T FOUNDATION PLAN - SEGMENT T

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401 N. Houston Street Dallas, TX
75202
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2	05.30.25	ISSUE FOR CONSTRUCTION



AG&E Structural Engenuity

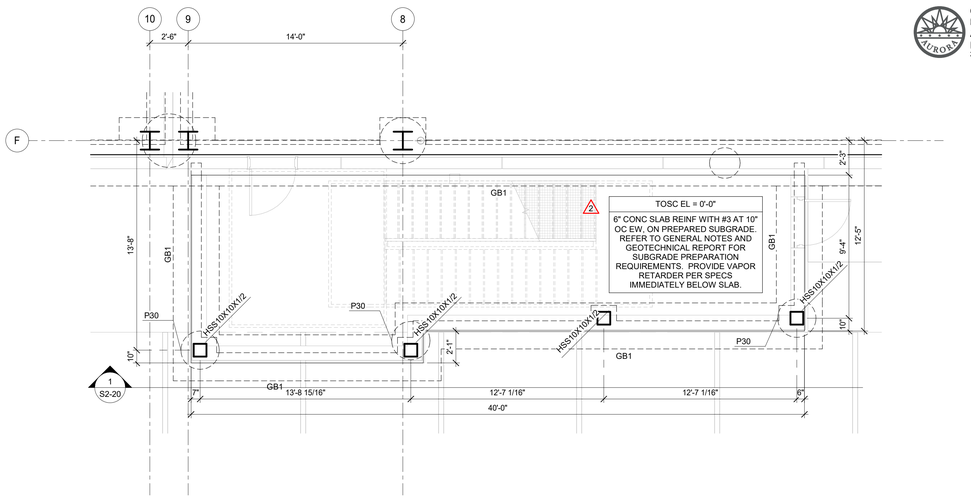
Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

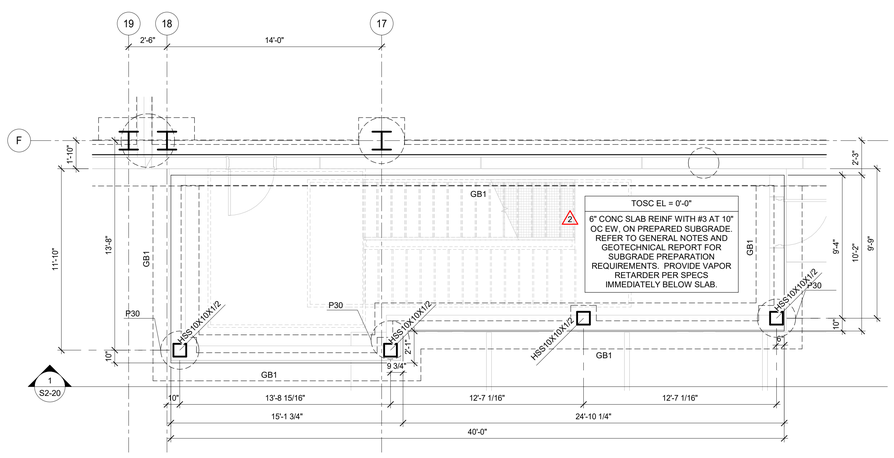
QTS

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



2 EXTERIOR STAIR FOUNDATION PLAN
1/4" = 1'-0"



1 EXTERIOR STAIR FOUNDATION PLAN
1/4" = 1'-0"

PARTIAL FOUNDATION PLANS

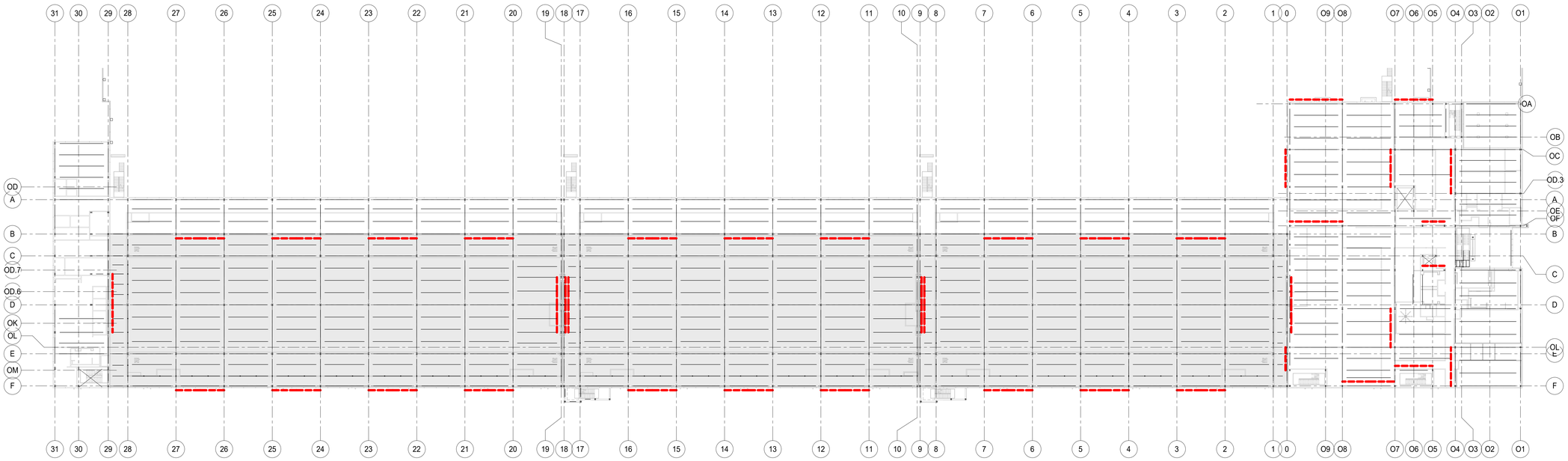
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-11

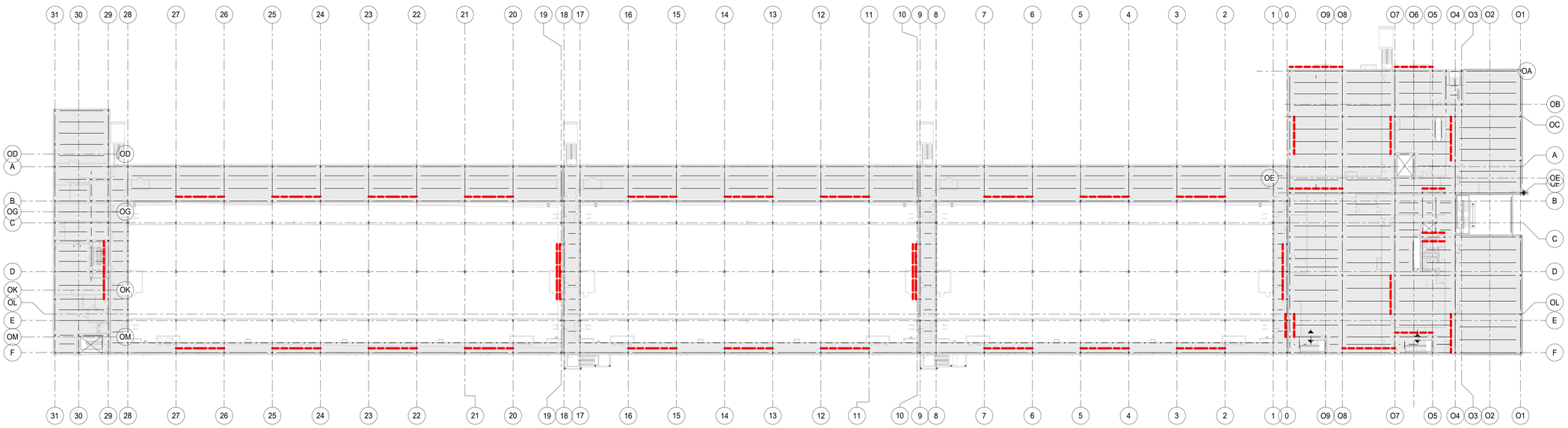
9/10/2025 5:02:52 PM S1-11 PARTIAL FOUNDATION PLANS

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City of Aurora Building Division
Reviewed for Code Compliance
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2021 INTERNATIONAL CODES & 2023 NEC



2 OVERALL SECOND FLOOR FRAMING PLAN (LO)
1" = 40'-0"



1 OVERALL SECOND FLOOR FRAMING PLAN (HI)
1" = 40'-0"

9/10/2025 5:03:00 PM S1-20 OVERALL SECOND FLOOR FRAMING PLAN

CORGAN
401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

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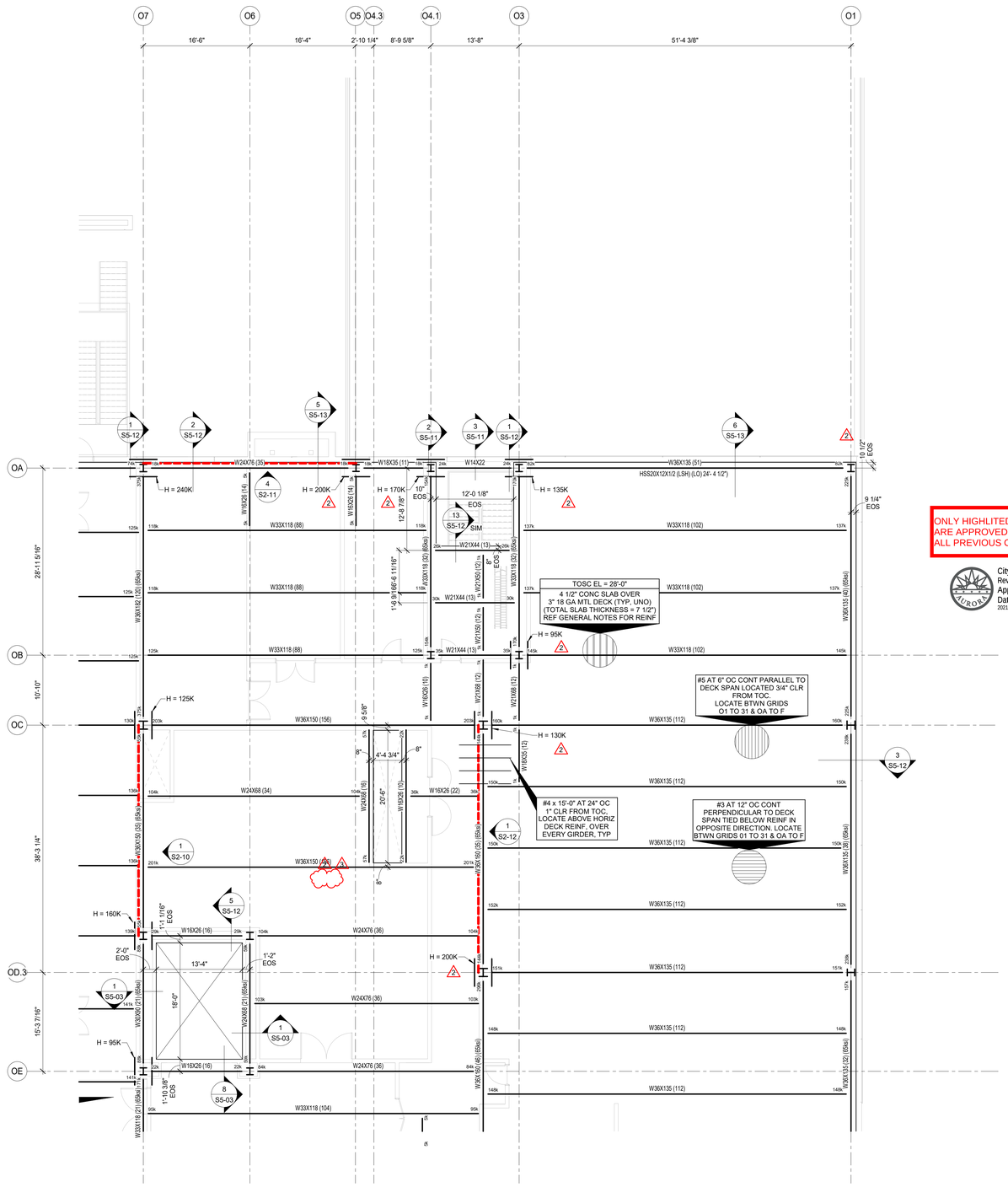
OVERALL SECOND FLOOR FRAMING PLAN

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
   DATA HALL LOW SLAB TOSC EL = 25'-0"
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. DESIGN LIVE LOAD = REF SCHEDULE
5. COMPOSITE BEAM CONSTRUCTION IS UNSHORED.
6. BEAMS THAT ARE MARKED "REF ELEVATION" ARE SHOWN ON FRAME ELEVATIONS.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



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2021 INTERNATIONAL CODES & 2023 NEC

1 SECOND FLOOR FRAMING PLAN - SEGMENT A
1/8" = 1'-0"

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SYSKA HENNESSY GROUP
TLD DESIGN CONSULTING LLC

ISSUES		
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6	09.10.25	IFC.1
7		
8		
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10		

REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD
QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B
N

SECOND FLOOR FRAMING PLAN - SEGMENT A

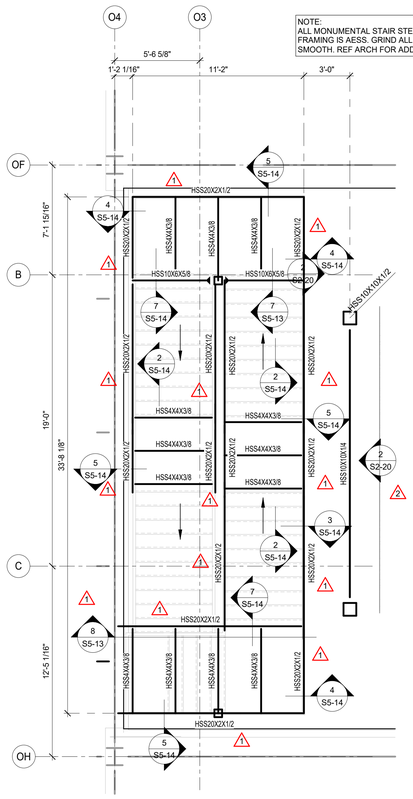
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20A

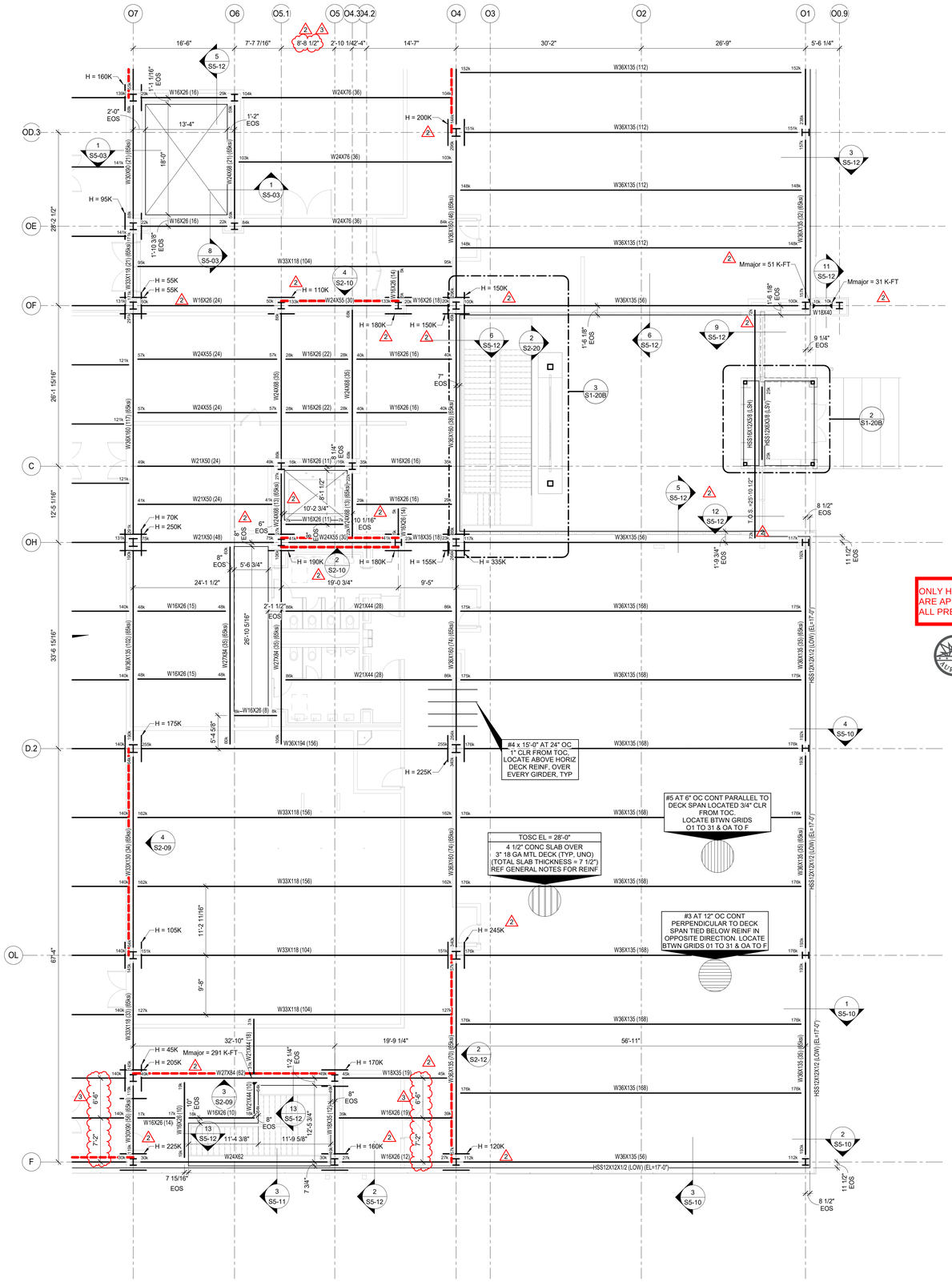
9/10/2025 5:03:03 PM S1-20A SECOND FLOOR FRAMING PLAN - SEGMENT A

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
   DATA HALL LOW SLAB TOSC EL = 25'-0"
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. DESIGN LIVE LOAD = REF SCHEDULE
5. COMPOSITE BEAM CONSTRUCTION IS UNSHORED.
6. BEAMS THAT ARE MARKED "REF ELEVATION" ARE SHOWN ON FRAME ELEVATIONS.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.

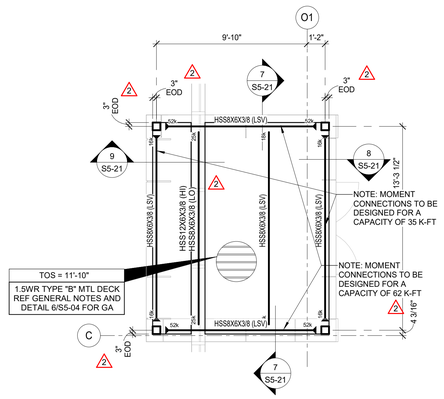


3 MONUMENT STAIR FRAMING PLAN
1/4" = 1'-0"



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2 VESTIBULE FRAMING PLAN
1/4" = 1'-0"

1 SECOND FLOOR FRAMING PLAN - SEGMENT B
1/8" = 1'-0"

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DESIGN CONSULTING LLC

	ISSUES	
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7		
8		
9		
10		

	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

SECOND FLOOR FRAMING PLAN - SEGMENT B

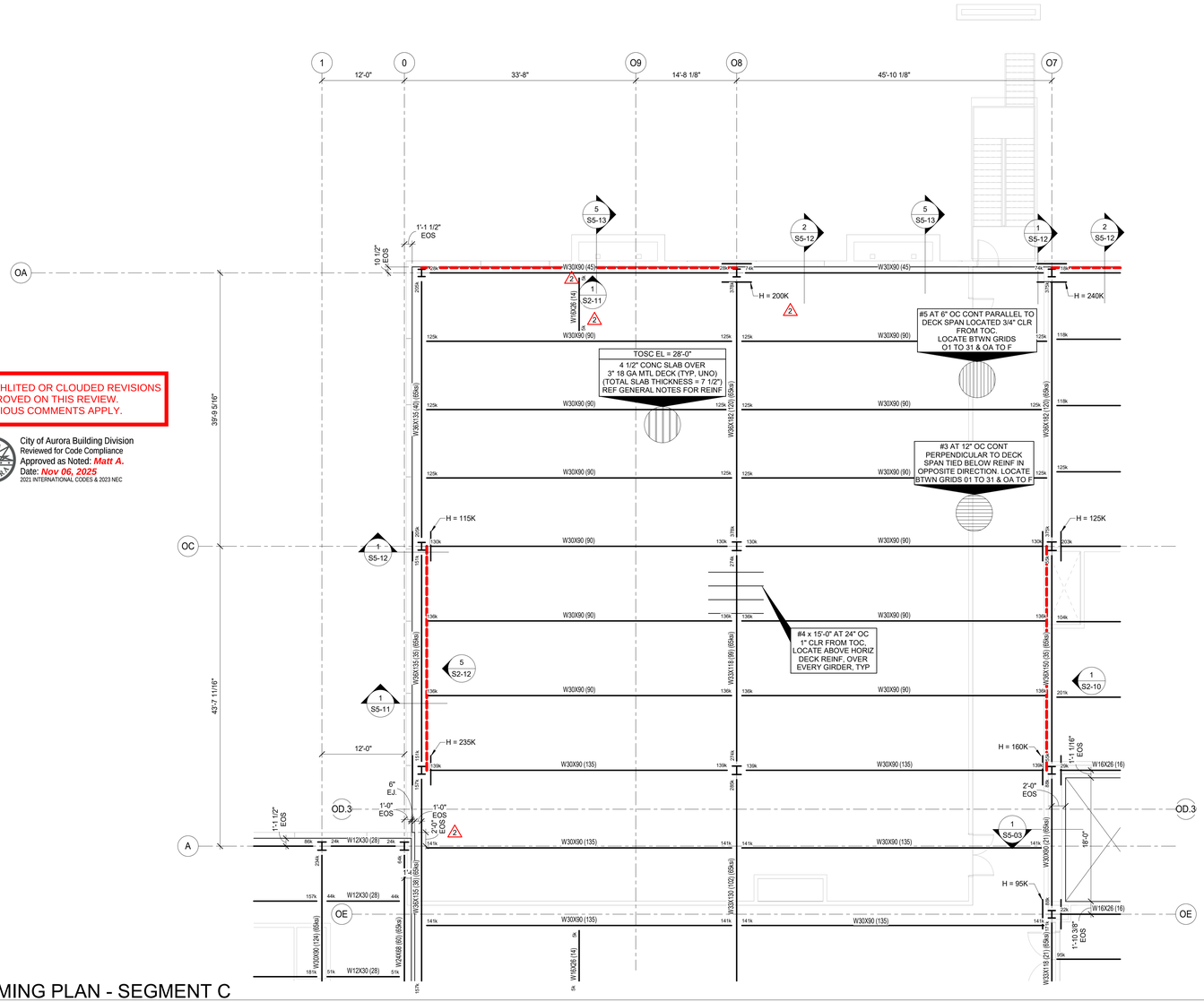
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20B

9/10/2025 5:03:08 PM S1-20B SECOND FLOOR FRAMING PLAN - SEGMENT B

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
   DATA HALL LOW SLAB TOSC EL = 25'-0"
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5. COMPOSITE BEAM CONSTRUCTION IS UNSHORED.
6. BEAMS THAT ARE MARKED "REF ELEVATION" ARE SHOWN ON FRAME ELEVATIONS.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



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2021 INTERNATIONAL CODES & 2023 NEC

1 SECOND FLOOR FRAMING PLAN - SEGMENT C
1/8" = 1'-0"

9/10/2025 5:03:10 PM S1-20C SECOND FLOOR FRAMING PLAN - SEGMENT C

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ISSUES		
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
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7		
8		
9		
10		

REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

N

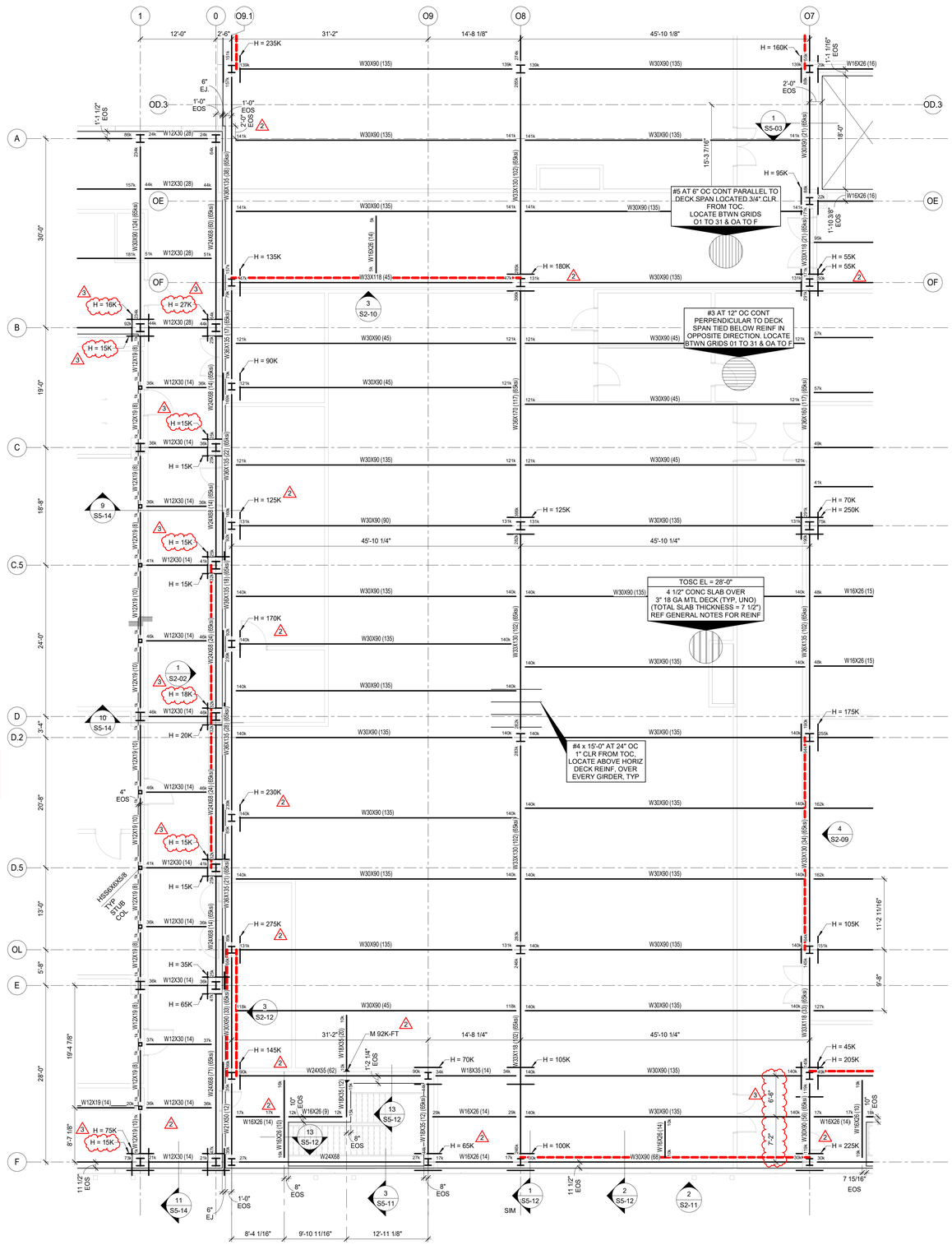
SECOND FLOOR FRAMING PLAN - SEGMENT C

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20C

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
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1 SECOND FLOOR FRAMING PLAN - SEGMENT D
1/8" = 1'-0"

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	ISSUES	
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4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B
N

SECOND FLOOR FRAMING PLAN - SEGMENT D

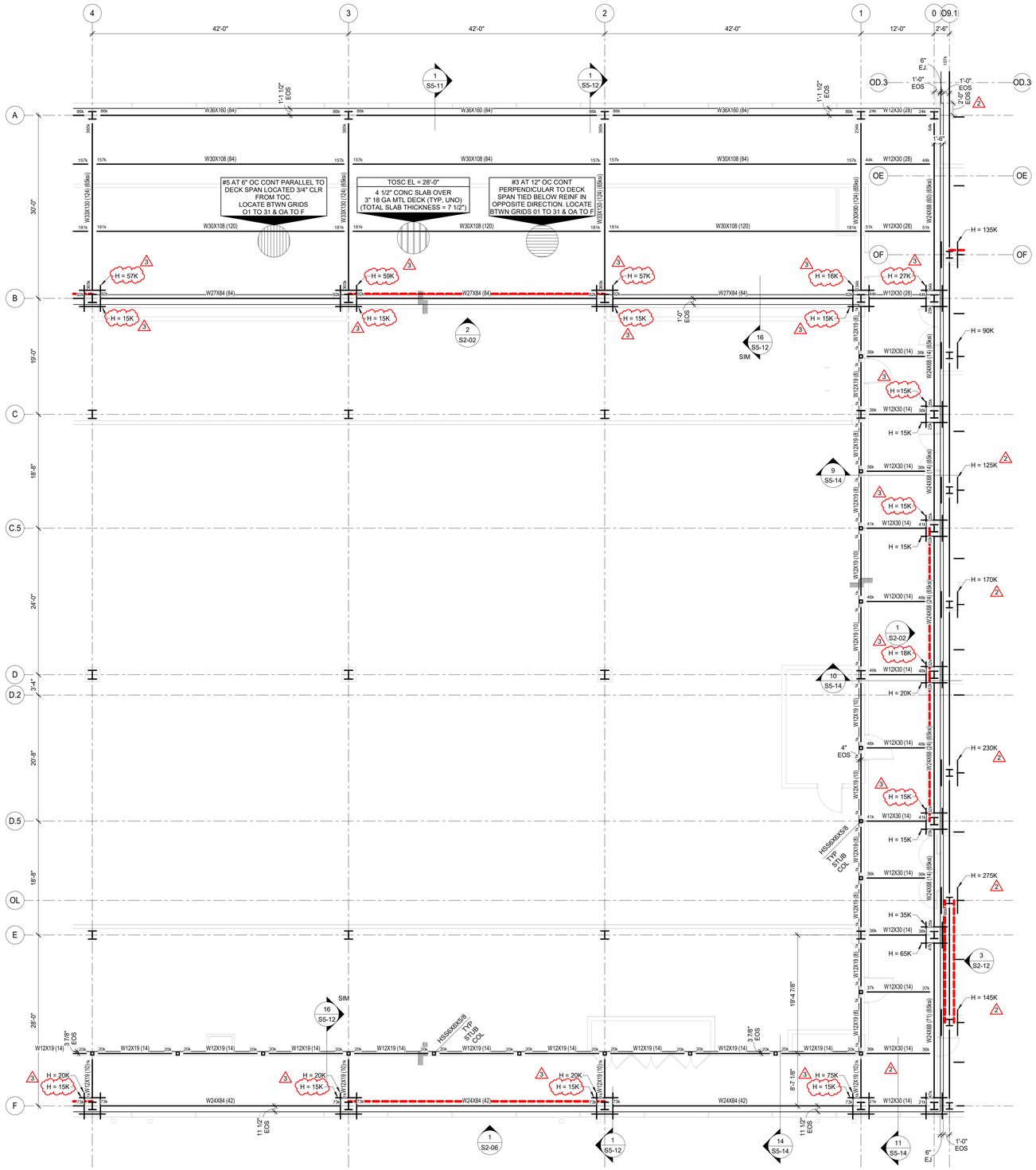
JOB 24464.0000
DATE 09.10.2025
SHEET

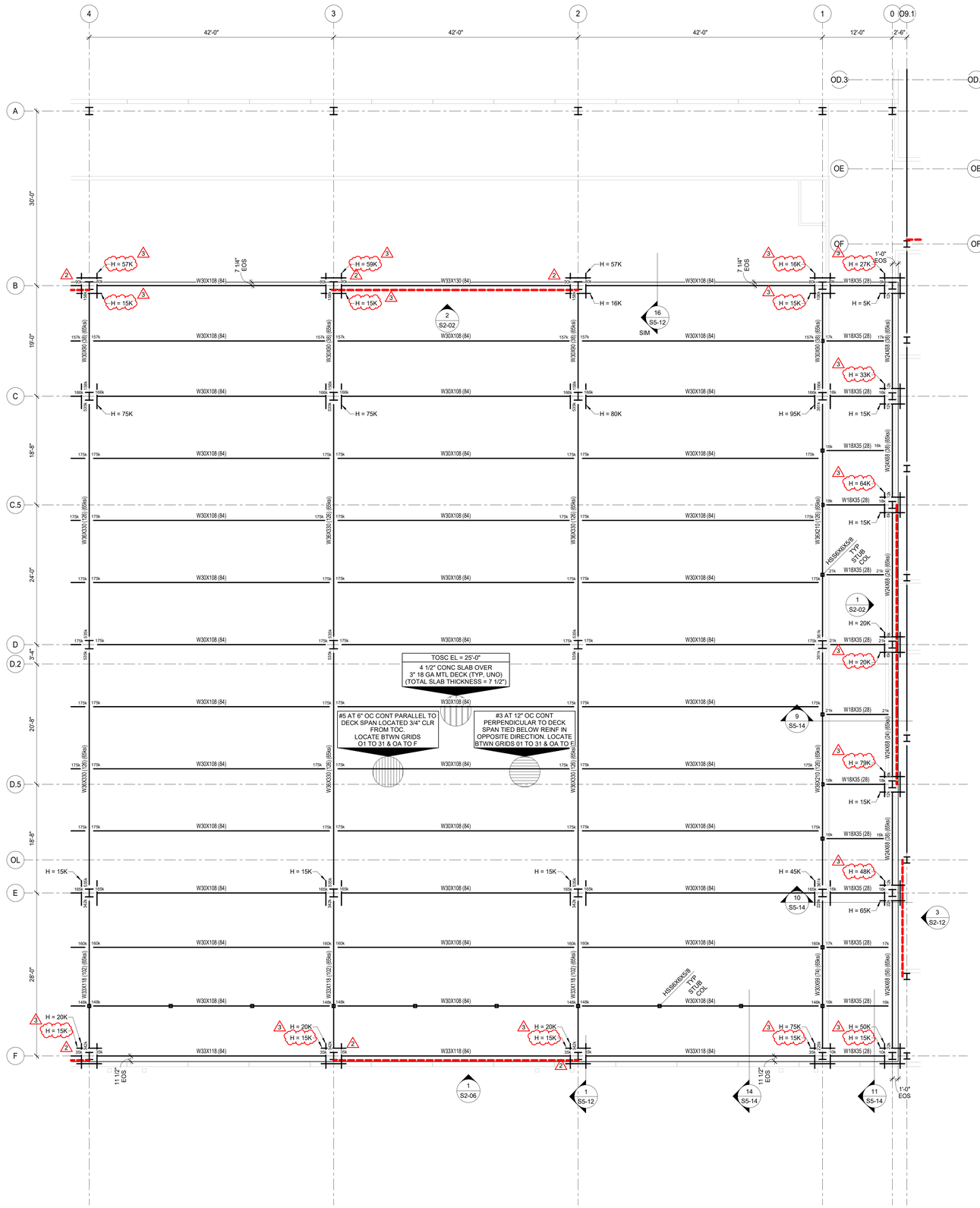
S1-20D

9/10/2025 5:03:14 PM S1-20D SECOND FLOOR FRAMING PLAN - SEGMENT D

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
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7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.


#5 AT 6" OC CONT PARALLEL TO DECK SPAN LOCATED 3/4" CLR FROM TOC, LOCATE BTWN GRIDS O1 TO 31 & OA TO F
TOSC EL = 28'-0"
4 1/2" CONC SLAB OVER 3" 18 GA MTL DECK (TYP, UNO) (TOTAL SLAB THICKNESS = 7 1/2")
#3 AT 12" OC CONT PERPENDICULAR TO DECK SPAN TIED BELOW REINF IN OPPOSITE DIRECTION. LOCATE BTWN GRIDS 01 TO 31 & OA TO F
W36X160 (84)
W30X108 (84)
W30X108 (120)
W27X84 (84)
W24X84 (42)
W12X30 (28)
W12X30 (14)
W12X19 (14)
W12X19 (10)
W12X19 (8)
W12X19 (10)
W12X19 (8)
W12X19 (10)
W12X19 (8



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2021 INTERNATIONAL CODES & 2023 NEC

1 SECOND FLOOR FRAMING PLAN - SEGMENT F-LOW
1/8" = 1'-0"

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401 N. Houston Street Dallas, TX
75202
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AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

	ISSUES	
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10		

	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B
N

LOWER SECOND FLOOR FRAMING PLAN - SEGMENT F

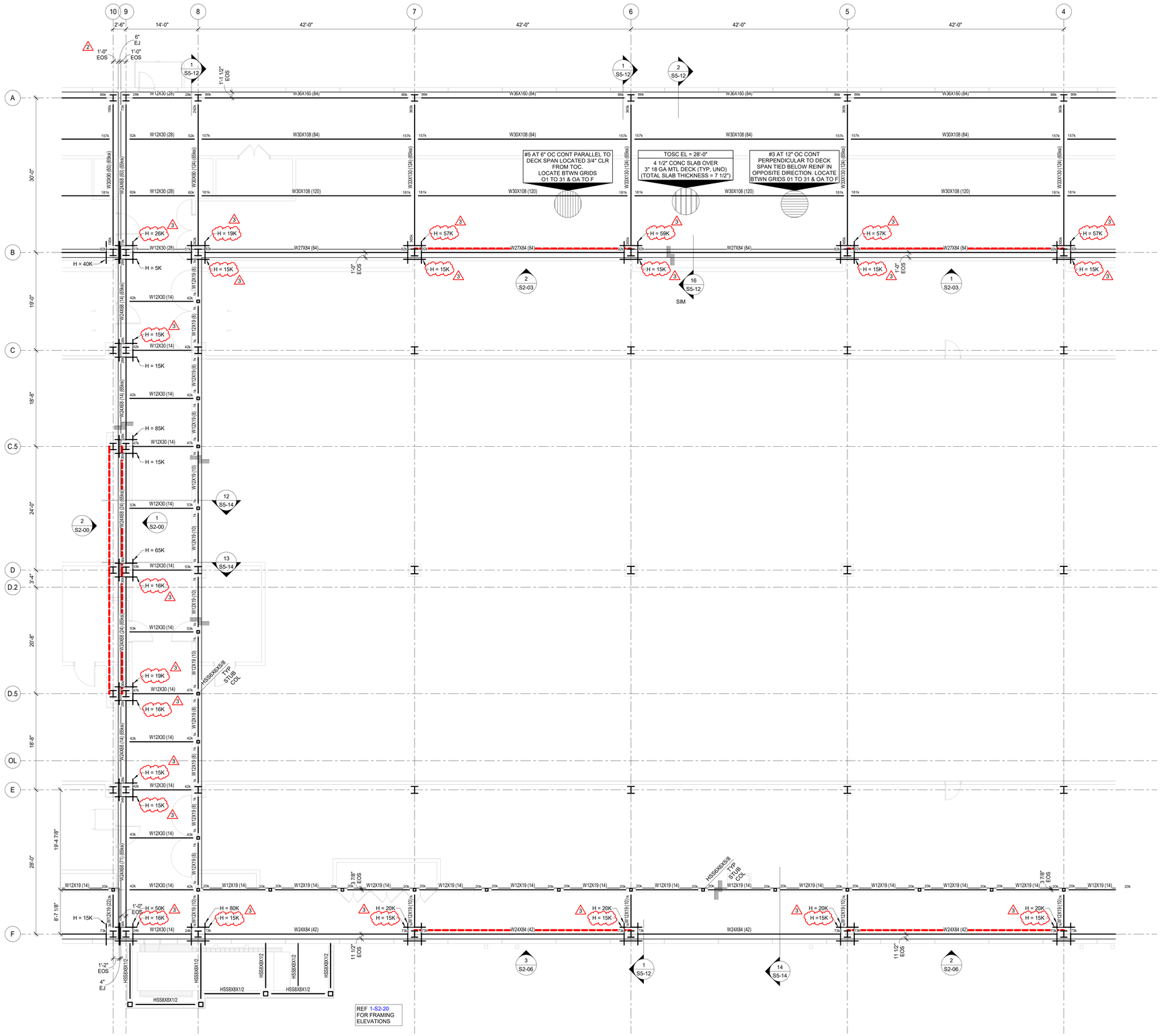
JOB 24464.0000
DATE 09.10.2025
SHEET

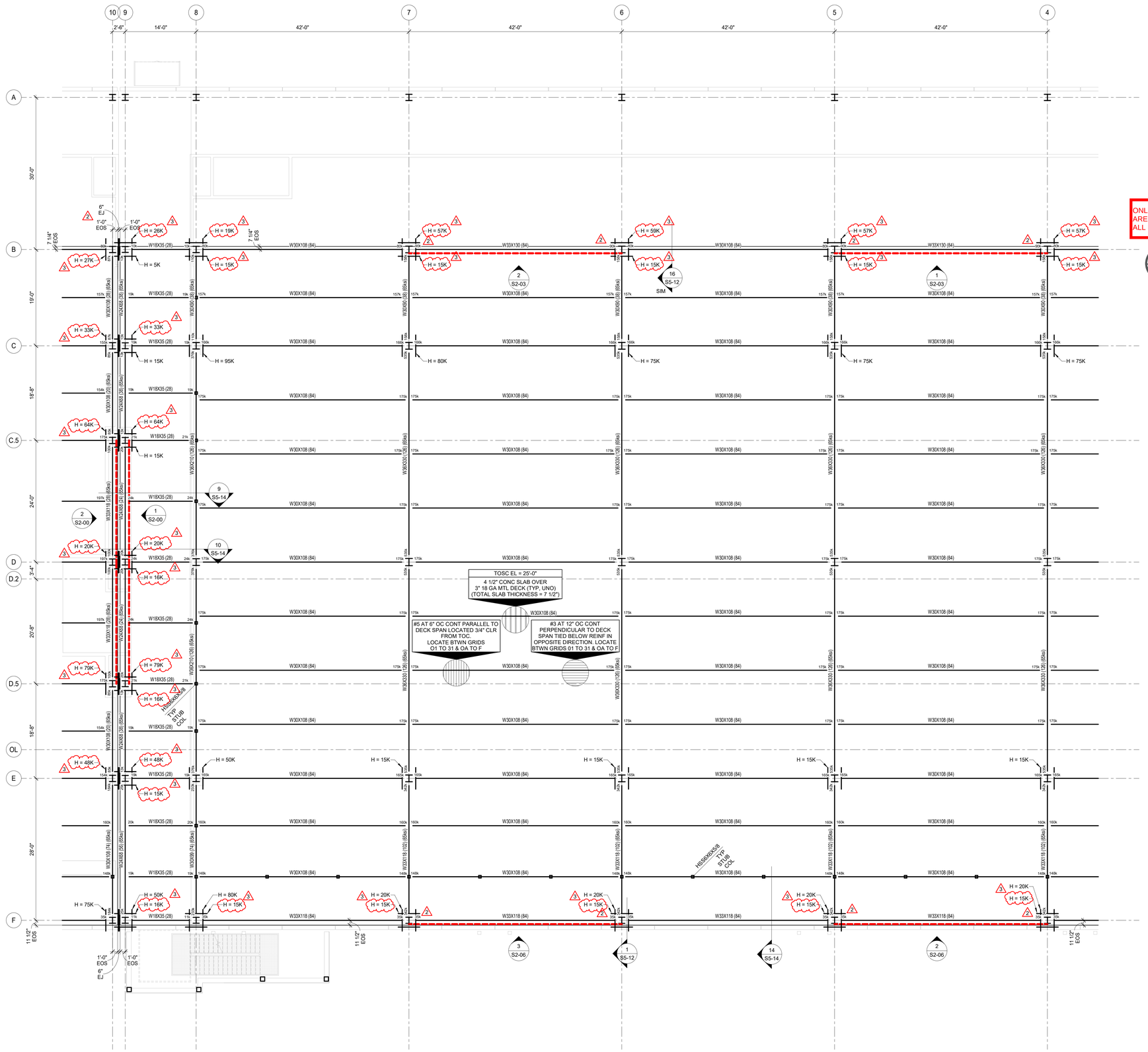
S1-20Fb

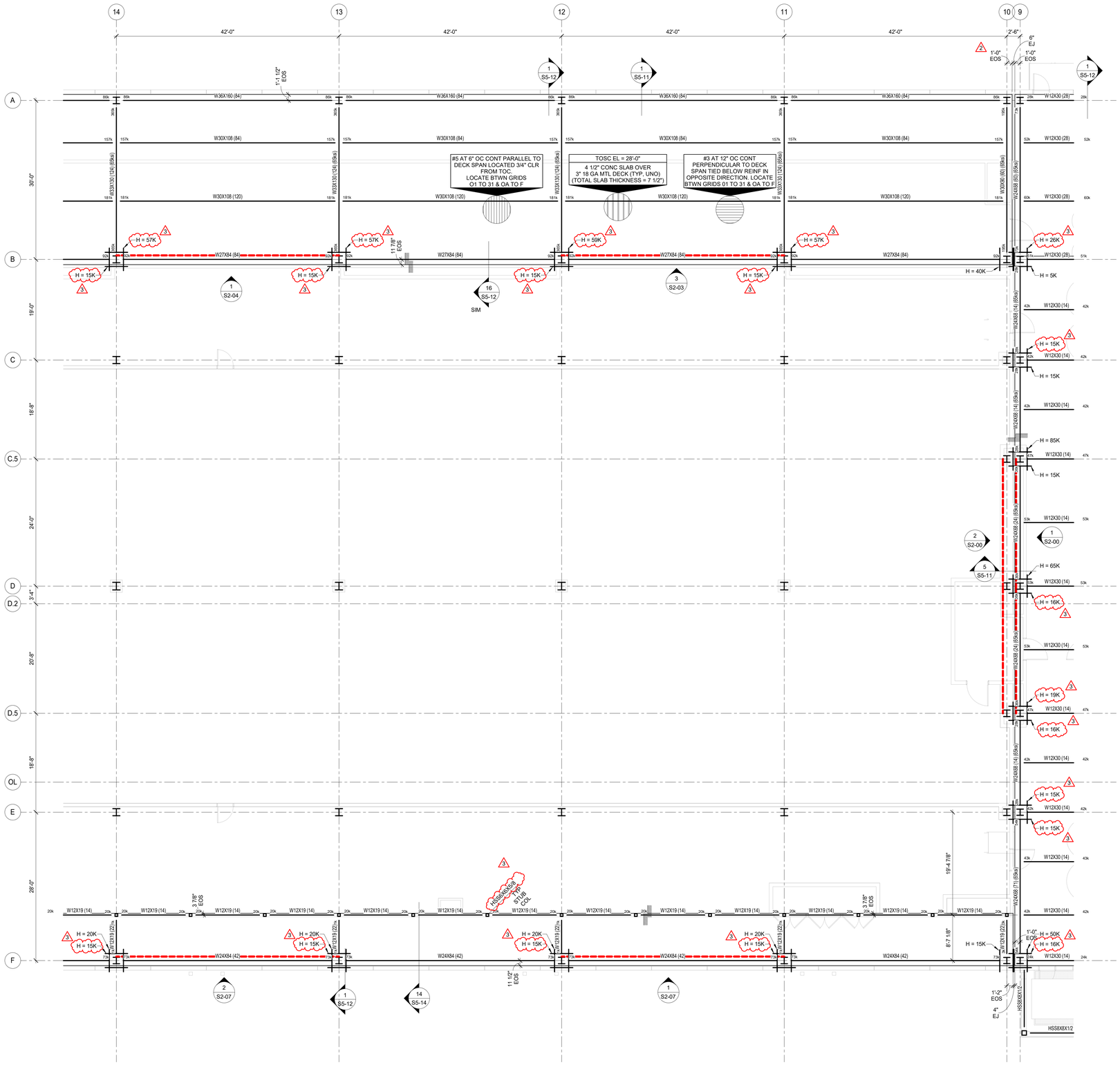
9/10/2025 5:03:19 PM S1-20Fb LOWER SECOND FLOOR FRAMING PLAN - SEGMENT F

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
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7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.


10
9
8
7
6
5
4
A
B
C
C.5
D
D.2
D.5
OL
E
F
2'-6"
14'-0"
42'-0"
30'-0"
19'-0"
18'-6"
24'-0"
3'-4"
20'-8"
18'-8"
19'-4 7/8"
26'-0"
8'-7 1/8"
6" EJ
1'-0" EOS
1 S5-12
2 S5-12
11'-1 1/2" EOS
W36X160 (84)
W30X108 (84)
W30X108 (120)
W27X84 (84)
W12X30 (28)
W12X30 (14)
W12X19 (14)
W12X19 (8)
W12X19 (10)
W12X19 (6)
W12X19 (12)
W24X84 (42)
W12X19 (14) 20'-0"
W12X19 (14) 19'-11 1/2"
W12X19 (14) 19'-10"
W12X19 (14) 19'-9 1/2"
W12X19 (14) 19'-9"
W12X19 (14) 19'-8 1/2"
W12X19 (14) 19'-8"
W12X19 (14) 19'-7 1/2"
W12X19 (14) 19'-7"
W12X19 (14) 19'-6 1/2"
W12X19 (14) 19'-6"
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W12X19 (14) 18'-11 1/2"
W12X19 (14) 18'-11"
W12X19 (14) 18'-10 1/2"
W12X19 (14) 18'-10"
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W12X19 (14) 18'-0"
W12X19 (14) 17'-11 1/2"
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SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
   DATA HALL LOW SLAB TOSC EL = 25'-0"
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. DESIGN LIVE LOAD = REF SCHEDULE
5. COMPOSITE BEAM CONSTRUCTION IS UNSHORED.
6. BEAMS THAT ARE MARKED "REF ELEVATION" ARE SHOWN ON FRAME ELEVATIONS.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW.
ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 SECOND FLOOR FRAMING PLAN - SEGMENT K
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
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9		
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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

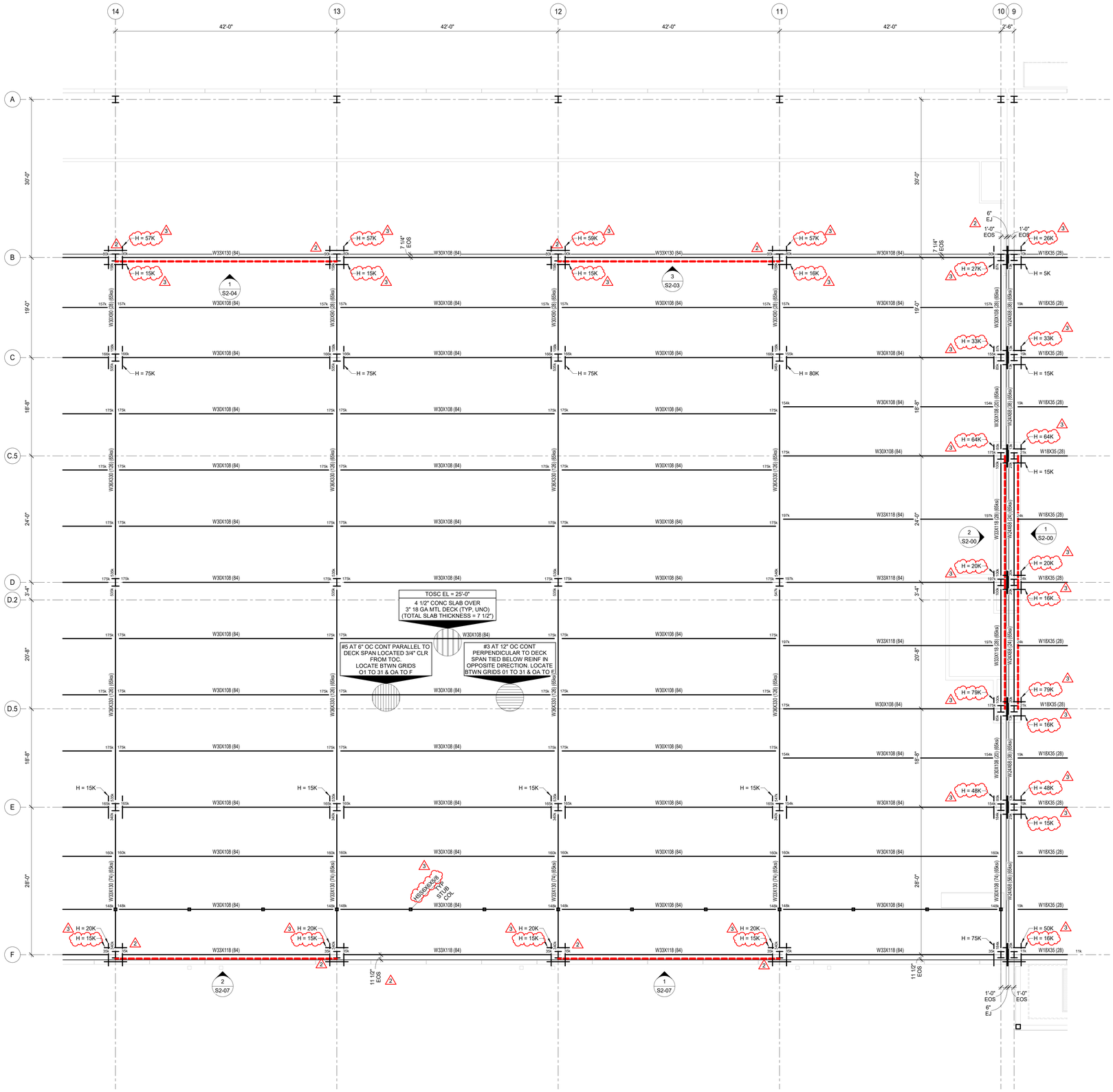
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SECOND FLOOR FRAMING PLAN - SEGMENT K

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20K

9/10/2025 5:03:28 PM S1-20K SECOND FLOOR FRAMING PLAN - SEGMENT K



1 SECOND FLOOR FRAMING PLAN - SEGMENT K-LOW
1/8" = 1'-0"

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW.
ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TCD DESIGN CONSULTING LLC

ISSUES		
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

N

LOWER SECOND FLOOR FRAMING PLAN - SEGMENT K

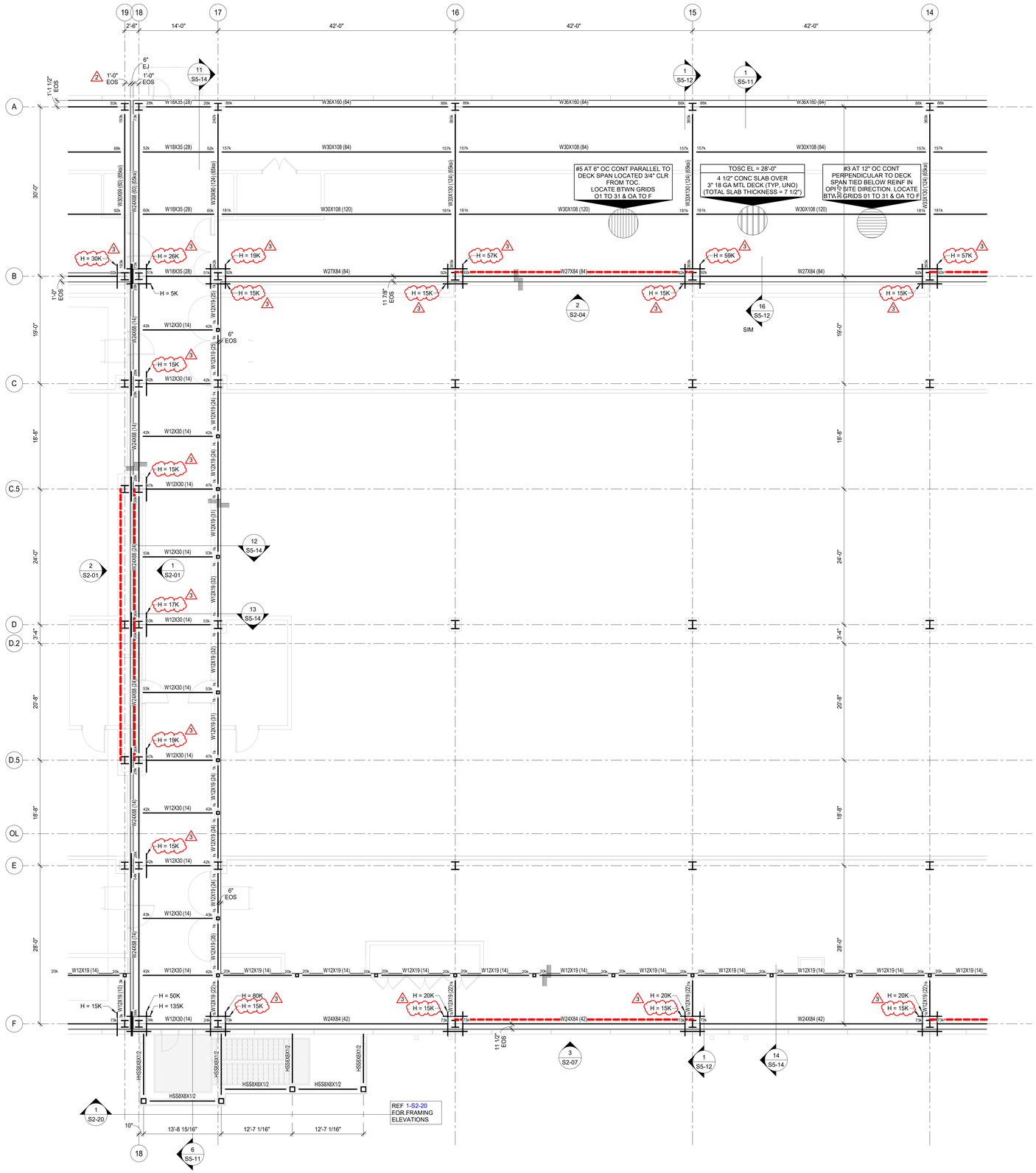
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20Kb

9/10/2025 5:03:31 PM S1-20Kb LOWER SECOND FLOOR FRAMING PLAN - SEGMENT K

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
   DATA HALL LOW SLAB TOSC EL = 25'-0"
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. DESIGN LIVE LOAD = REF SCHEDULE
5. COMPOSITE BEAM CONSTRUCTION IS UNSHORED.
6. BEAMS THAT ARE MARKED "REF ELEVATION" ARE SHOWN ON FRAME ELEVATIONS.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



1 SECOND FLOOR FRAMING PLAN - SEGMENT M
1/8" = 1'-0"

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
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	REVISIONS	
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED NICHOLAS W. REVID 65236 PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

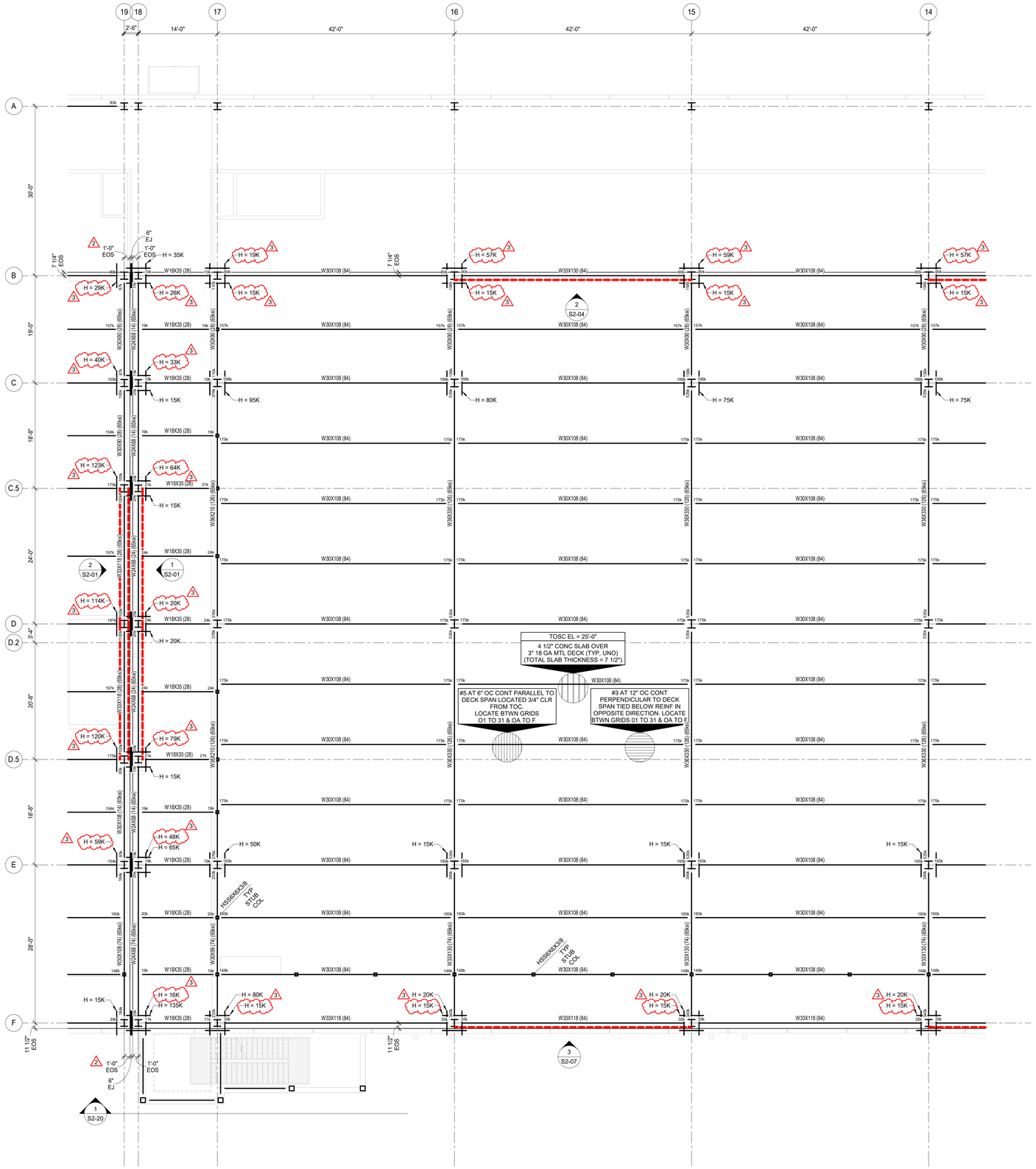
S Q N L J G E C A
T R P M K H F D B
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SECOND FLOOR FRAMING PLAN - SEGMENT M

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20M

9/10/2025 5:03:34 PM S1-20M SECOND FLOOR FRAMING PLAN - SEGMENT M



1 SECOND FLOOR FRAMING PLAN - SEGMENT M-LOW
1/8" = 1'-0"

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City of Aurora Building Division
Reviewed for Code Compliance
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2021 INTERNATIONAL CODES & 2023 NEC

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ISSUES		
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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KIVLIN
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

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LOWER SECOND FLOOR FRAMING PLAN - SEGMENT M

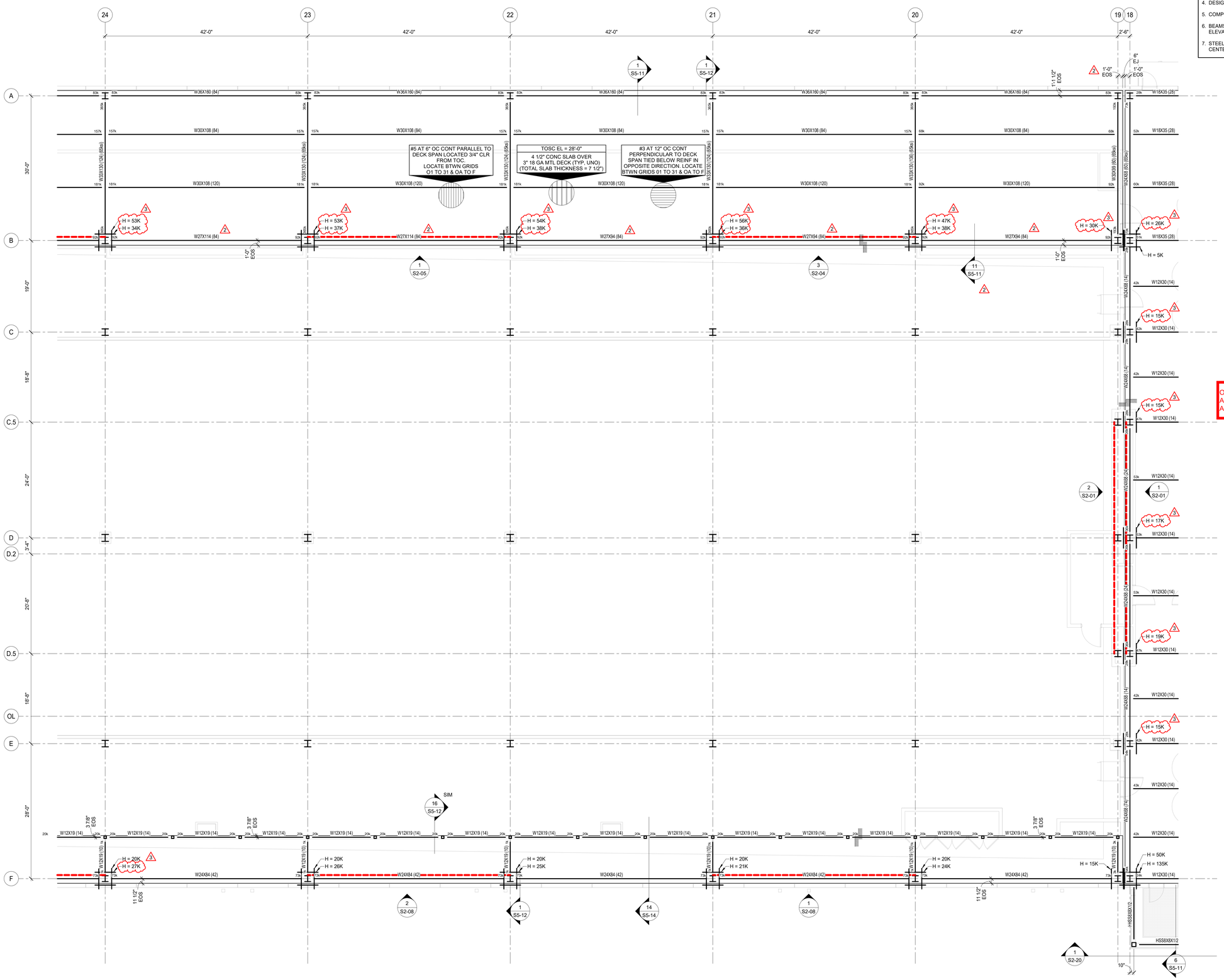
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20Mb

9/10/2025 5:03:37 PM S1-20Mb LOWER SECOND FLOOR FRAMING PLAN - SEGMENT M

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
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6. BEAMS THAT ARE MARKED "REF ELEVATION" ARE SHOWN ON FRAME ELEVATIONS.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



1 SECOND FLOOR FRAMING PLAN - SEGMENT P
1/8" = 1'-0"

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

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SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

	ISSUES	
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	REVISIONS	
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

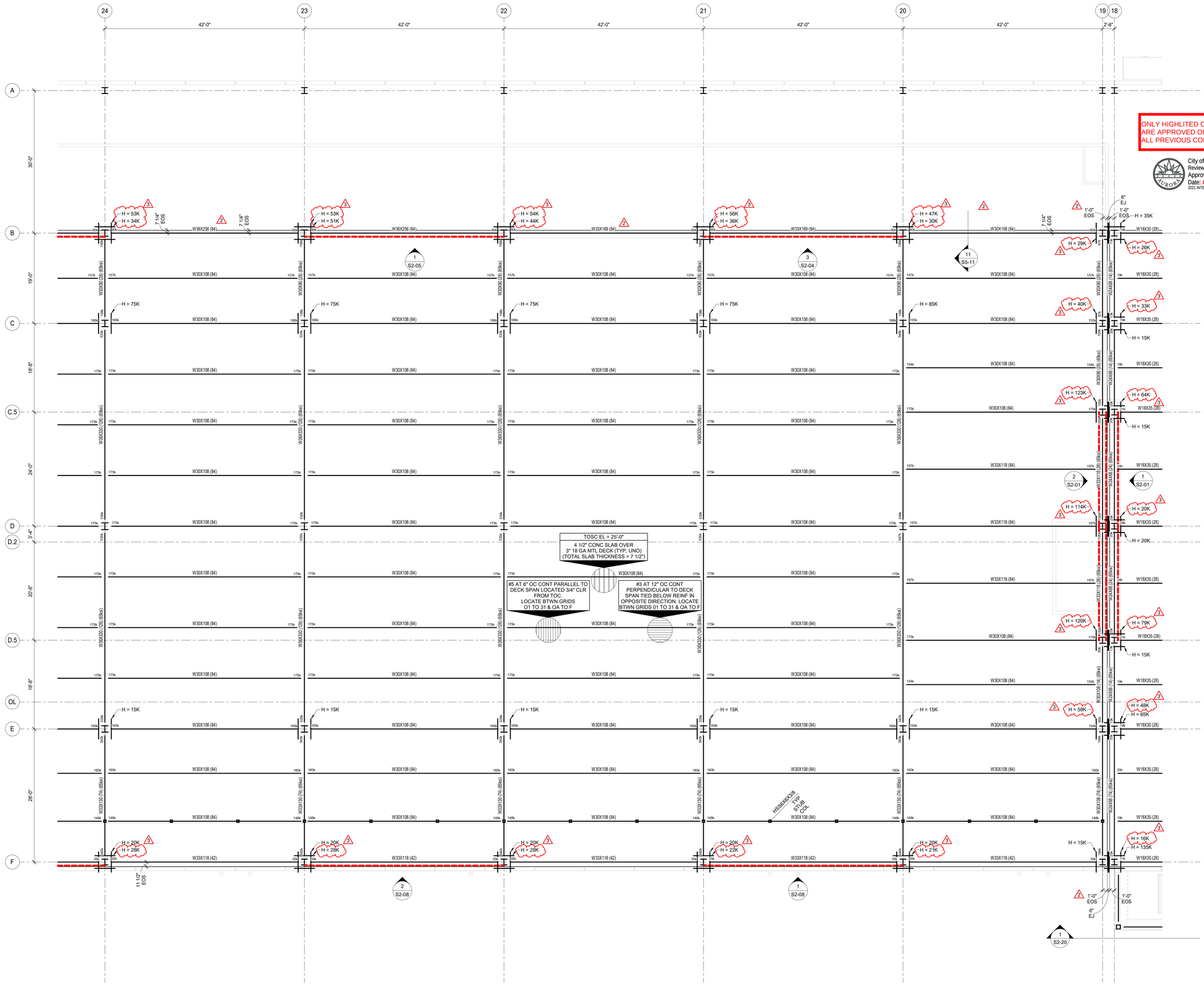
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SECOND FLOOR FRAMING PLAN - SEGMENT P

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20P

9/10/2025 5:03:40 PM S1-20P SECOND FLOOR FRAMING PLAN - SEGMENT P



1 SECOND FLOOR FRAMING PLAN - SEGMENT P-LOW
1/8" = 1'-0"

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ISSUES		
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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEYID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

LOWER SECOND FLOOR FRAMING PLAN - SEGMENT P

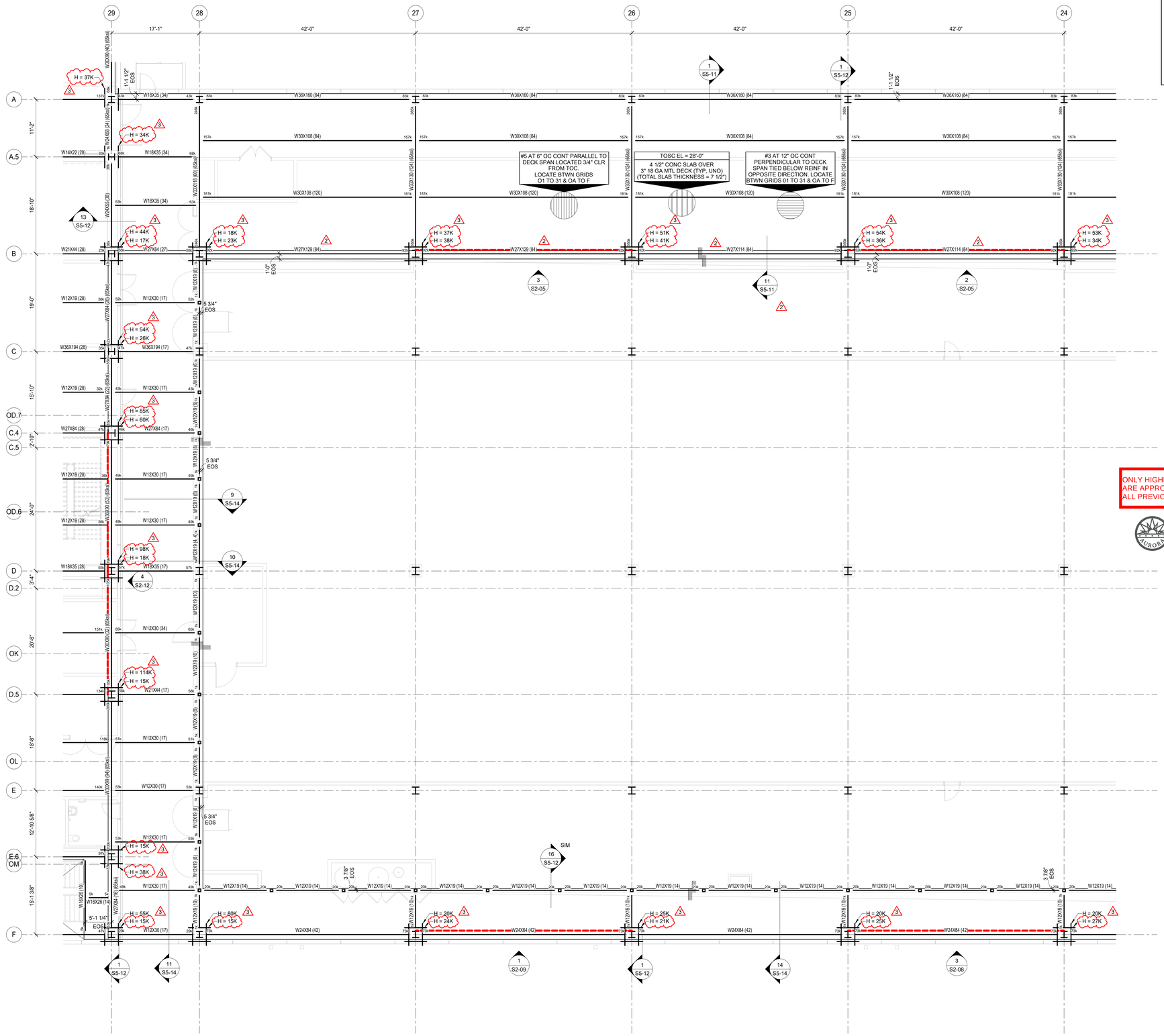
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20Pb

9/10/2025 5:03:43 PM S1-20Pb LOWER SECOND FLOOR FRAMING PLAN - SEGMENT P

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
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1 SECOND FLOOR FRAMING PLAN - SEGMENT R
1/8" = 1'-0"

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	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

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T	R	P	M	K	H	F	D	B

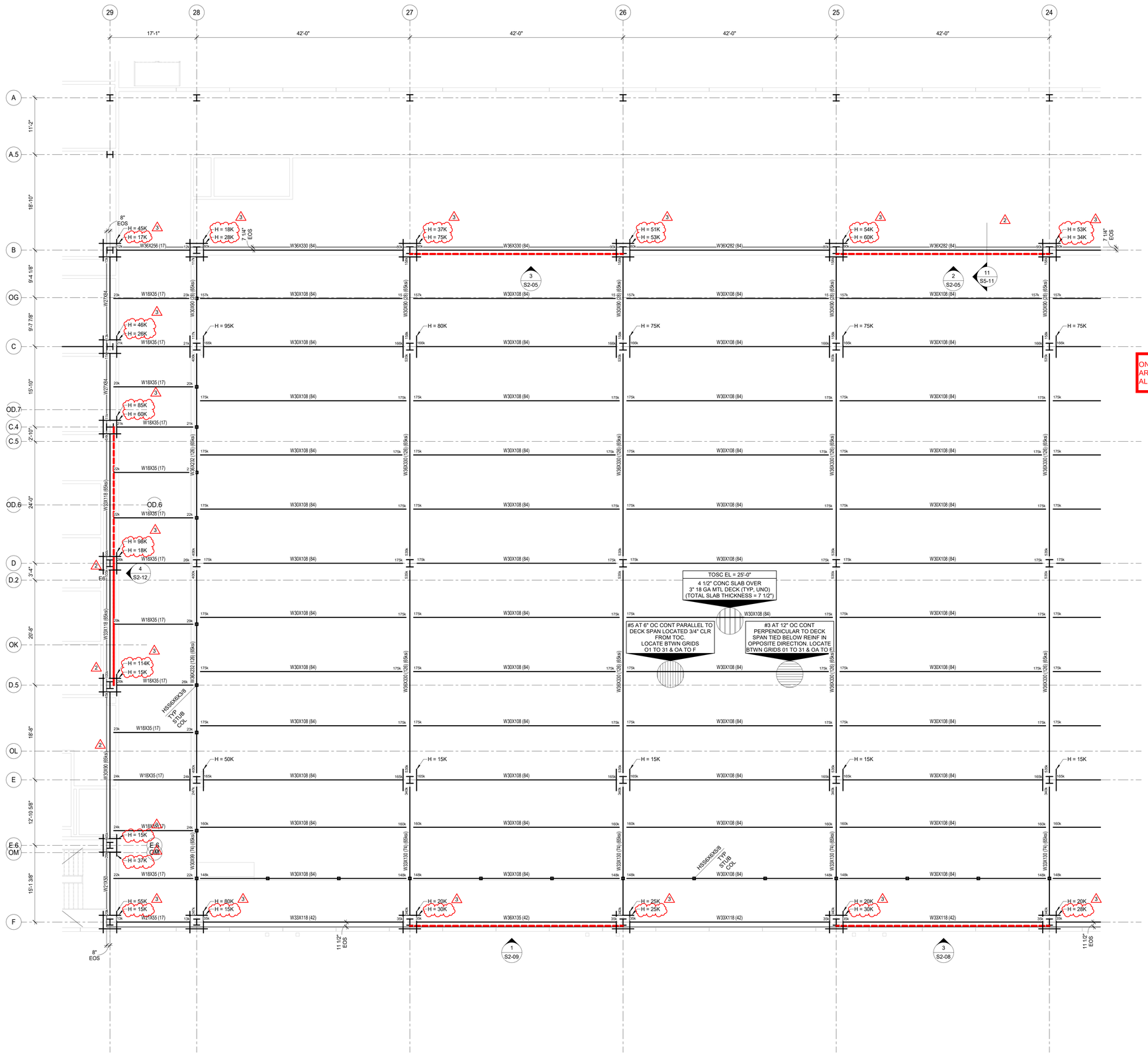
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SECOND FLOOR FRAMING PLAN - SEGMENT R

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20R

9/10/2025 5:03:46 PM S1-20R SECOND FLOOR FRAMING PLAN - SEGMENT R



1 SECOND FLOOR FRAMING PLAN - SEGMENT R-LOW
1/8" = 1'-0"

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△	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

N

LOWER SECOND FLOOR FRAMING PLAN - SEGMENT R

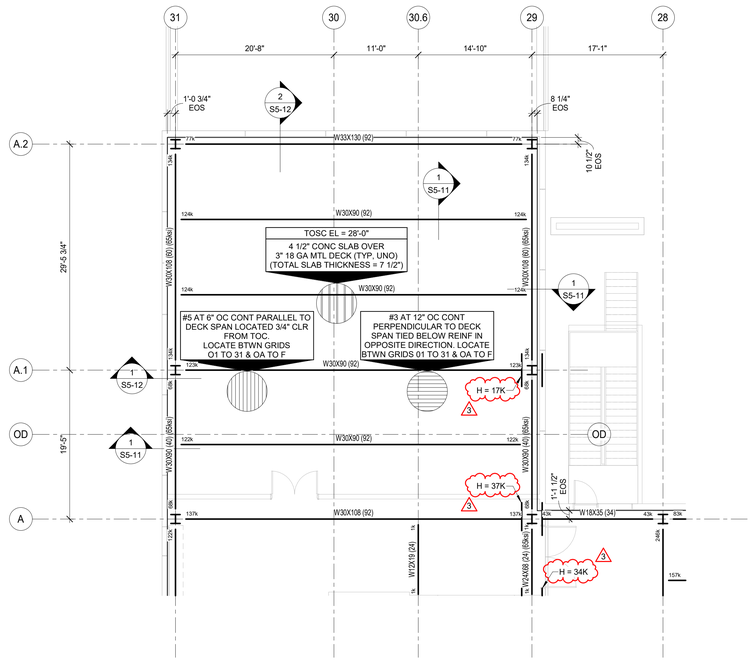
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20Rb

9/10/2025 5:03:48 PM S1-20Rb LOWER SECOND FLOOR FRAMING PLAN - SEGMENT R

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
   DATA HALL HIGH SLAB TOSC EL = 28'-0"
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4. DESIGN LIVE LOAD = REF SCHEDULE
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1 SECOND FLOOR FRAMING PLAN - SEGMENT S
1/8" = 1'-0"

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	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
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	REVISIONS	
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

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T	R	P	M	K	H	F	D	B

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SECOND FLOOR FRAMING PLAN - SEGMENT S

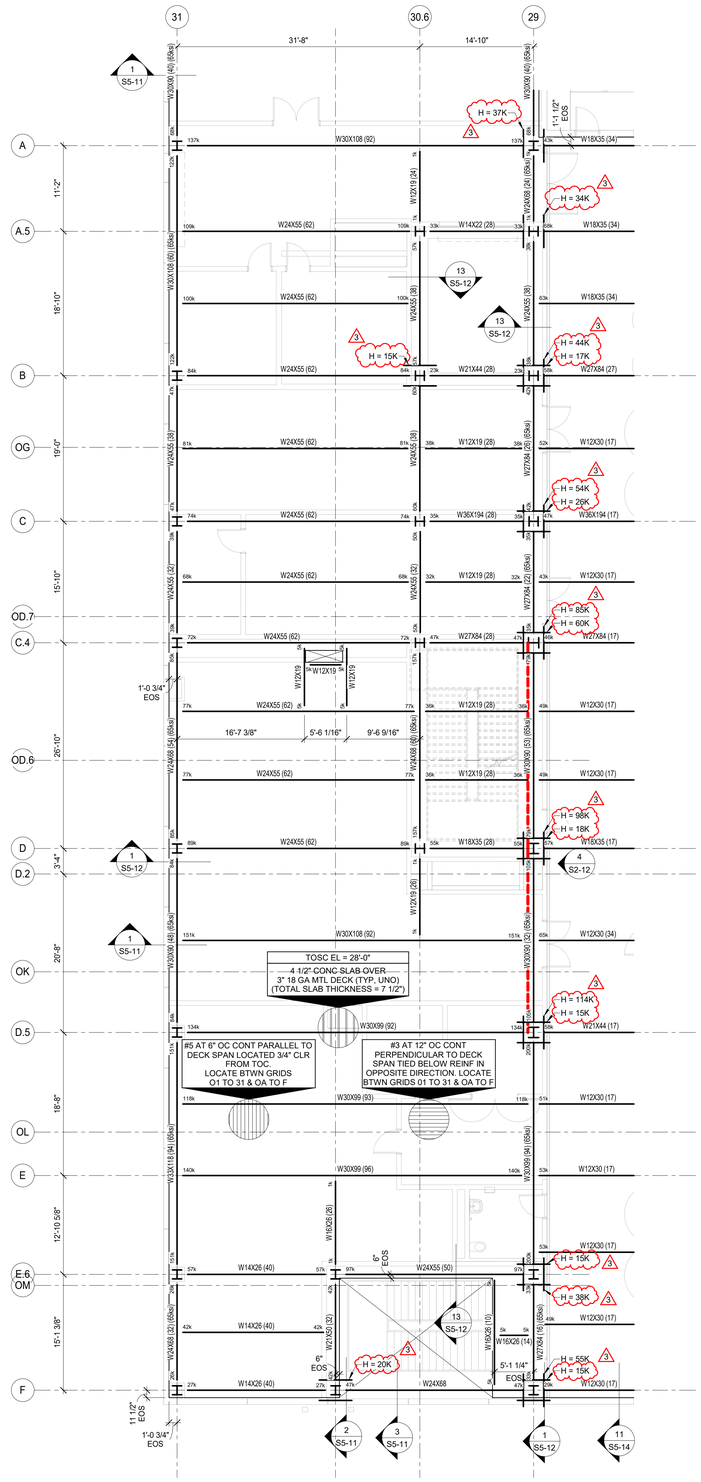
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20S

9/10/2025 5:03:51 PM S1-20S SECOND FLOOR FRAMING PLAN - SEGMENT S

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. FOH & BOH TOSC EL = 28'-0"
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1 SECOND FLOOR FRAMING PLAN - SEGMENT T
1/8" = 1'-0"

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TLD DESIGN CONSULTING LLC

ISSUES		
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REVISIONS		
1	03.28.25	ADDENDUM 1
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEYID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

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SECOND FLOOR FRAMING PLAN - SEGMENT T

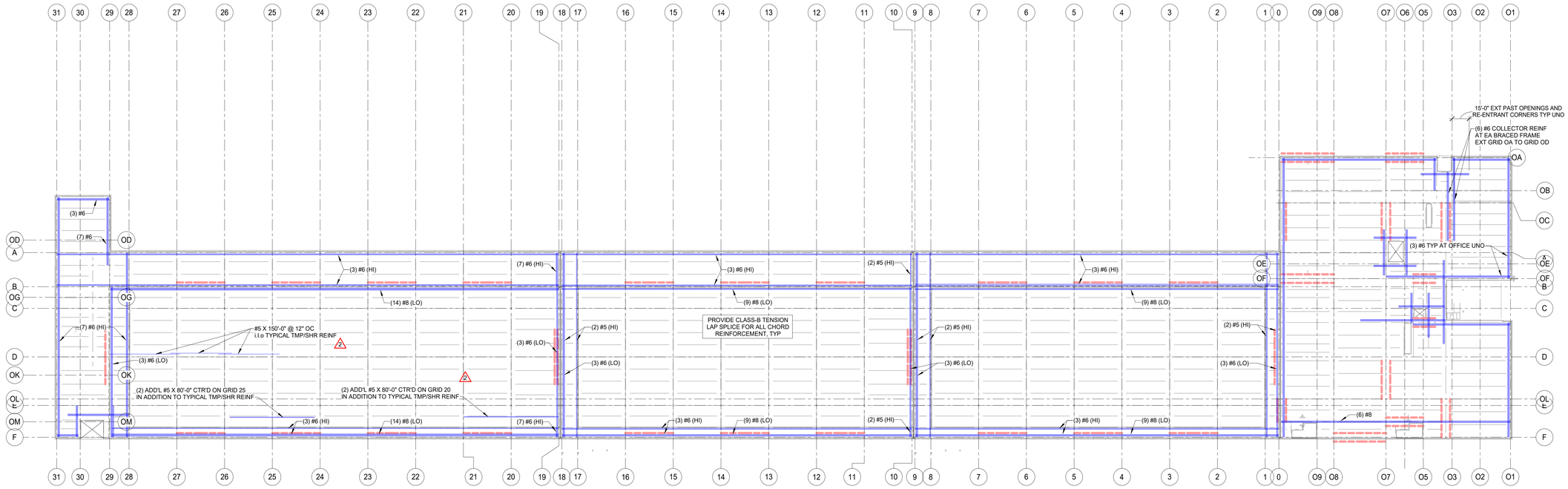
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-20T

9/10/2025 5:03:53 PM S1-20T SECOND FLOOR FRAMING PLAN - SEGMENT T

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1 SECOND FLOOR CHORD REINFORCING DIAGRAM
1" = 40'-0"

9/10/2025 5:04:00 PM S1-21 SECOND FLOOR CHORD REINFORCING DIAGRAM

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	REVISIONS	
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

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QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

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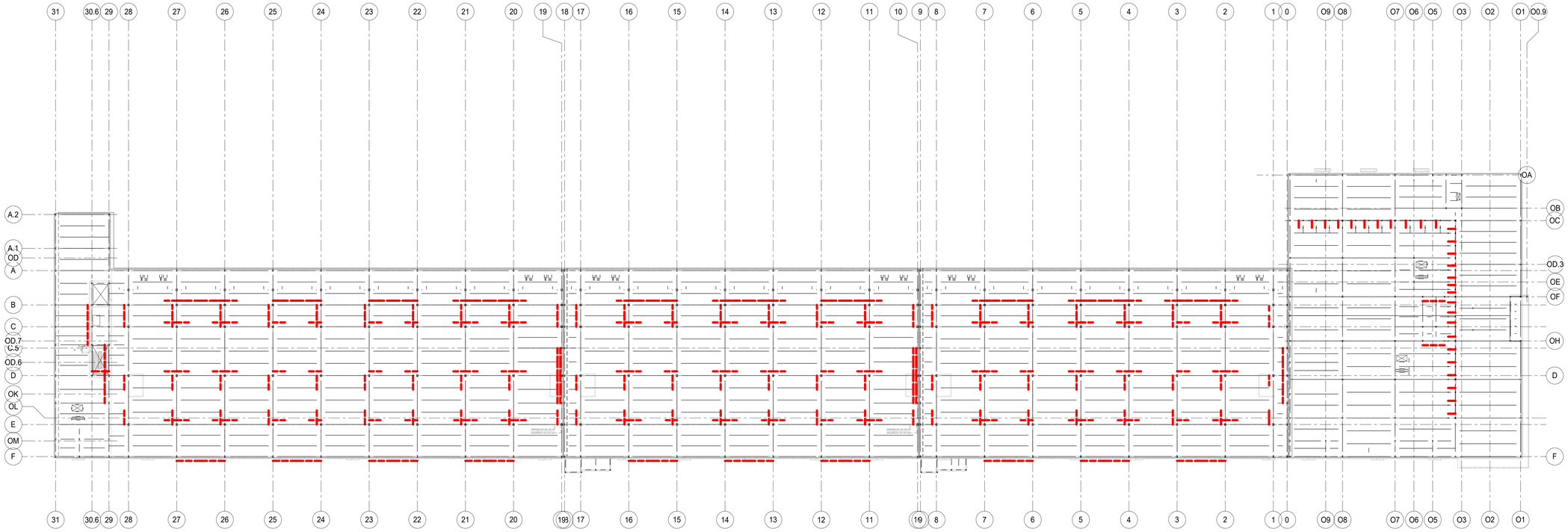
SECOND FLOOR CHORD REINFORCING DIAGRAM

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SHEET

S1-21

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1 OVER ALL ROOF FRAMING PLAN
1" = 40'-0"

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COLORADO LICENSED
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PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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QTS DEN1 DC2
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QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

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OVERALL ROOF FRAMING PLAN

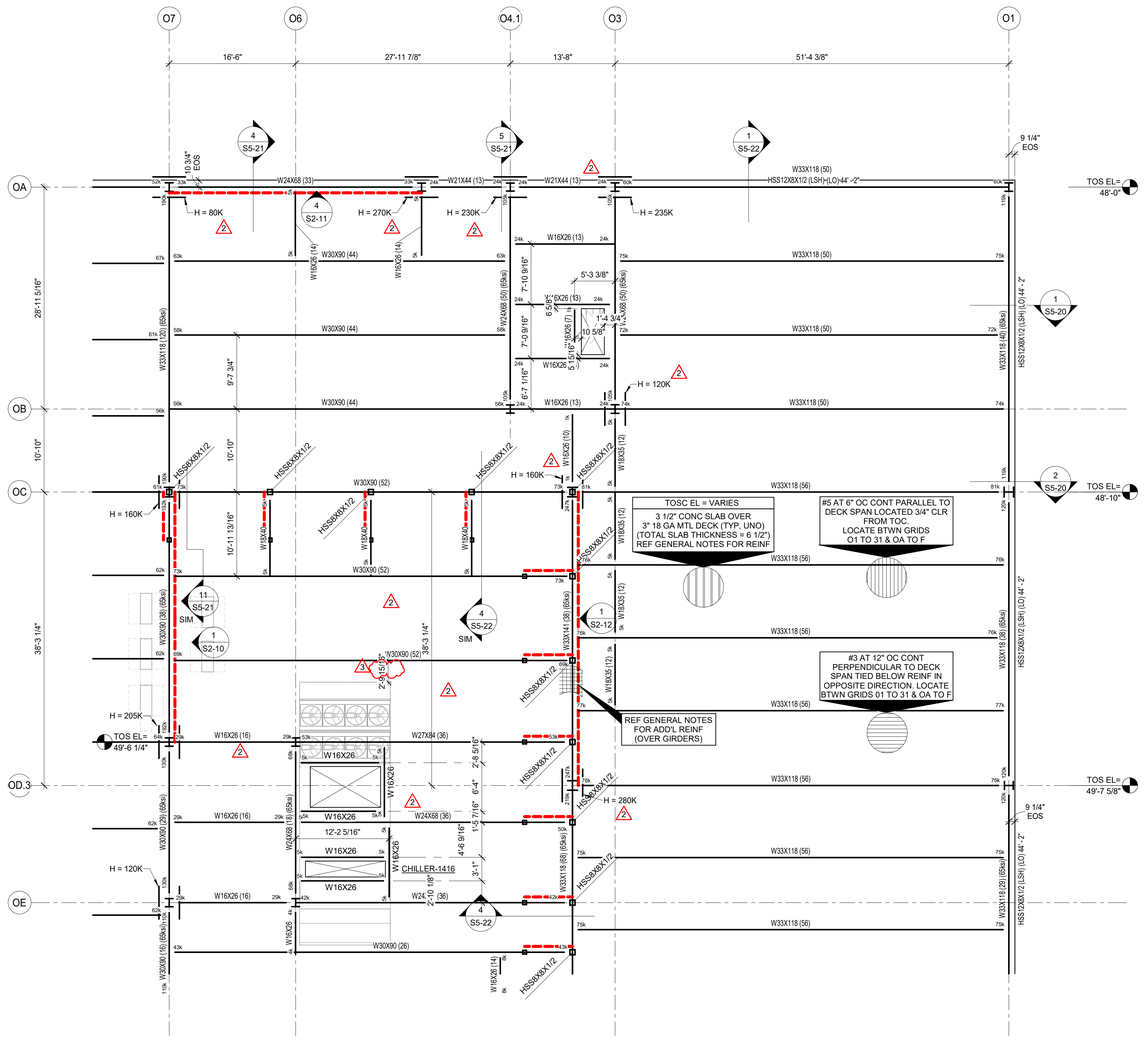
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30

9/10/2025 5:04:08 PM S1-30 OVERALL ROOF FRAMING PLAN

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOSC EL = SEE PLAN
3. REFER TO ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
4. REF ARCH FOR TOP OF WALL ELEVATIONS.
5. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
6. TOS EL = BOTTOM OF DECK.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



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1 ROOF FRAMING PLAN - SEGMENT A
1/8" = 1'-0"

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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEYID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

N

ROOF FRAMING PLAN - SEGMENT A

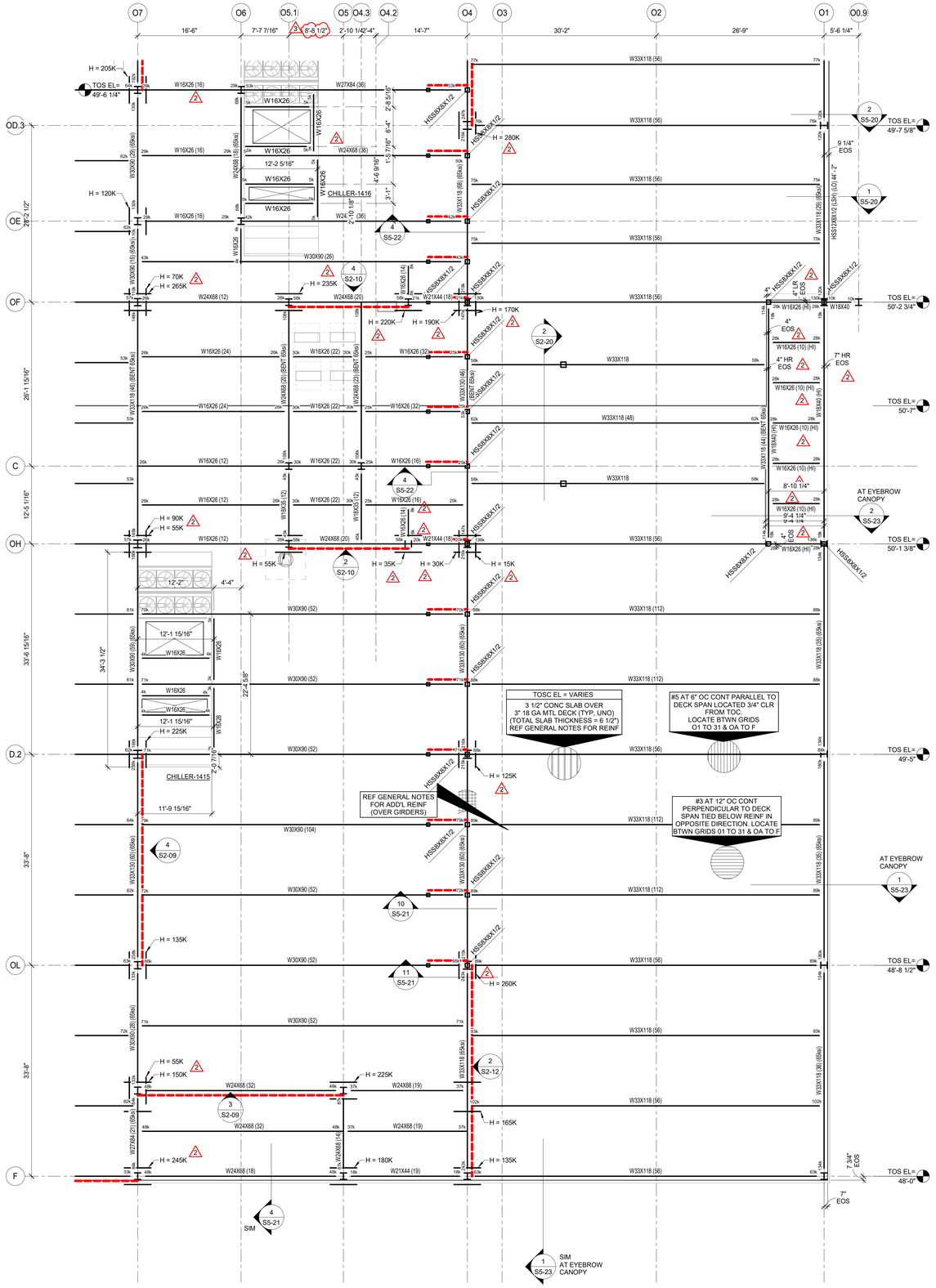
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30A

9/10/2025 5:04:12 PM S1-30A ROOF FRAMING PLAN - SEGMENT A

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOSC EL = SEE PLAN
3. REFER TO ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
4. REF ARCH FOR TOP OF WALL ELEVATIONS.
5. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
6. TOS EL = BOTTOM OF DECK.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT B
1/8" = 1'-0"

9/10/2025 5:04:17 PM S1-30B ROOF FRAMING PLAN - SEGMENT B

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

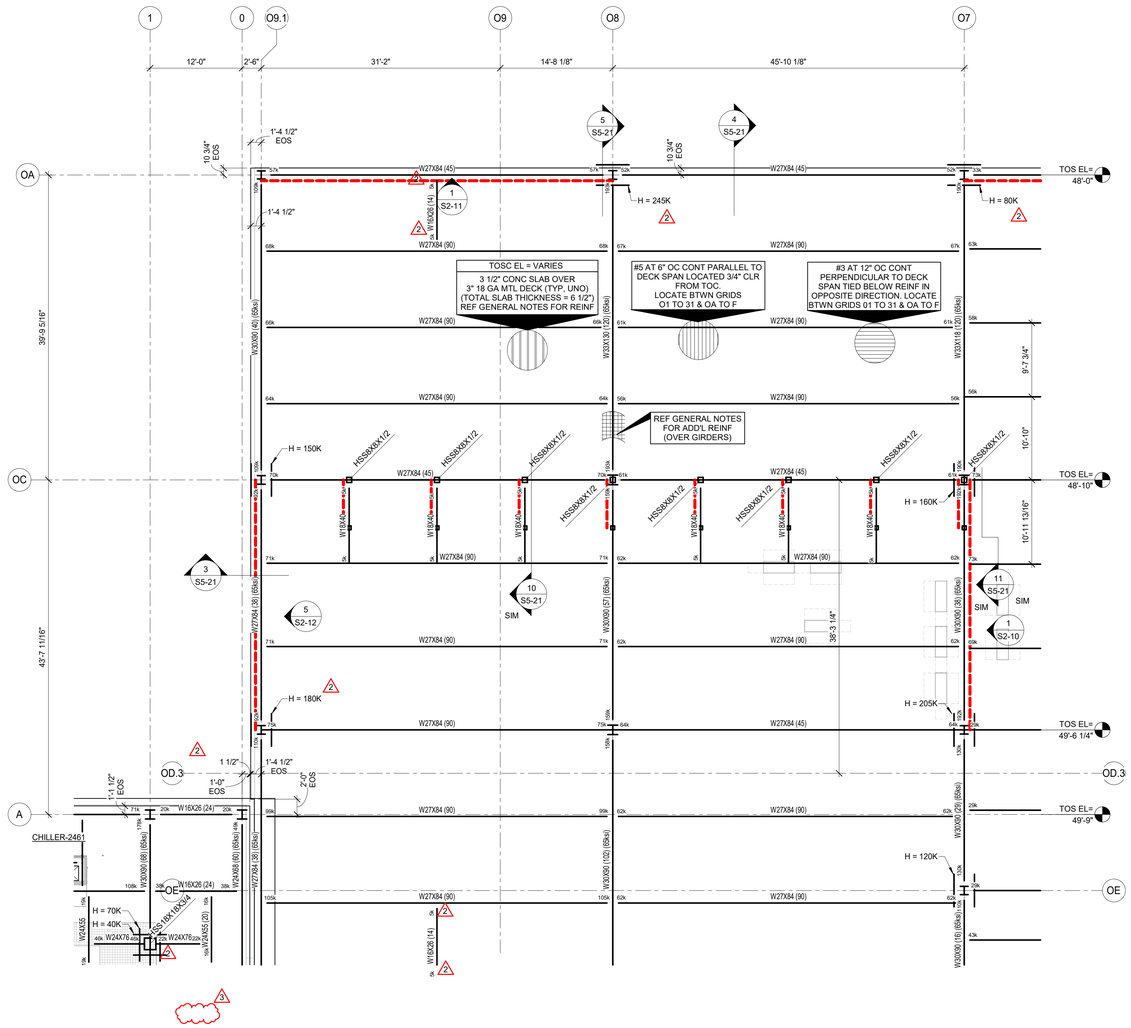
ROOF FRAMING PLAN - SEGMENT B

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30B

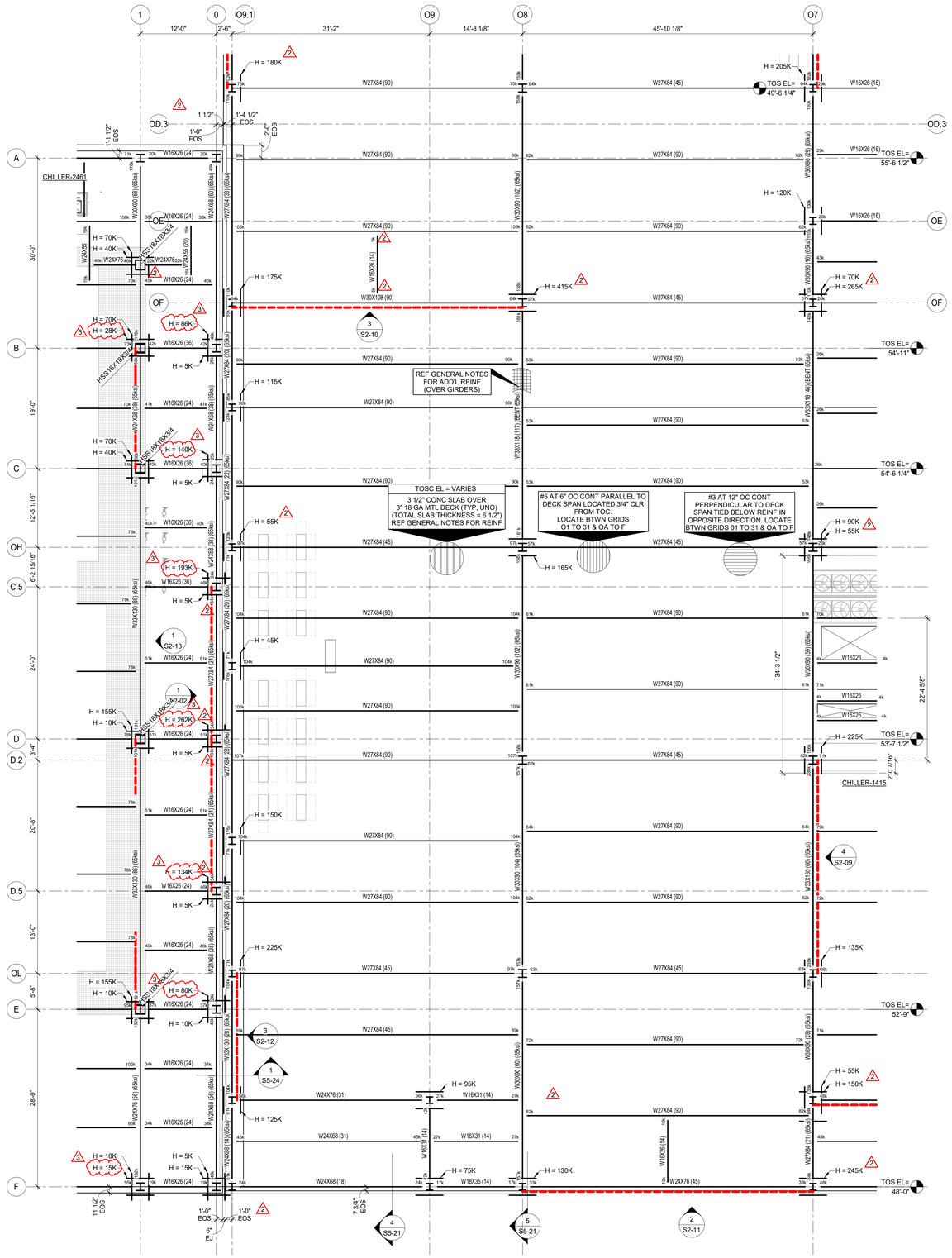
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1
0
O9.1
O9
O8
O7
12'-0"
2'-6"
31'-2"
14'-8 1/8"
45'-10 1/8"
OA
OC
OD.3
A
OE
CHILLER-2461
39'-9 5/16"
43'-7 11/16"
TOS EL= 48'-0"
TOS EL= 48'-10"
TOS EL= 49'-6 1/4"
TOS EL= 49'-9"
W27X84 (45)
W27X84 (90)
H = 245K
H = 80K
H = 150K
H = 160K
H = 180K
H = 205K
H = 120K
H = 70K
H = 40K
TOSC EL = VARIES
3 1/2" CONC SLAB OVER
3" 18 GA MTL DECK (TYP. UNO)
(TOTAL SLAB THICKNESS = 6 1/2")
REF GENERAL NOTES FOR REINF
#5 AT 6" OC CONT PARALLEL TO DECK SPAN LOCATED 3/4" CLR FROM TOC. LOCATE BTWN GRIDS O1 TO 31 & OA TO F
#3 AT 12" OC CONT PERPENDICULAR TO DECK SPAN TIED BELOW REINF IN OPPOSITE DIRECTION. LOCATE BTWN GRIDS 01 TO 31 & OA TO F
REF GENERAL NOTES FOR ADDTL REINF (OVER GIRDERS)
HSS8X8X1/2
W18X40
W16X26 (24)
W24X76
W24X55
W24X55 (20)
W24X68 (60) (65ksi)
W27X84 (90) (65ksi)
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SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOSC EL = SEE PLAN
3. REFER TO ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
4. REF ARCH FOR TOP OF WALL ELEVATIONS.
5. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
6. TOS EL = BOTTOM OF DECK.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT D
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B
N

ROOF FRAMING PLAN - SEGMENT D

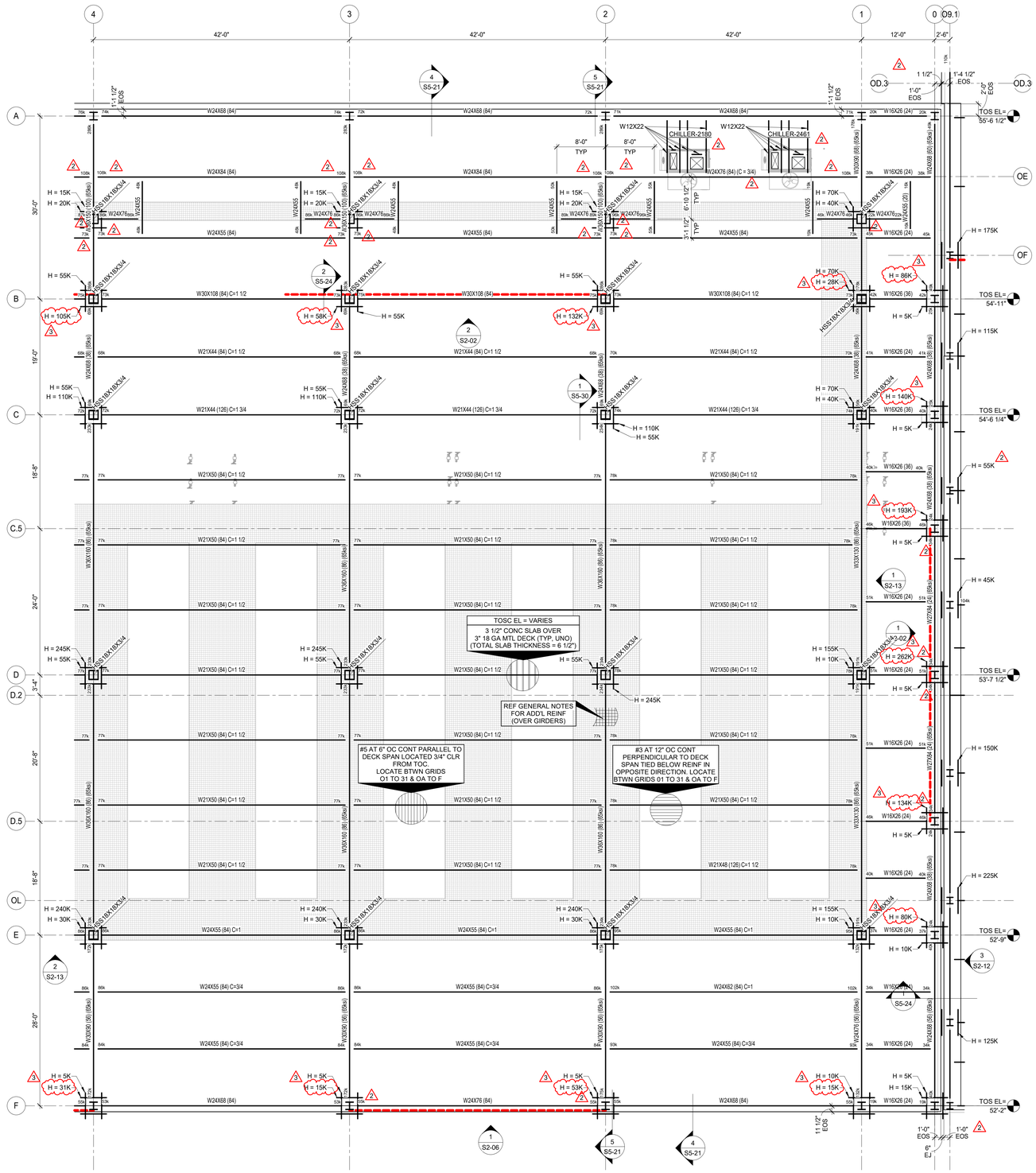
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30D

9/10/2025 5:04:25 PM S1-30D ROOF FRAMING PLAN - SEGMENT D

SHEET NOTES

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City of Aurora Building Division
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Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT F
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

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10		

REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B
N

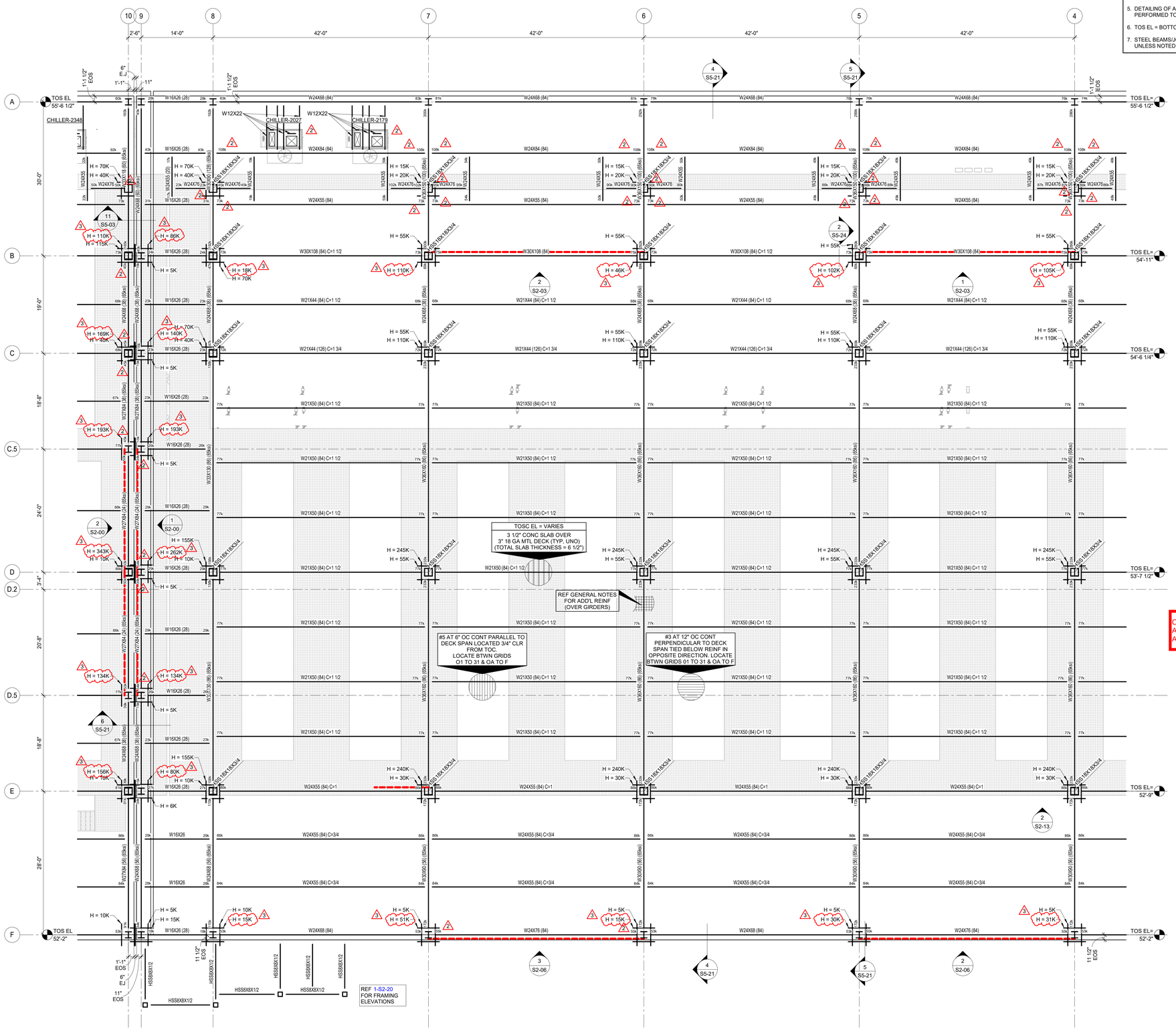
ROOF FRAMING PLAN - SEGMENT F

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30F

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
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1 ROOF FRAMING PLAN - SEGMENT H
1/8" = 1'-0"

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES		
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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

N

ROOF FRAMING PLAN - SEGMENT H

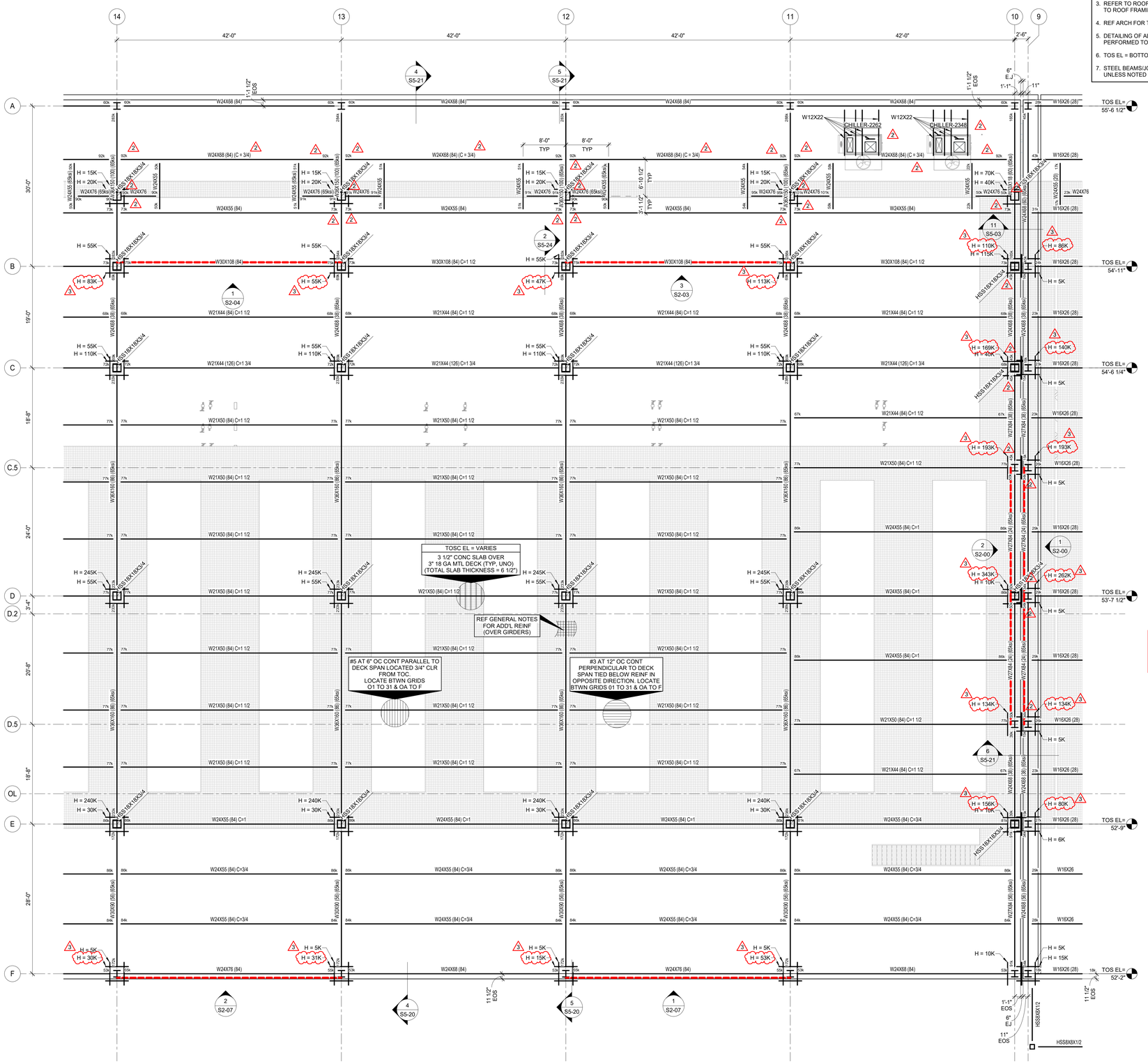
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DATE 09.10.2025
SHEET

S1-30H

9/10/2025 5:04:36 PM S1-30H ROOF FRAMING PLAN - SEGMENT H

SHEET NOTES

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1 ROOF FRAMING PLAN - SEGMENT K
1/8" = 1'-0"

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KIVLIN
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

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ROOF FRAMING PLAN - SEGMENT K

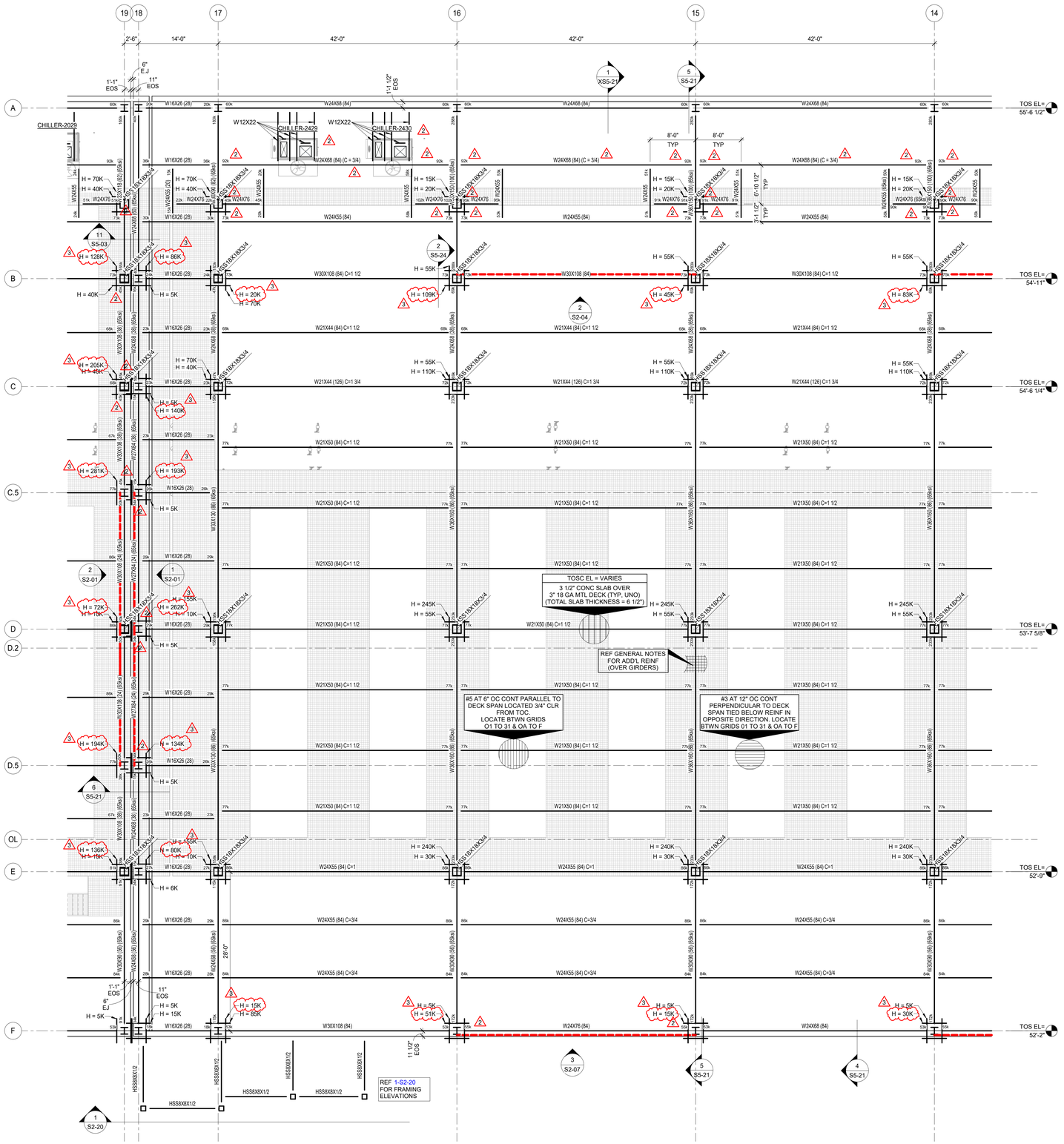
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DATE 09.10.2025
SHEET

S1-30K

9/10/2025 5:04:41 PM S1-30K ROOF FRAMING PLAN - SEGMENT K

SHEET NOTES

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3. REFER TO ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
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5. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
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ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW.
ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT M
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEYID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

N

ROOF FRAMING PLAN - SEGMENT M

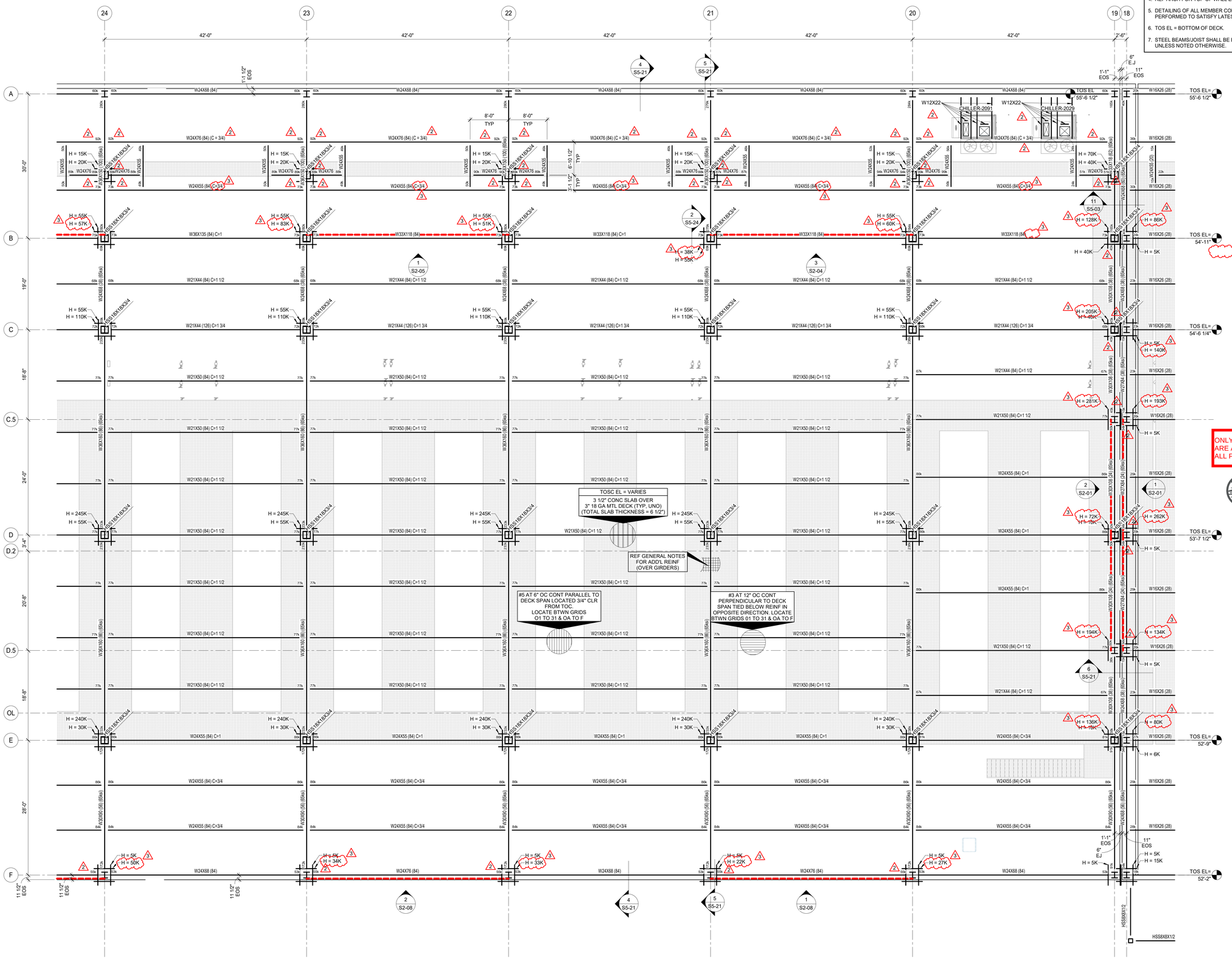
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30M

9/10/2025 5:04:47 PM S1-30M ROOF FRAMING PLAN - SEGMENT M

SHEET NOTES

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT P
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

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REVISIONS

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COLORADO LICENSED NICHOLAS W. KIVLIN 65236 PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue: 09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

N

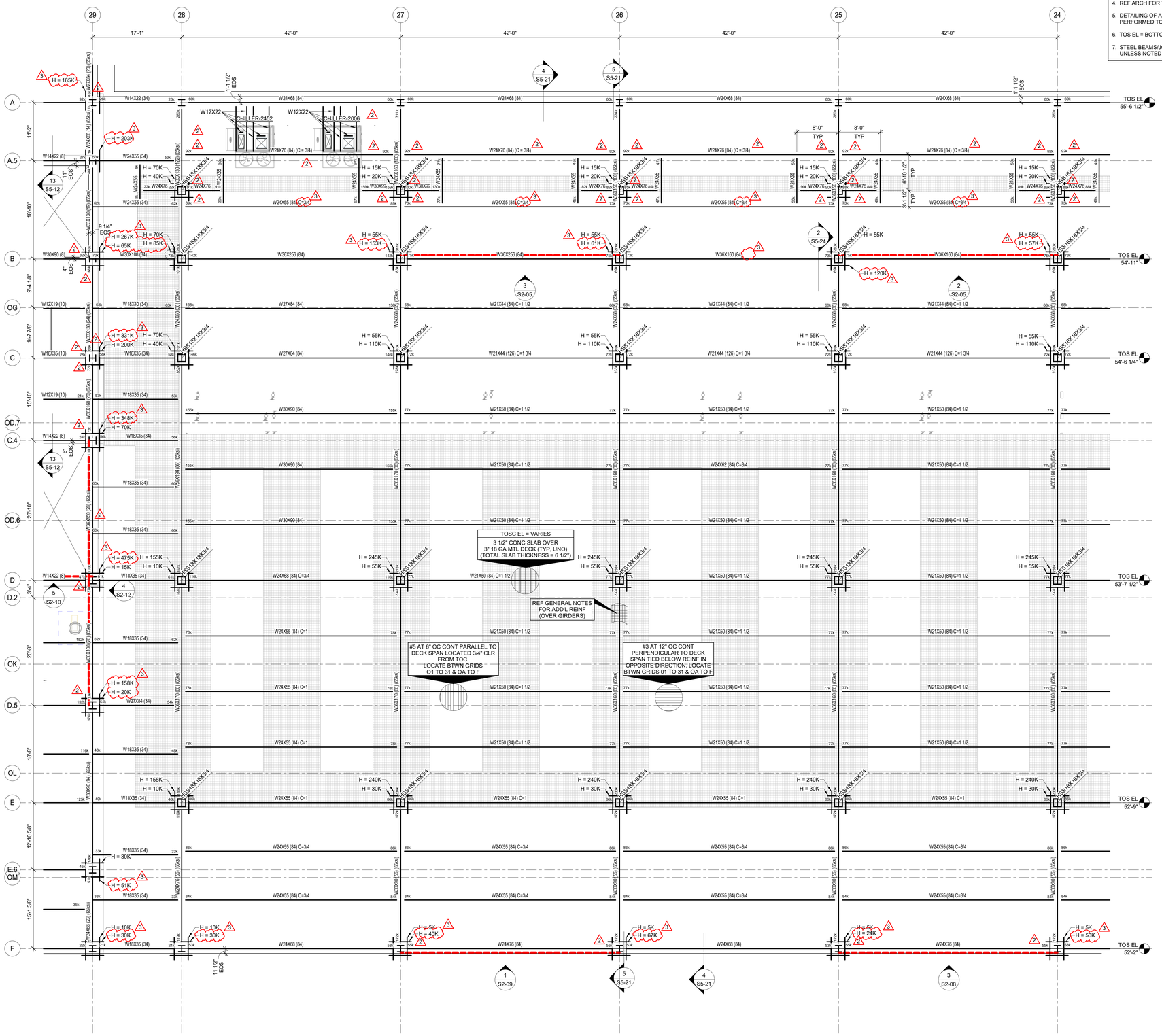
ROOF FRAMING PLAN - SEGMENT P

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30P

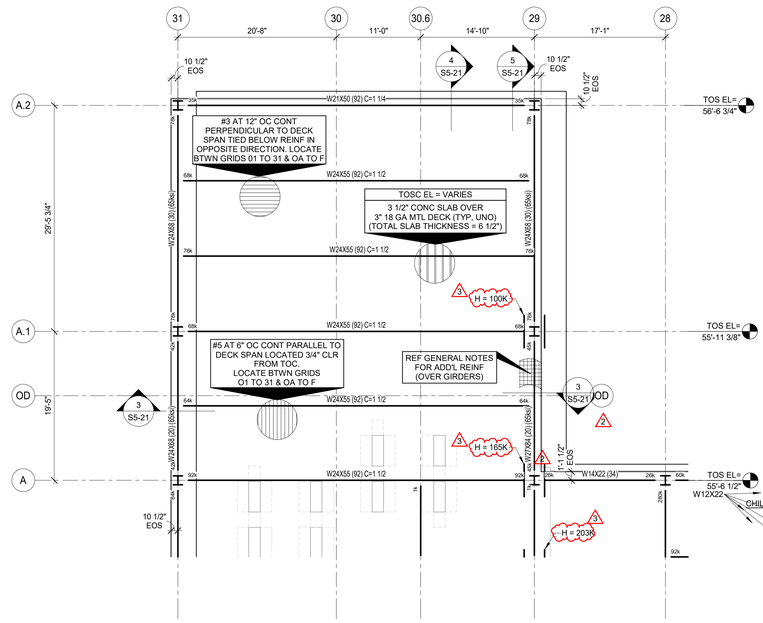
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29
28
27
26
25
24
17'-1"
42'-0"
A
A.5
B
OG
C
OD.7
C.4
OD.6
D
D.2
OK
D.5
OL
E
E.6
OM
F
TOS EL 55'-6 1/2"
TOS EL 54'-11"
TOS EL 54'-6 1/4"
TOS EL 53'-7 1/2"
TOS EL 52'-9"
TOS EL 52'-2"
CHILLER-2452
CHILLER-2006
W12X22
W24X68 (84)
W24X76 (84) (C = 3/4)
W24X55 (84) C=3/4
W24X55 (84) C=1
W36X256 (84)
W36X160 (84)
W27X84 (84)
W21X44 (84) C=1 1/2
W21X44 (126) C=1 3/4
W21X50 (84) C=1 1/2
W30X90 (84)
W24X68 (84) C=3/4
W24X62 (84) C=3/4
W24X76 (84)
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W40X266215 (84)
W40X266392 (84)

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOSC EL = SEE PLAN
3. REFER TO ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
4. REF ARCH FOR TOP OF WALL ELEVATIONS.
5. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
6. TOS EL = BOTTOM OF DECK.
7. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.



ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT S
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

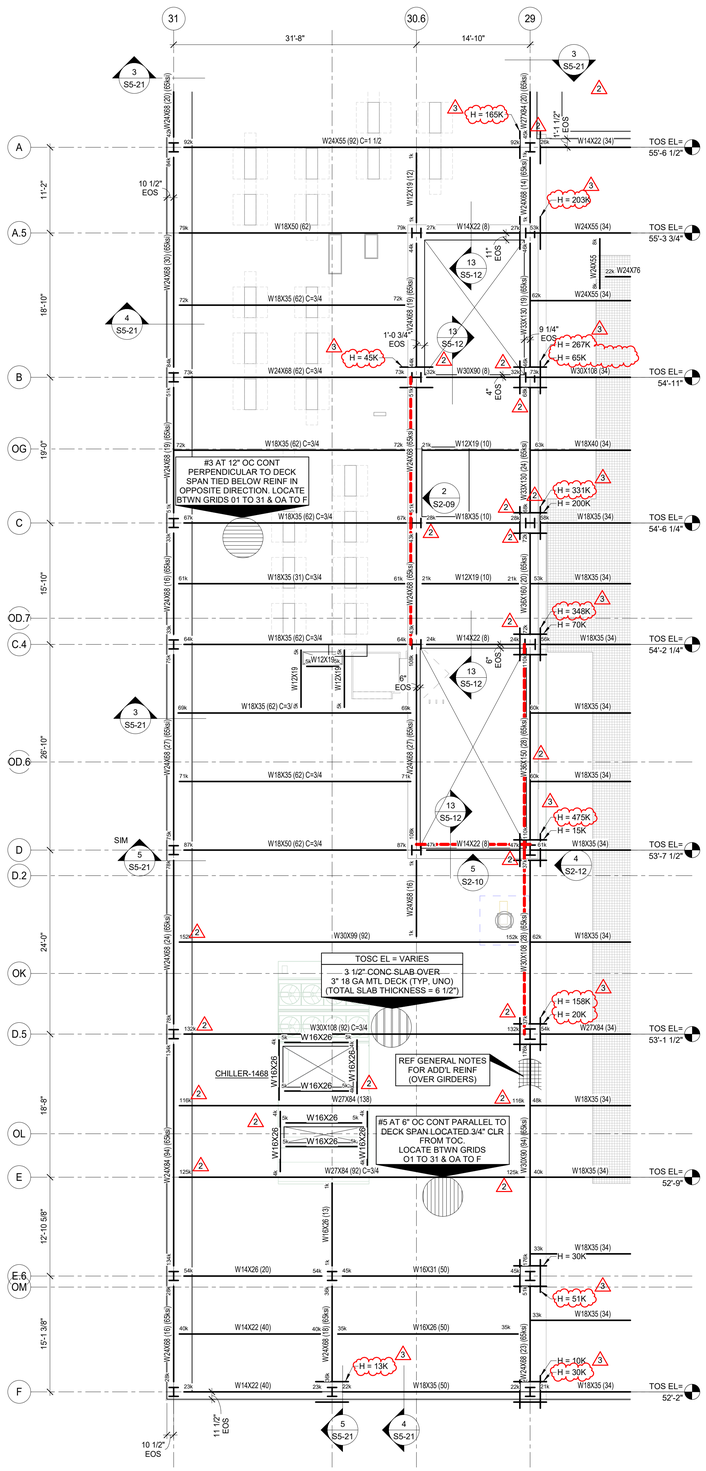
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ROOF FRAMING PLAN - SEGMENT S

JOB 24464.0000
DATE 09.10.2025
SHEET S1-30S

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOSC EL = SEE PLAN
3. REFER TO ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
4. REF ARCH FOR TOP OF WALL ELEVATIONS.
5. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
6. TOS EL = BOTTOM OF DECK.
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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

1 ROOF FRAMING PLAN - SEGMENT T
1/8" = 1'-0"

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES		
1	12.20.24	DESIGN DEVELOPMENT
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7		
8		
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10		

REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. KEYID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

N

ROOF FRAMING PLAN - SEGMENT T

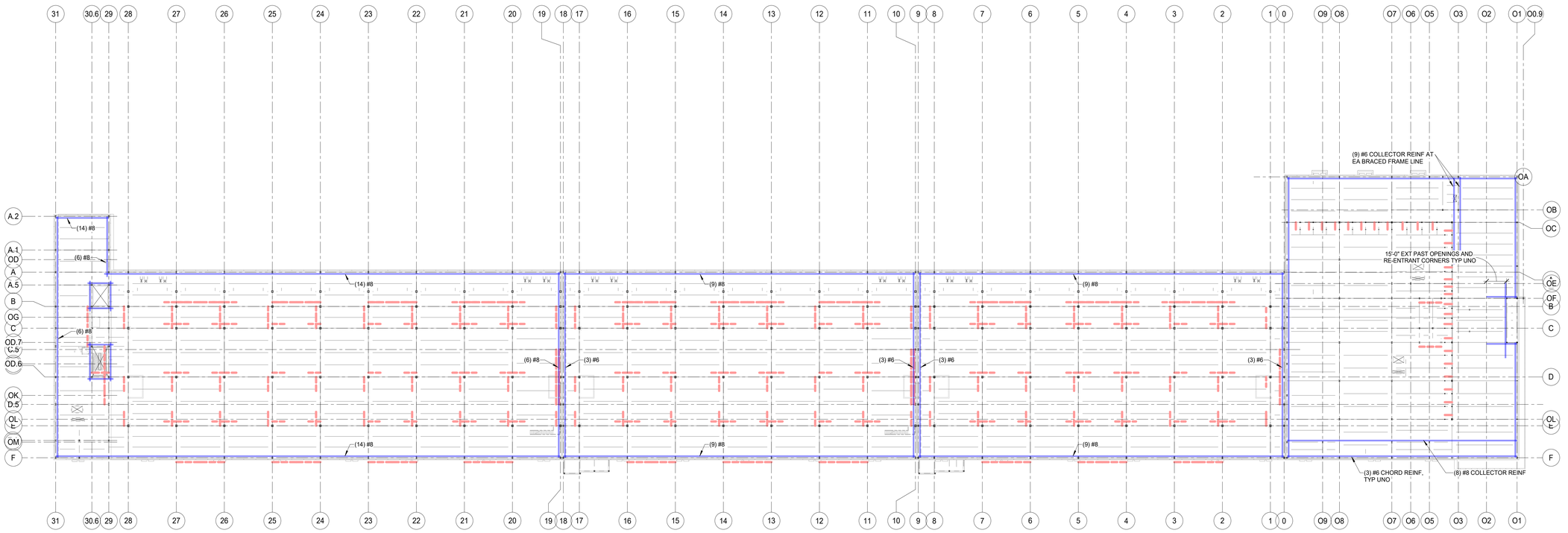
JOB 24464.0000
DATE 09.10.2025
SHEET

S1-30T

9/10/2025 5:05:06 PM S1-30T ROOF FRAMING PLAN - SEGMENT T

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City of Aurora Building Division
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Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



1 ROOF CHORD REINFORCING DIAGRAM
1" = 40'-0"

9/10/2025 5:05:14 PM S1-31 ROOF CHORD REINFORCING DIAGRAM

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401 N. Houston Street Dallas, TX
75202
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SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

	ISSUES	
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7		
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	REVISIONS	

COLORADO LICENSED
NICHOLAS W. HAYID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

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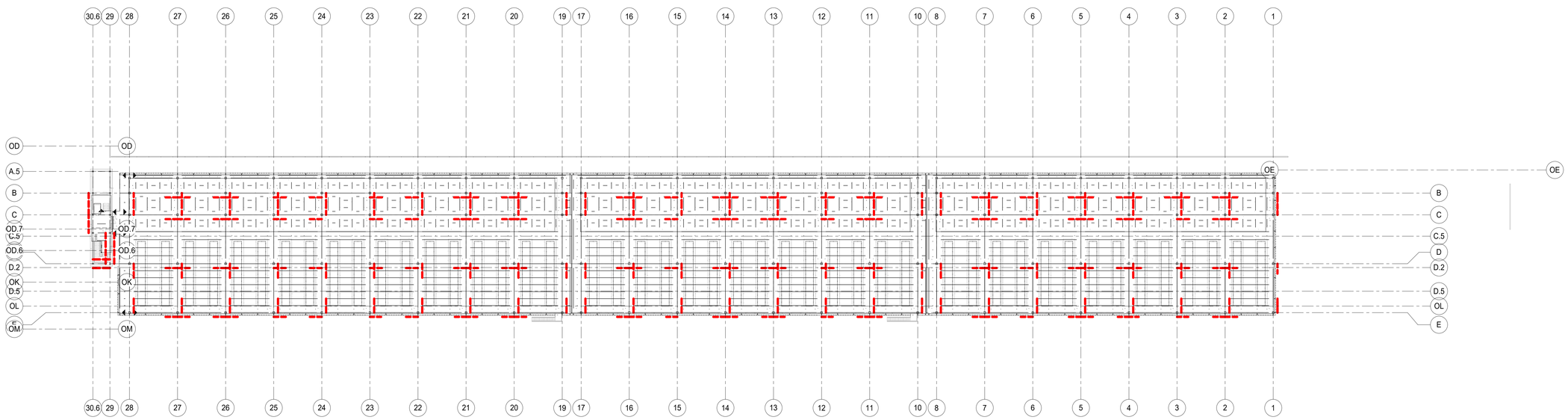
ROOF CHORD REINFORCING DIAGRAM

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-31

ONLY HIGHLITED OR CLOUDED REVISIONS
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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



1 OVERALL PLATFORM FRAMING PLAN
1" = 40'-0"

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401 N. Houston Street Dallas, TX
75202
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SYSKA HENNESSY GROUP

TLC DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
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4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
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	REVISIONS	

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

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OVERALL PLATFORM FRAMING PLAN

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40

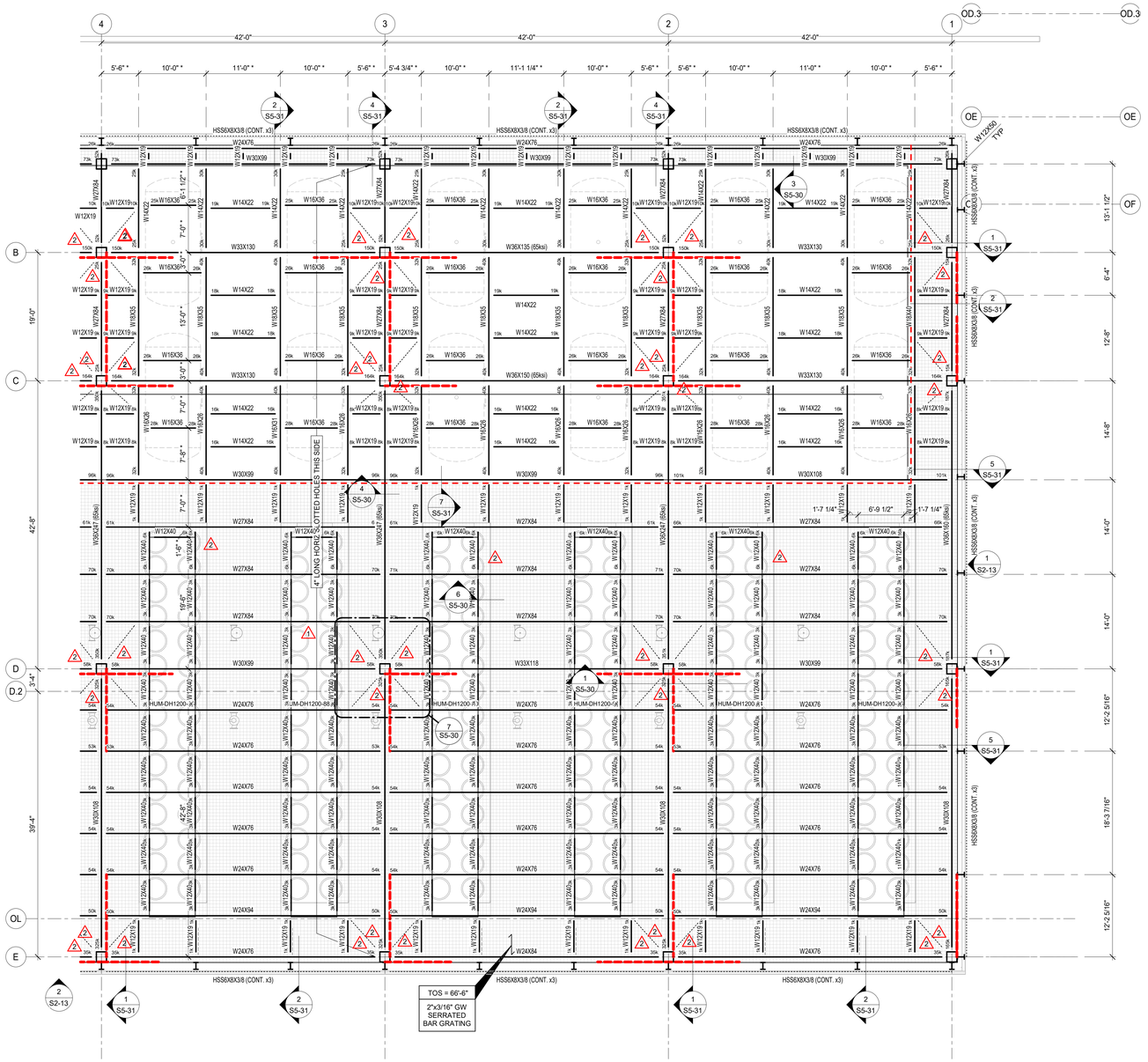
9/10/2025 5:05:32 PM S1-40 OVERALL PLATFORM FRAMING PLAN

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOS EL = SEE PLAN
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. TOS EL = BOTTOM OF GRATING
5. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.
6. GC TO COORDINATE SPACING OF CHILLER SUPPORT SKID BEAMS WITH MANUFACTURER.
7. DIMENSIONS DENOTED * REQUIRE GC VERIFICATION WITH PROCURED EQUIPMENT. REF DETAIL 6/S5-30 FOR CLARITY ON CHILLER ISOLATOR CONNECTION.
8. ALL BEAM CONNECTIONS TO BE DESIGNED FOR COMBINED AXIAL AND MINOR AXIS SHEAR LOADS (LRFD) AS NOTED IN THE TABLE BELOW. REF PLANS FOR MAJOR AXIS SHEAR (LRFD).
9. HORIZONTAL BRACE L5x5x5/16 "H" FORCE = +/-30K (LRFD) TYP.

BEAM AXIAL AND MINOR SHEAR FORCES (LRFD)

BEAM SIZE	AXIAL FORCE	MINOR AXIS SHEAR
W12X19	±17K	0K
W12X40	±21K	±5K
W14X22	±18K	±1K
W16X26	±12K	±5K
W16X31	±5K	±1K
W16X36	±8K	±0K
W18X35	±16K	±1K
W18X40	±12K	±1K
W24X62	±8K	±1K
W24X68	±22K	±4K
W24X76	±63K	±12K
W24X84	±76K	±2K
W24X94	±23K	±6K
W27X84	±146K	±11K
W30X99	±73K	±23K
W30X108	±178K	±12K
W33X118	±156K	±14K
W33X130	±183K	±10K
W36X135	±170K	±10K
W36X150	±187K	±4K
W36X160	±17K	±6K
W36X170	±27K	±3K
W36X247	±41K	±13K



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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

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401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KIVLIN
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

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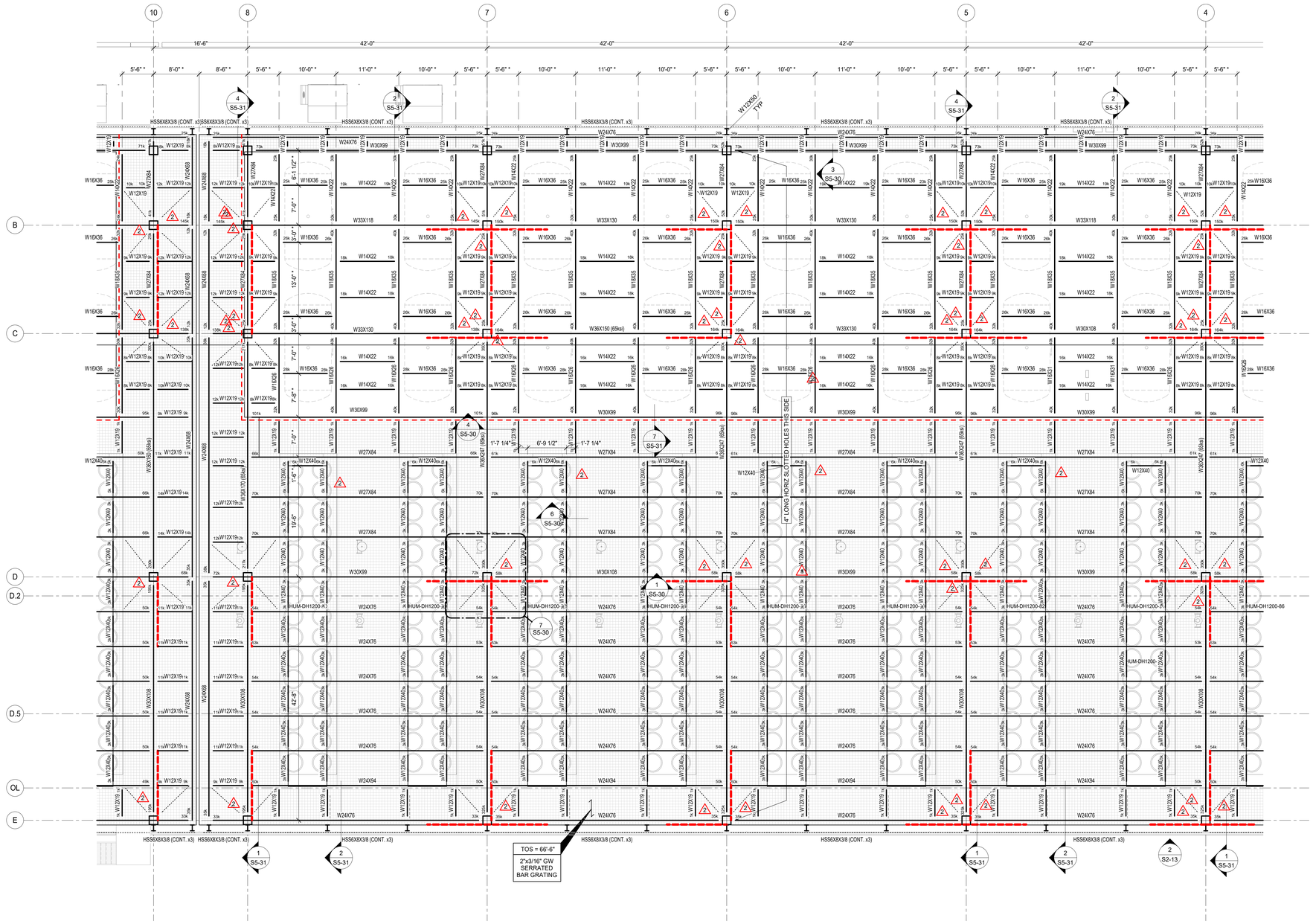
PLATFORM FRAMING - SEGMENT F

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40F

1 PLATFORM FRAMING PLAN - SEGMENT F
1/8" = 1'-0"

9/10/2025 5:05:37 PM S1-40F PLATFORM FRAMING - SEGMENT F



1 PLATFORM FRAMING PLAN - SEGMENT H
1/8" = 1'-0"

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOS EL = SEE PLAN
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. TOS EL = BOTTOM OF GRATING
5. STEEL BEAMS/JOIST SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.
6. GC TO COORDINATE SPACING OF CHILLER SUPPORT SKID BEAMS WITH MANUFACTURER.
7. DIMENSIONS DENOTED * REQUIRE GC VERIFICATION WITH PROCURED EQUIPMENT. REF DETAIL 6/S5-30 FOR CLARITY ON CHILLER ISOLATOR CONNECTION.
8. ALL BEAM CONNECTIONS TO BE DESIGNED FOR COMBINED AXIAL AND MINOR AXIS SHEAR LOADS (LRFD) AS NOTED IN THE TABLE BELOW. REF PLANS FOR MAJOR AXIS SHEAR (LRFD).
9. HORIZONTAL BRACE L5x5x5/16 "H" FORCE = +/-30K (LRFD) TYP.

BEAM AXIAL AND MINOR SHEAR FORCES (LRFD)

BEAM SIZE	AXIAL FORCE	MINOR AXIS SHEAR
W12X19	±17K	0K
W12X40	±21K	±5K
W14X22	±18K	±1K
W16X26	±12K	±5K
W16X31	±5K	±1K
W16X36	±8K	±0K
W18X35	±16K	±1K
W18X40	±12K	±1K
W24X62	±8K	±1K
W24X68	±22K	±4K
W24X76	±63K	±12K
W24X84	±76K	±2K
W24X94	±23K	±6K
W27X84	±146K	±11K
W30X99	±73K	±23K
W30X108	±178K	±12K
W33X118	±156K	±14K
W33X130	±183K	±10K
W36X135	±170K	±10K
W36X150	±187K	±4K
W36X160	±17K	±6K
W36X170	±27K	±3K
W36X247	±41K	±13K

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KUYLE
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

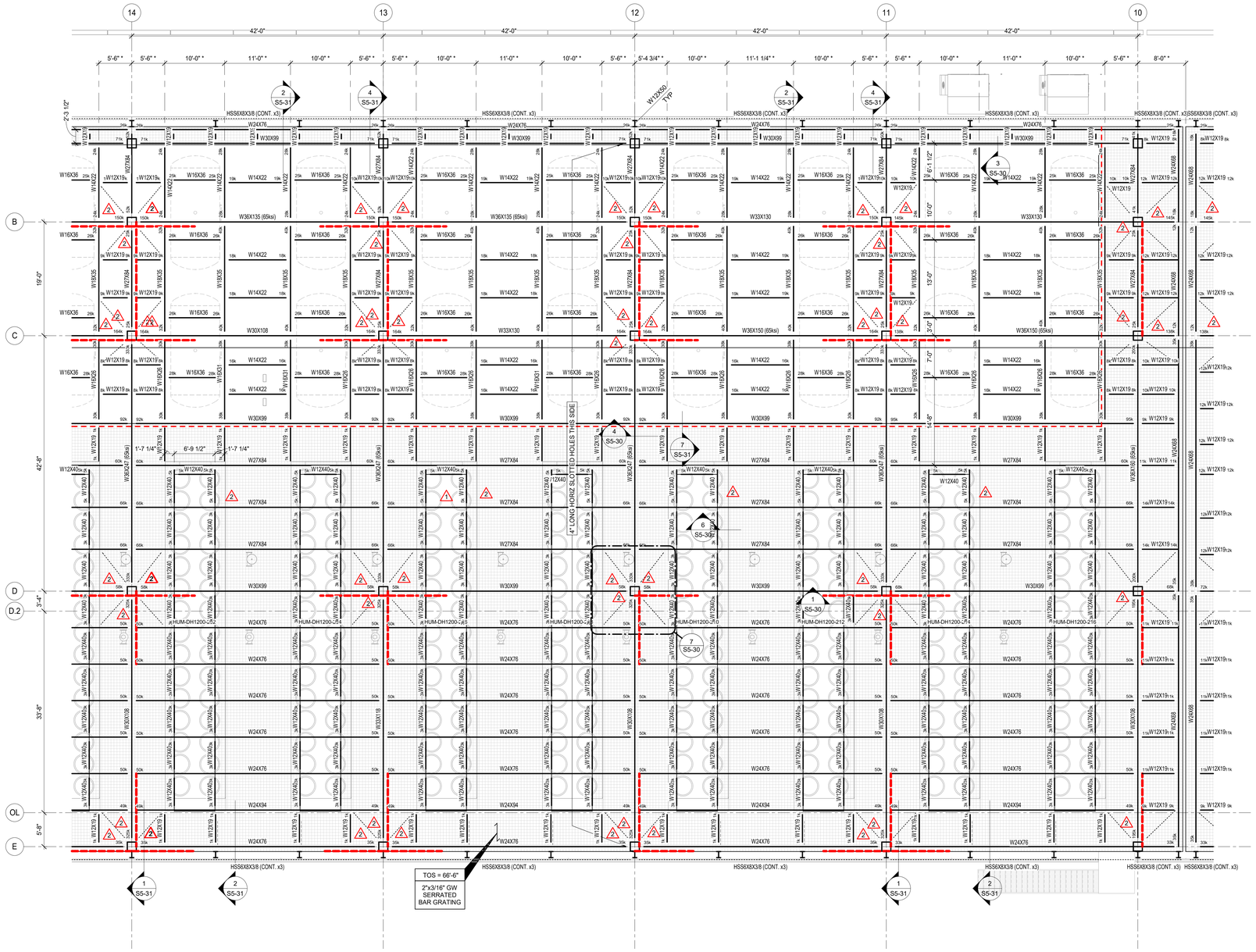
N

PLATFORM FRAMING - SEGMENT H

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40H

9/10/2025 5:05:45 PM S1-40H PLATFORM FRAMING - SEGMENT H



1 PLATFORM FRAMING PLAN - SEGMENT K
1/8" = 1'-0"

SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOS EL = SEE PLAN
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8. ALL BEAM CONNECTIONS TO BE DESIGNED FOR COMBINED AXIAL AND MINOR AXIS SHEAR LOADS (LRFD) AS NOTED IN THE TABLE BELOW. REF PLANS FOR MAJOR AXIS SHEAR (LRFD).
9. HORIZONTAL BRACE L5x5x5/16 "H" FORCE = +/-30K (LRFD) TYP.

2

BEAM AXIAL AND MINOR SHEAR FORCES (LRFD)

BEAM SIZE	AXIAL FORCE	MINOR AXIS SHEAR
W12X19	±17K	0K
W12X40	±21K	±5K
W14X22	±18K	±1K
W16X26	±12K	±5K
W16X31	±5K	±1K
W16X36	±8K	±0K
W18X35	±16K	±1K
W18X40	±12K	±1K
W24X62	±8K	±1K
W24X68	±22K	±4K
W24X76	±63K	±12K
W24X84	±76K	±2K
W24X94	±23K	±6K
W27X84	±146K	±11K
W30X99	±73K	±23K
W30X108	±178K	±12K
W33X118	±156K	±14K
W33X130	±183K	±10K
W36X135	±170K	±10K
W36X150	±187K	±4K
W36X160	±17K	±6K
W36X170	±27K	±3K
W36X247	±41K	±13K

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES		
1	12.20.24	DESIGN DEVELOPMENT
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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KNUD
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

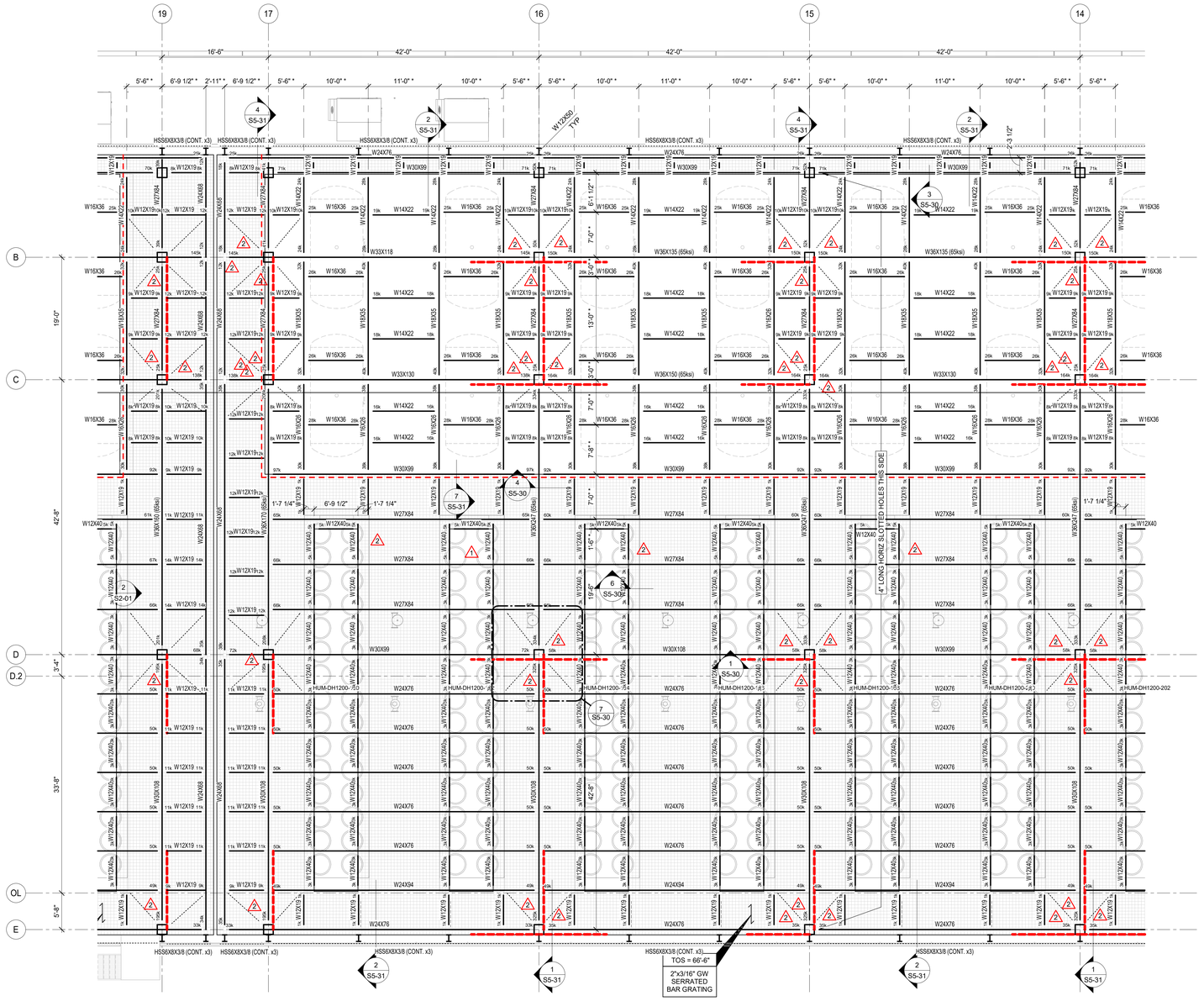
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PLATFORM FRAMING - SEGMENT K

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40K

9/10/2025 5:05:52 PM S1-40K PLATFORM FRAMING - SEGMENT K



SHEET NOTES

1. REFER TO SHEETS S0 SERIES AND S5 SERIES FOR GENERAL NOTES, SCHEDULES, AND DESIGN CRITERIA.
2. ALL ELEVATIONS ARE BASED ON TOS EL = SEE PLAN
3. DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
4. TOS EL = BOTTOM OF GRATING
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8. ALL BEAM CONNECTIONS TO BE DESIGNED FOR COMBINED AXIAL AND MINOR AXIS SHEAR LOADS (LRFD) AS NOTED IN THE TABLE BELOW. REF PLANS FOR MAJOR AXIS SHEAR (LRFD).
9. HORIZONTAL BRACE L5x5x5/16 "H" FORCE = +/-30K (LRFD) TYP.

BEAM AXIAL AND MINOR SHEAR FORCES (LRFD)

BEAM SIZE	AXIAL FORCE	MINOR AXIS SHEAR
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W12X40	±21K	±5K
W14X22	±18K	±1K
W16X26	±12K	±5K
W16X31	±5K	±1K
W16X36	±8K	±0K
W18X35	±16K	±1K
W18X40	±12K	±1K
W24X62	±8K	±1K
W24X68	±22K	±4K
W24X76	±63K	±12K
W24X84	±76K	±2K
W24X94	±23K	±6K
W27X84	±146K	±11K
W30X99	±73K	±23K
W30X108	±178K	±12K
W33X118	±156K	±14K
W33X130	±183K	±10K
W36X135	±170K	±10K
W36X150	±187K	±4K
W36X160	±17K	±6K
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COLORADO LICENSED NICHOLAS W. KEVIL 65236 PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

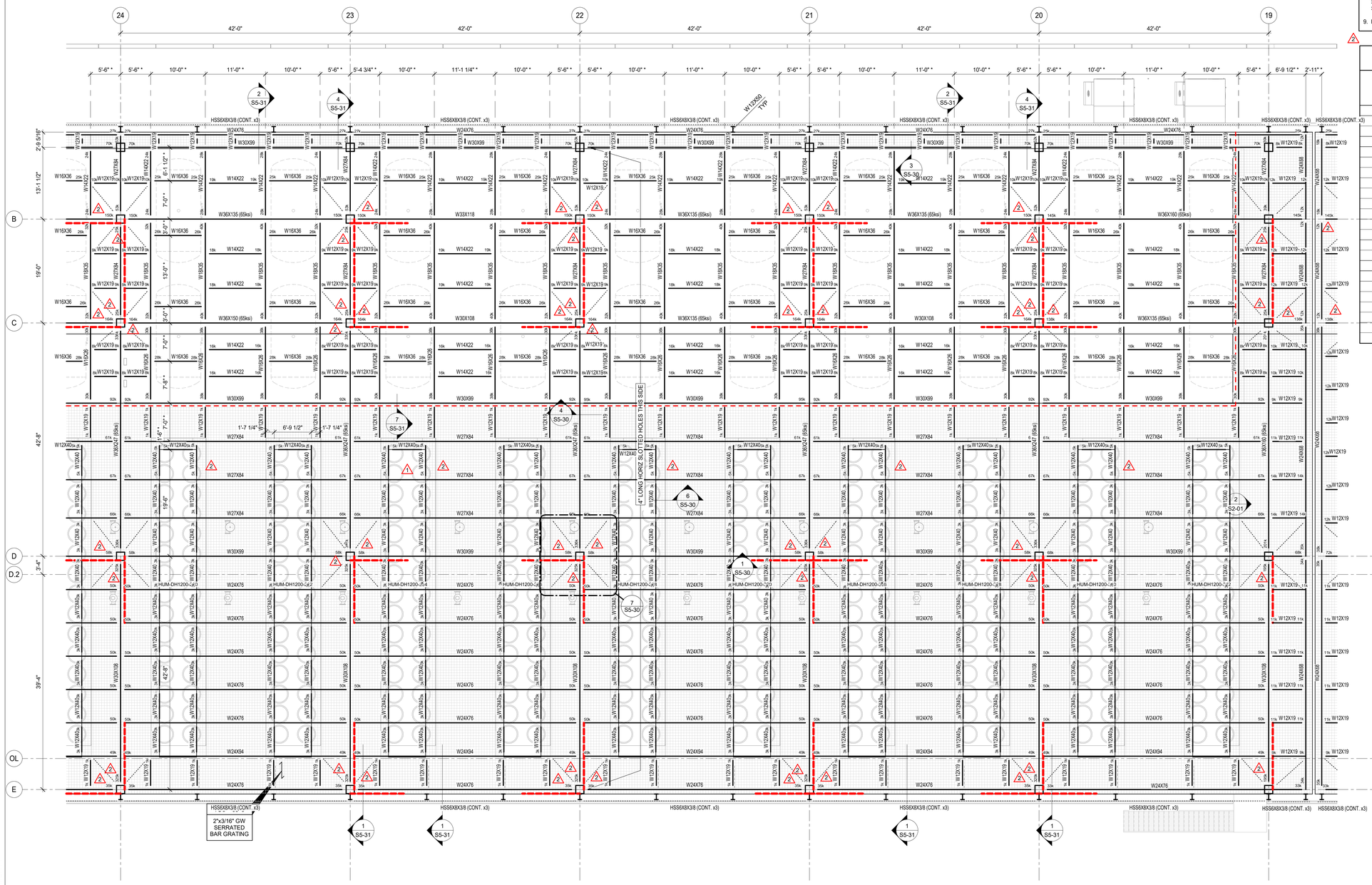
PLATFORM FRAMING - SEGMENT M

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40M

1 PLATFORM FRAMING PLAN - SEGMENT M
1/8" = 1'-0"

9/10/2025 5:05:58 PM S1-40M PLATFORM FRAMING - SEGMENT M



1 PLATFORM FRAMING PLAN - SEGMENT P
1/8" = 1'-0"

SHEET NOTES

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W24X76	±63K	±12K
W24X84	±76K	±2K
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2021 INTERNATIONAL CODES & 2023 NEC

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SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

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REVISIONS		
1	03.28.25	ADDENDUM 1
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COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

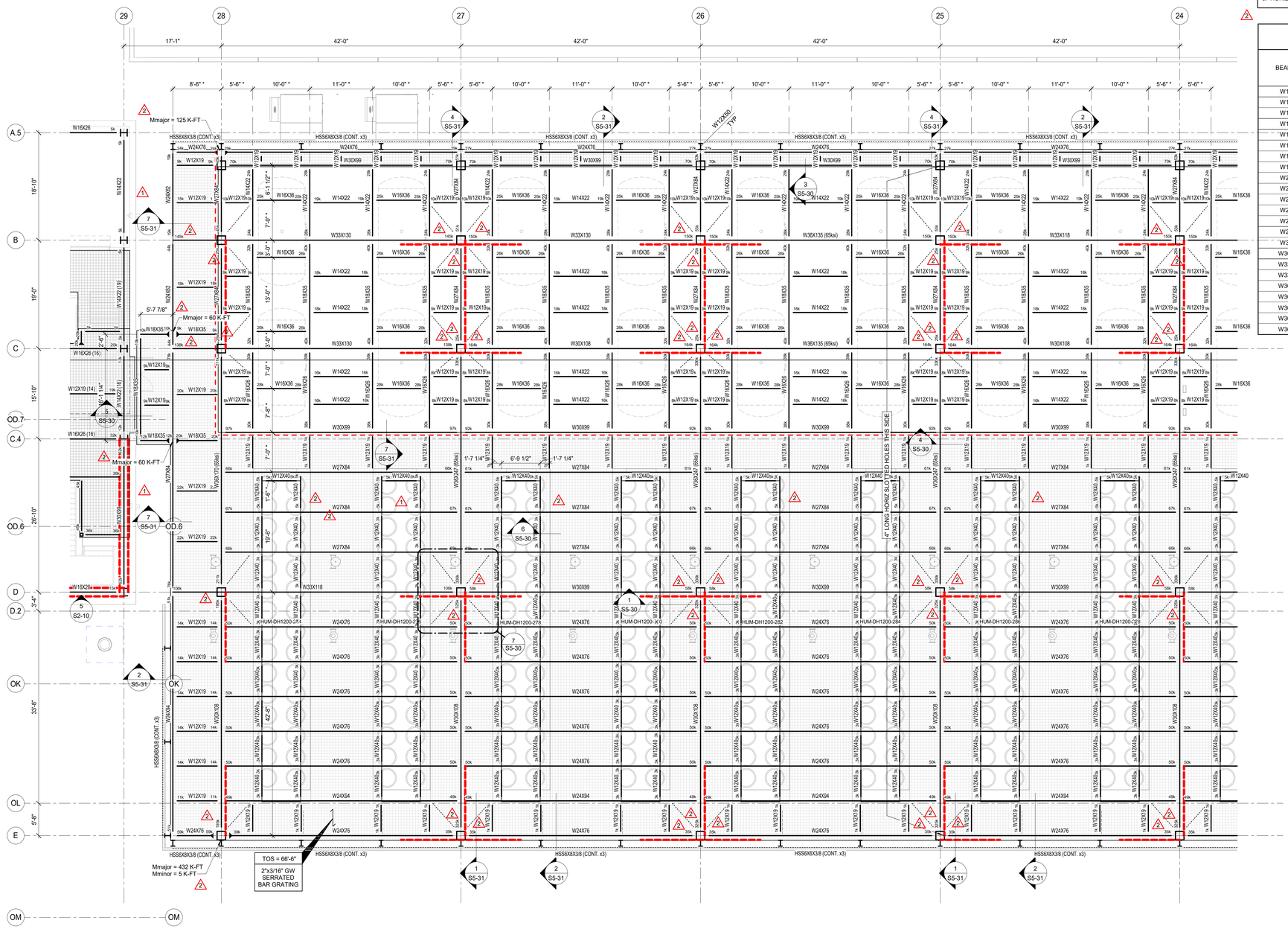
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PLATFORM FRAMING - SEGMENT P

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40P

9/10/2025 5:06:07 PM S1-40P PLATFORM FRAMING - SEGMENT P



SHEET NOTES

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1 PLATFORM FRAMING PLAN - SEGMENT R
1/8" = 1'-0"

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SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

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REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KUYLE
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

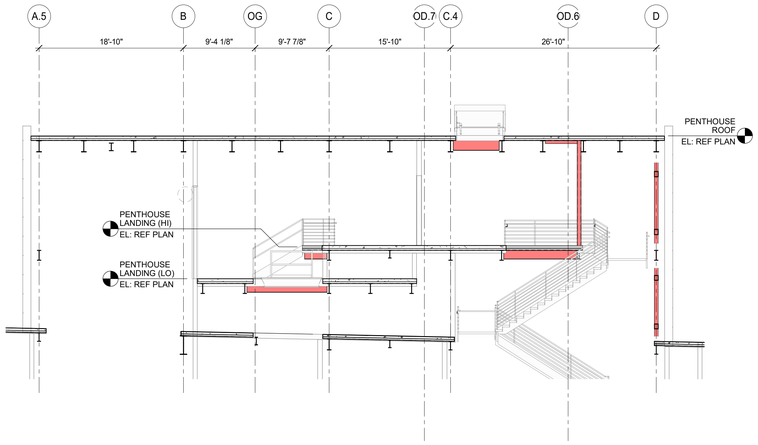
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T	R	P	M	K	H	F	D	B	

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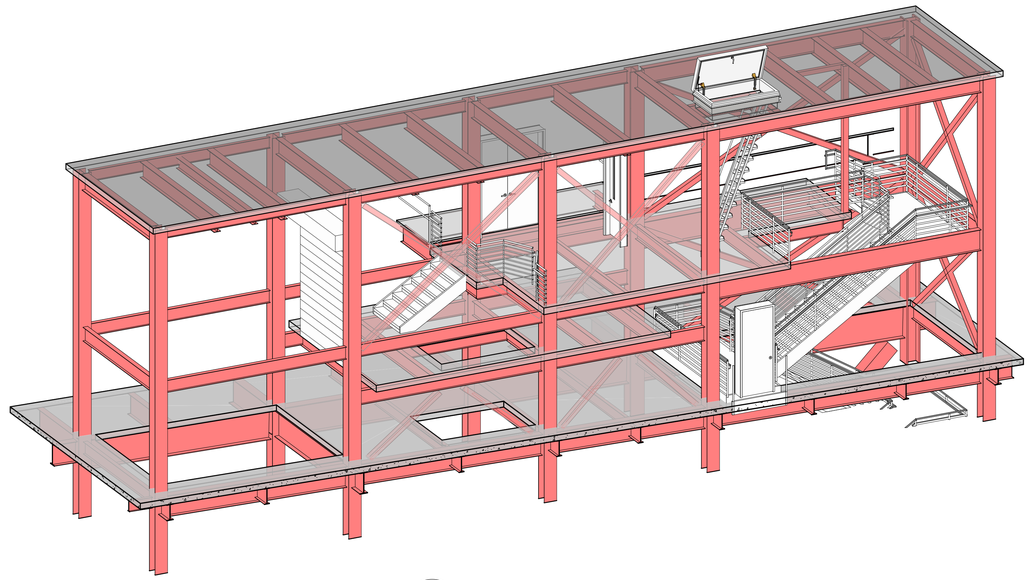
PLATFORM FRAMING - SEGMENT R

JOB 24464.0000
DATE 09.10.2025
SHEET
S1-40R

9/10/2025 5:06:14 PM S1-40R PLATFORM FRAMING - SEGMENT R



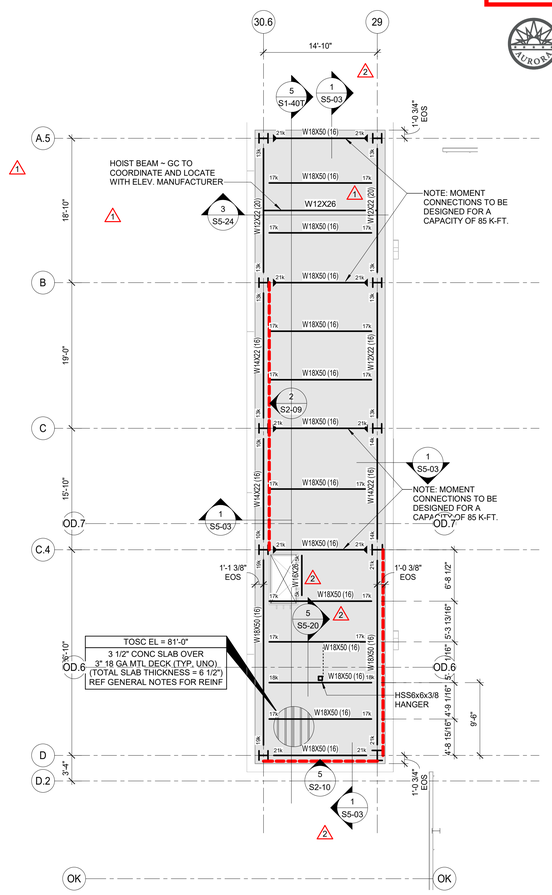
5 PENTHOUSE SECTION
1/8" = 1'-0"



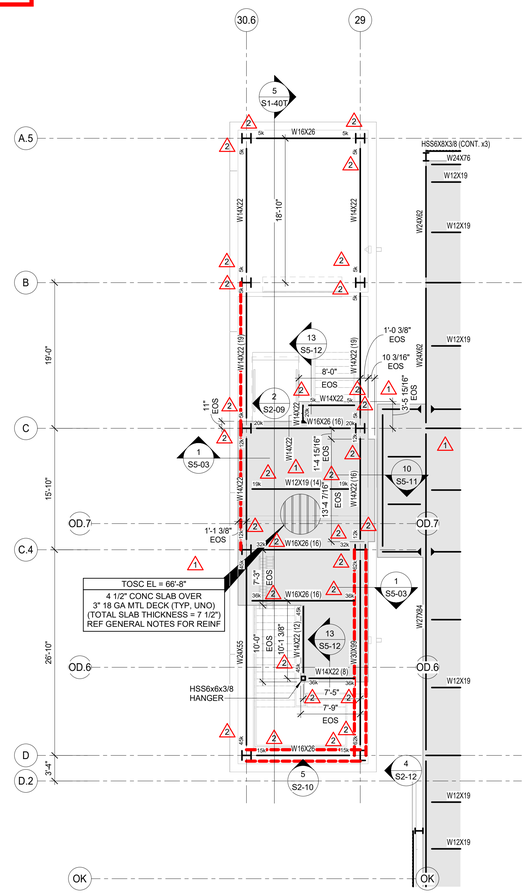
4 PENTHOUSE VIEW

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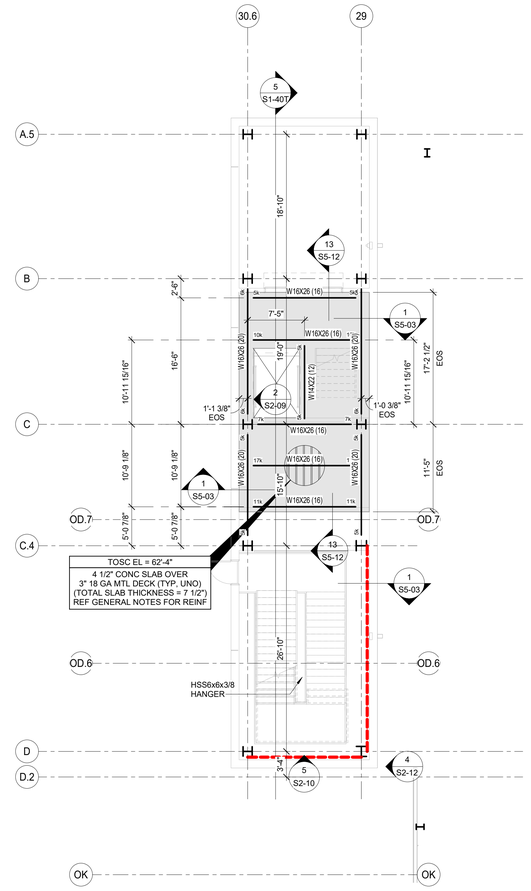
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3 PENTHOUSE ROOF FRAMING PLAN
1/8" = 1'-0"



2 PENTHOUSE LANDING FRAMING PLAN (HI)
1/8" = 1'-0"



1 PENTHOUSE LANDING FRAMING PLAN (LO)
1/8" = 1'-0"

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DESIGN CONSULTING LLC

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COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

N

PENTHOUSE ROOF FRAMING PLAN - SEGMENT T

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-40T

9/10/2025 5:06:20 PM S1-40T PENTHOUSE ROOF FRAMING PLAN - SEGMENT T

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SYSKA HENNESSY GROUP

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	REVISIONS	

COLORADO LICENSED
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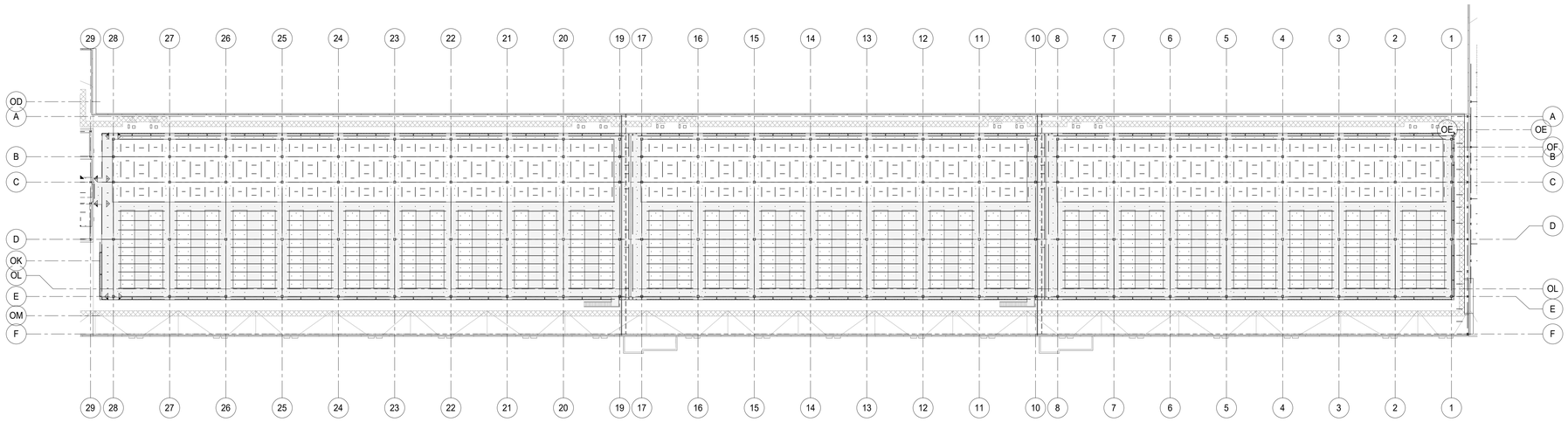
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1 OVERALL PLATFORM GIRT FRAMING PLAN
1" = 40'-0"

KEYPLAN

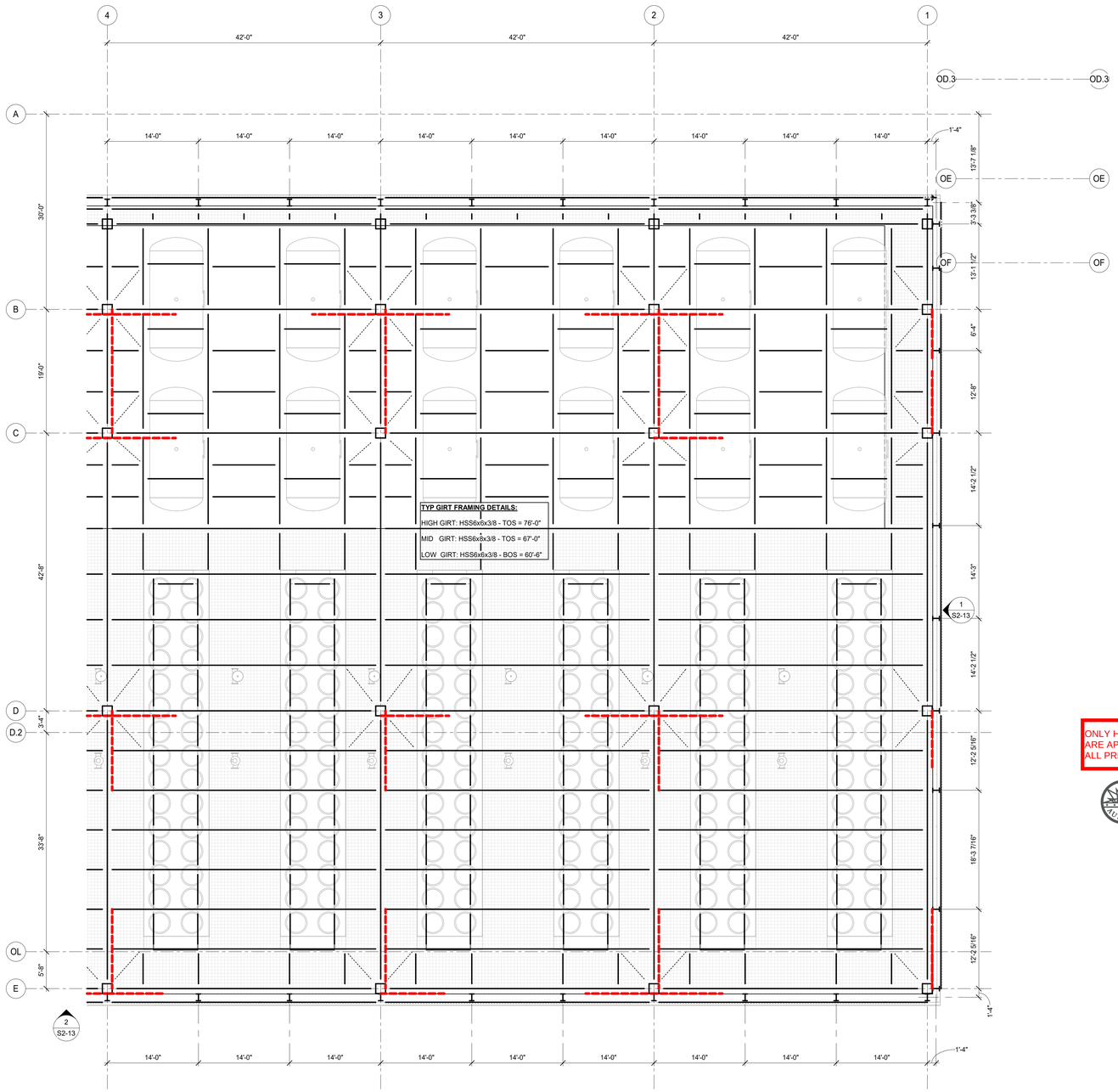


OVERALL PLATFORM GIRT FRAMING PLAN

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-50

9/10/2025 5:06:43 PM S1-50 OVERALL PLATFORM GIRT FRAMING PLAN



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1 PLATFORM GIRT FRAMING PLAN - SEGMENT F
1/8" = 1'-0"

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REVISIONS		
1	03.28.25	ADDENDUM 1

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KEYPLAN

S	Q	N	L	J	G	E	C	A
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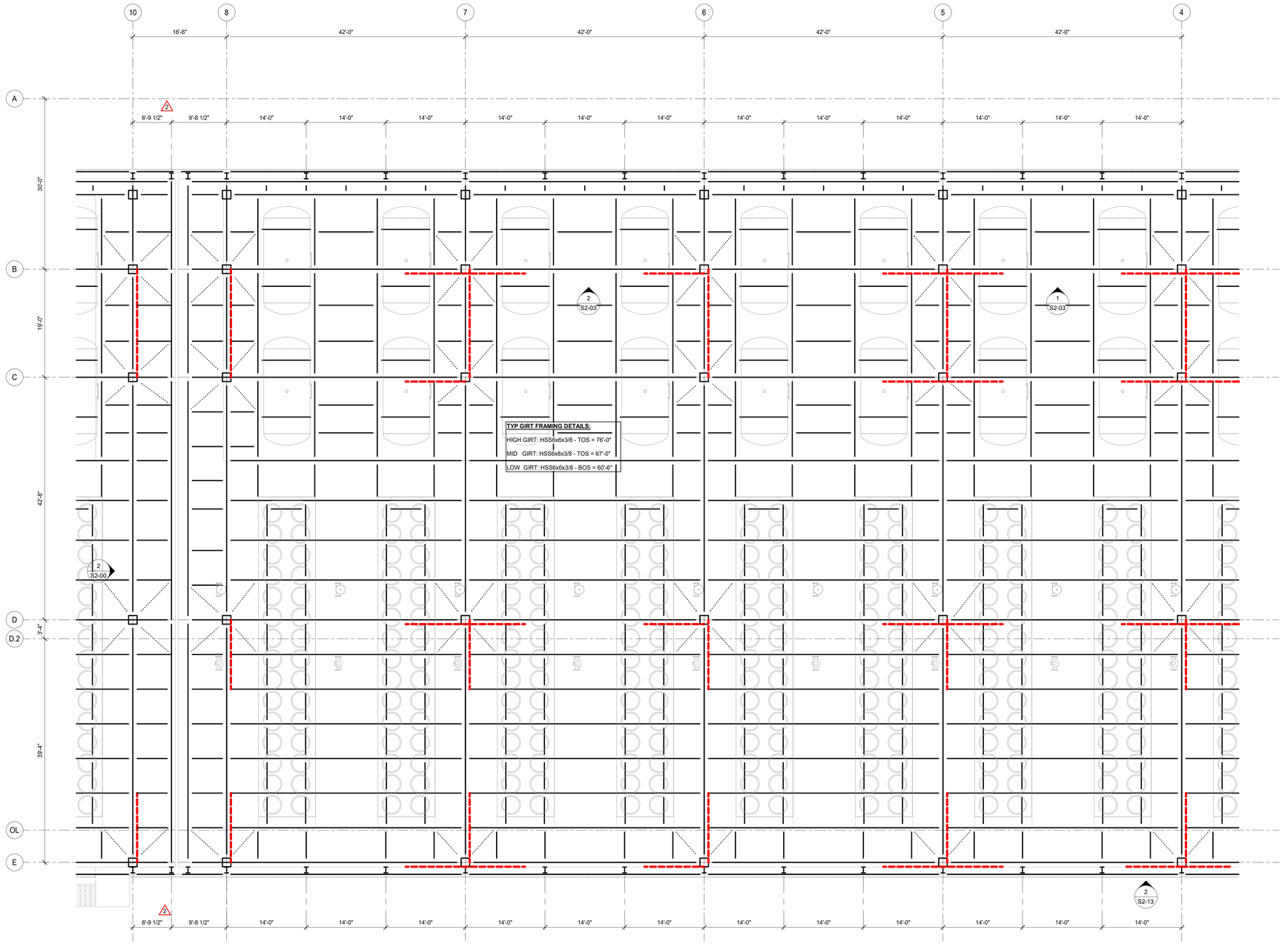
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PLATFORM GIRT FRAMING PLAN - SEGMENT F

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-50F

9/10/2025 5:06:45 PM S1-50F PLATFORM GIRT FRAMING PLAN - SEGMENT F



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1 PLATFORM GIRT FRAMING PLAN - SEGMENT H
1/8" = 1'-0"

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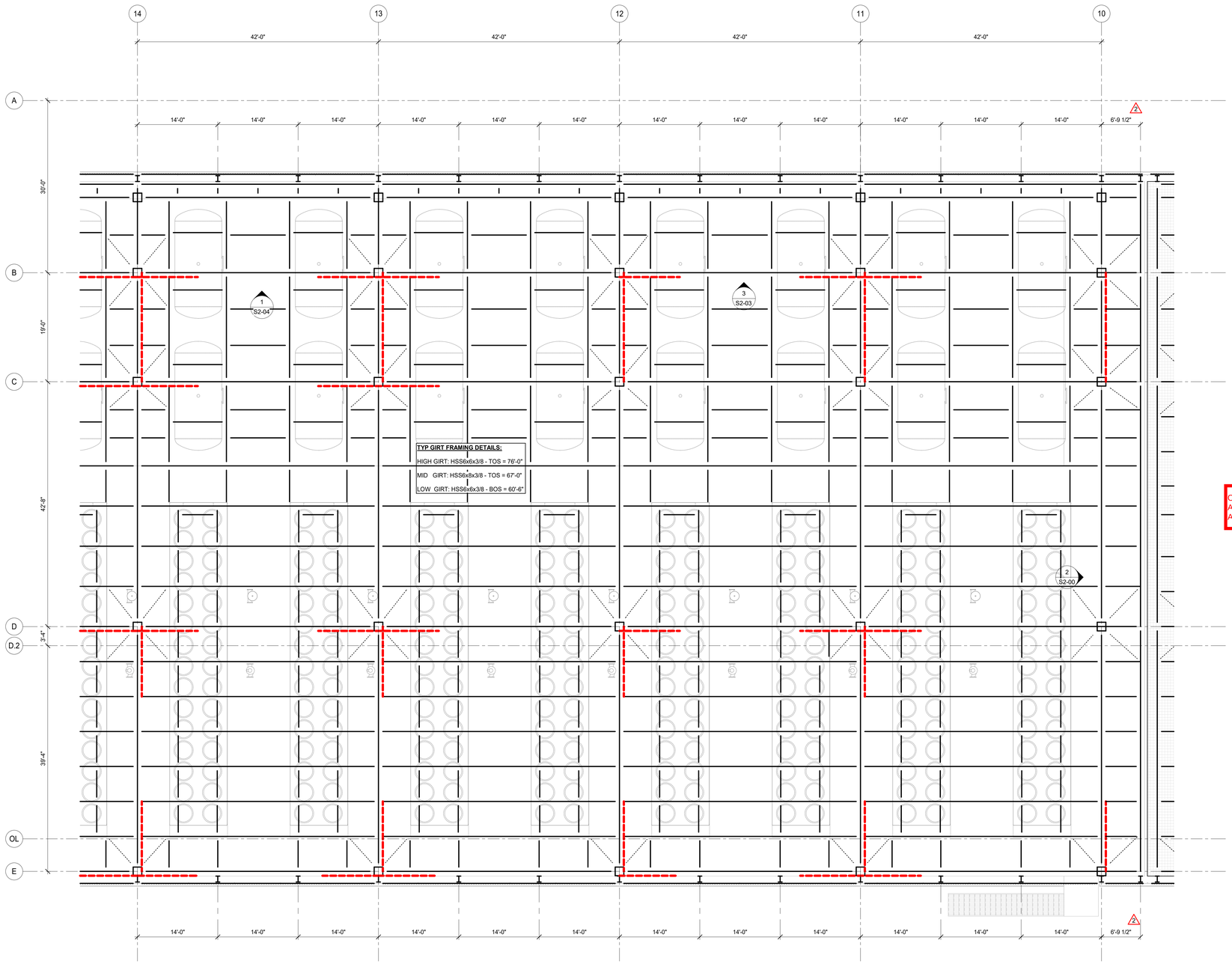
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PLATFORM GIRT FRAMING PLAN - SEGMENT H

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-50H

9/10/2025 5:06:48 PM S1-50H PLATFORM GIRT FRAMING PLAN - SEGMENT H



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1 PLATFORM GIRT FRAMING PLAN - SEGMENT K
1/8" = 1'-0"

9/10/2025 5:06:50 PM S1-50K PLATFORM GIRT FRAMING PLAN - SEGMENT K

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	REVISIONS	
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

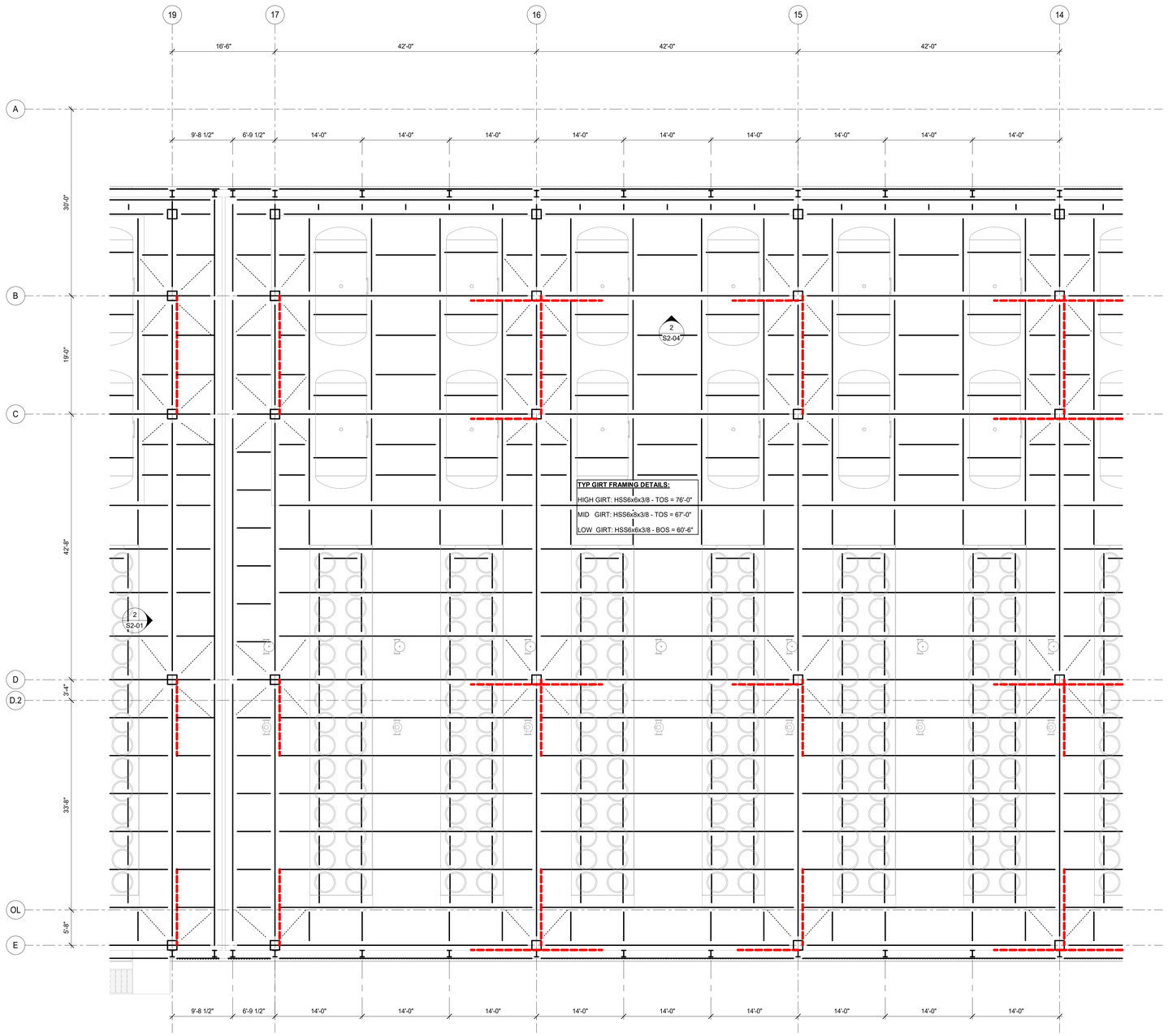
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T	R	P	M	K	H	F	D	B	

N

PLATFORM GIRT FRAMING PLAN - SEGMENT K

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-50K



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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

1 PLATFORM GIRT FRAMING PLAN - SEGMENT M
1/8" = 1'-0"

9/10/2025 5:06:52 PM S1-50M PLATFORM GIRT FRAMING PLAN - SEGMENT M

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	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
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	REVISIONS	

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NICHOLAS W. REYID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

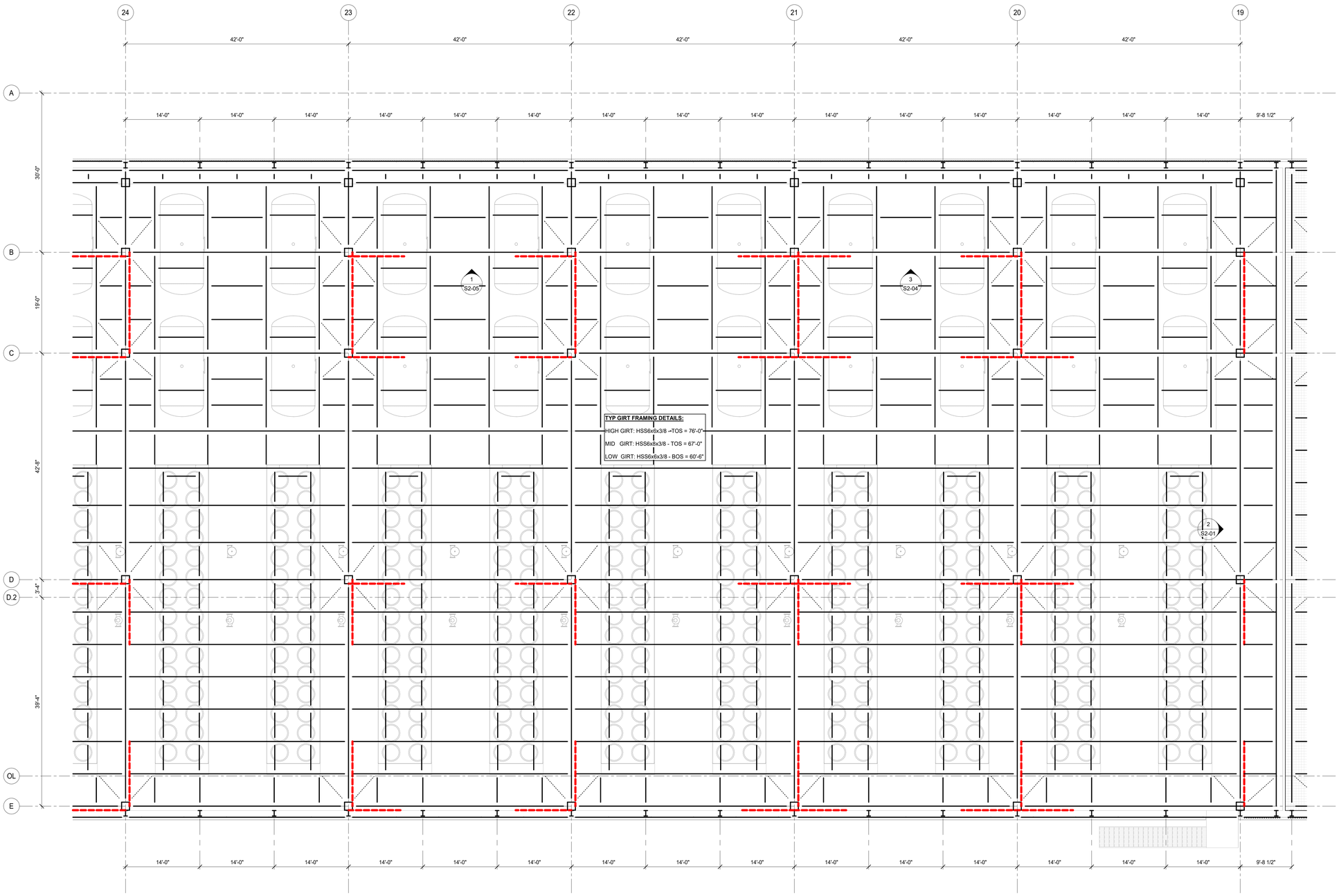
S Q N L J G E C A
T R P M K H F D B

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PLATFORM GIRT FRAMING PLAN - SEGMENT M

JOB 24464.0000
DATE 09.10.2025
SHEET

S1-50M



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2021 INTERNATIONAL CODES & 2023 NEC

1 PLATFORM GIRT FRAMING PLAN - SEGMENT P
1/8" = 1'-0"

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DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
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	REVISIONS	

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65236
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AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

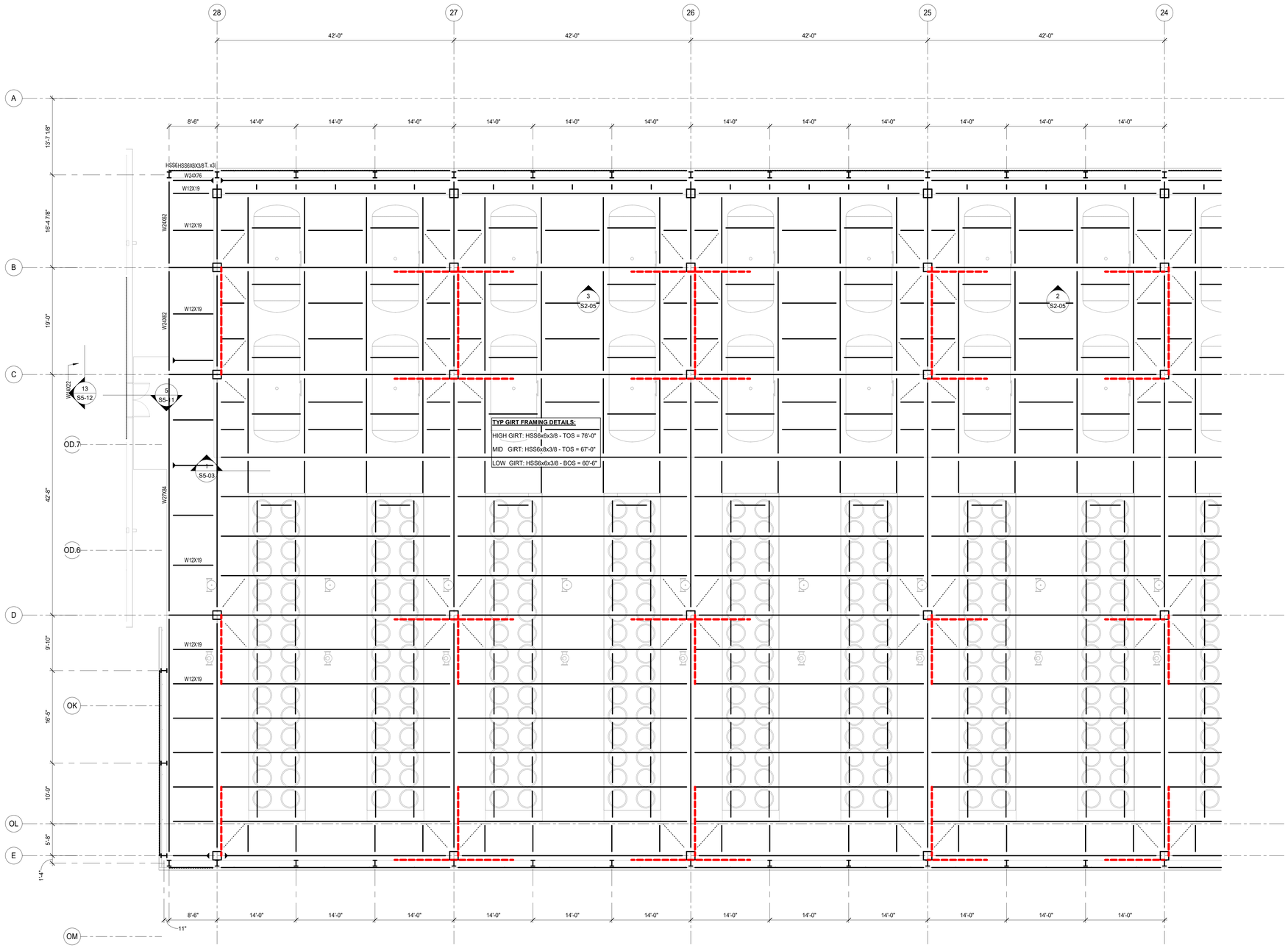
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PLATFORM GIRT FRAMING PLAN - SEGMENT P

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DATE 09.10.2025
SHEET

S1-50P

9/10/2025 5:06:55 PM S1-50P PLATFORM GIRT FRAMING PLAN - SEGMENT P



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2021 INTERNATIONAL CODES & 2023 NEC

1 PLATFORM GIRT FRAMING PLAN - SEGMENT R
1/8" = 1'-0"

9/10/2025 5:06:57 PM S1-50R PLATFORM GIRT FRAMING PLAN - SEGMENT R

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ISSUES

1	12.20.24	DESIGN DEVELOPMENT
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REVISIONS

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A
T	R	P	M	K	H	F	D	B

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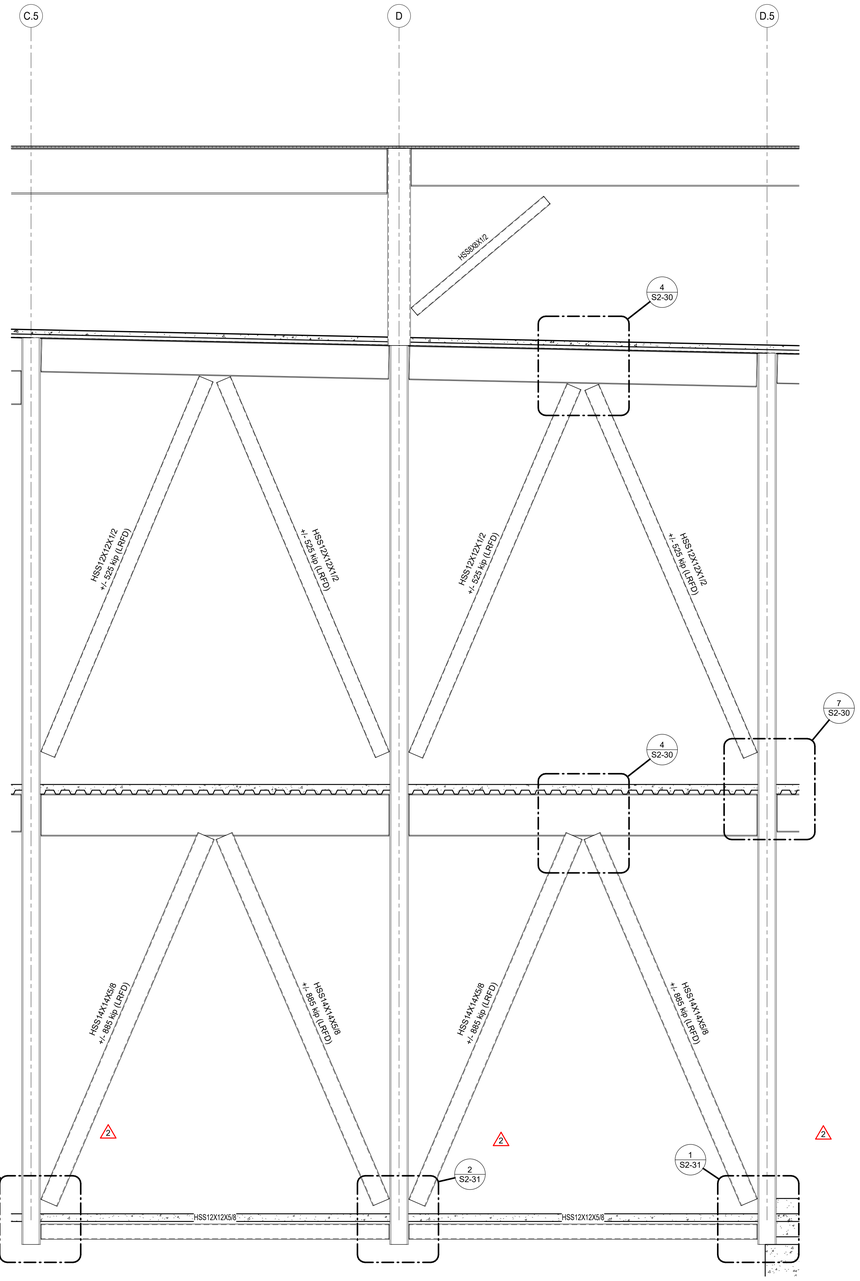
PLATFORM GIRT FRAMING PLAN - SEGMENT R

JOB 24464.0000
DATE 09.10.2025
SHEET

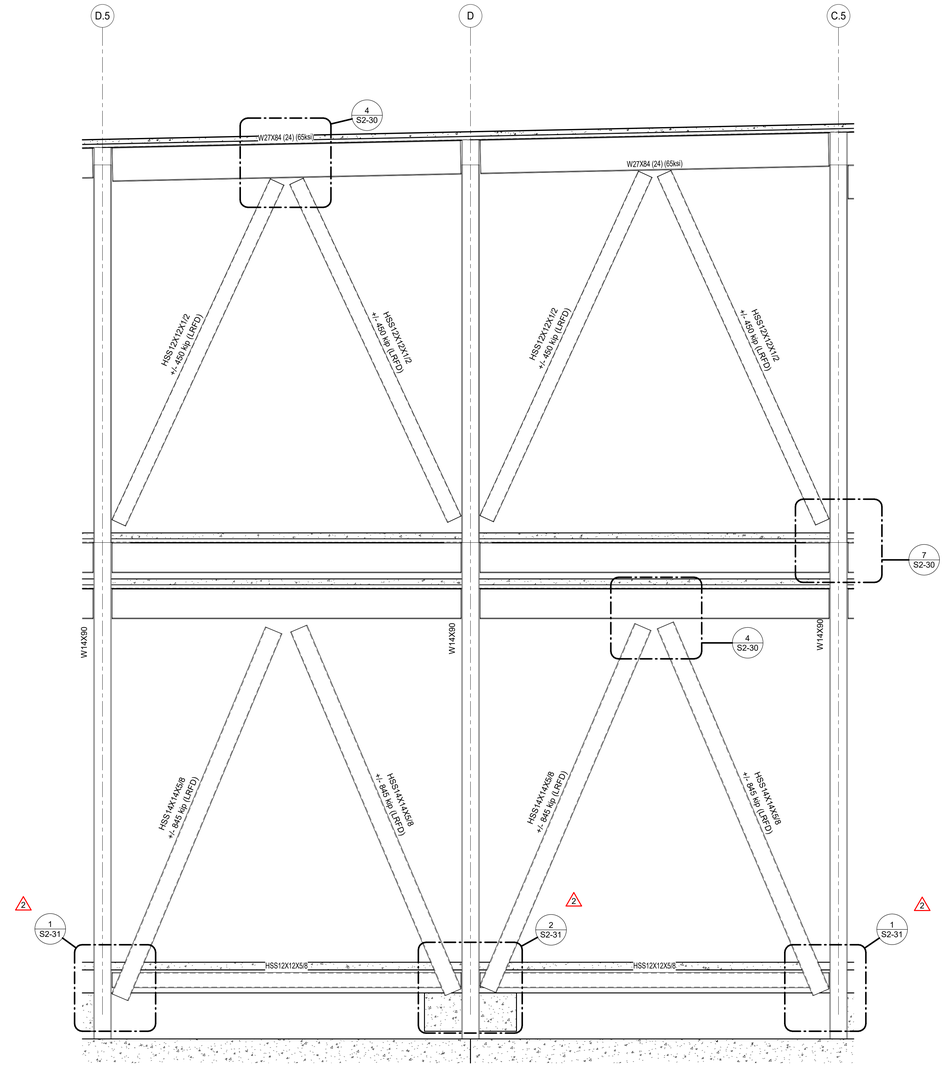
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2021 INTERNATIONAL CODES & 2023 NEC



2 BRACED FRAME ELEVATION ALONG GRID 10
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID 9
1/4" = 1'-0"

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	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

FRAME ELEVATIONS

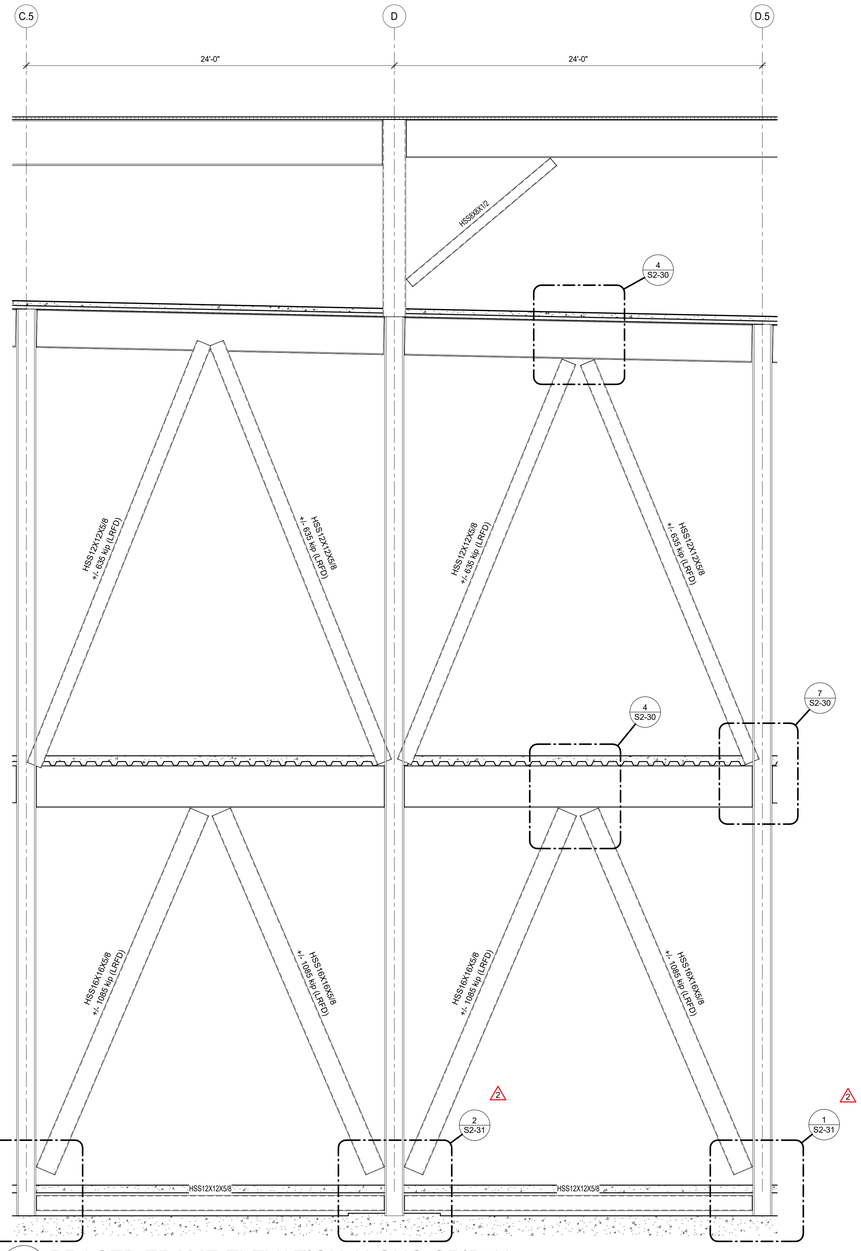
JOB 24464.0000
DATE 09.10.2025
SHEET

S2-00

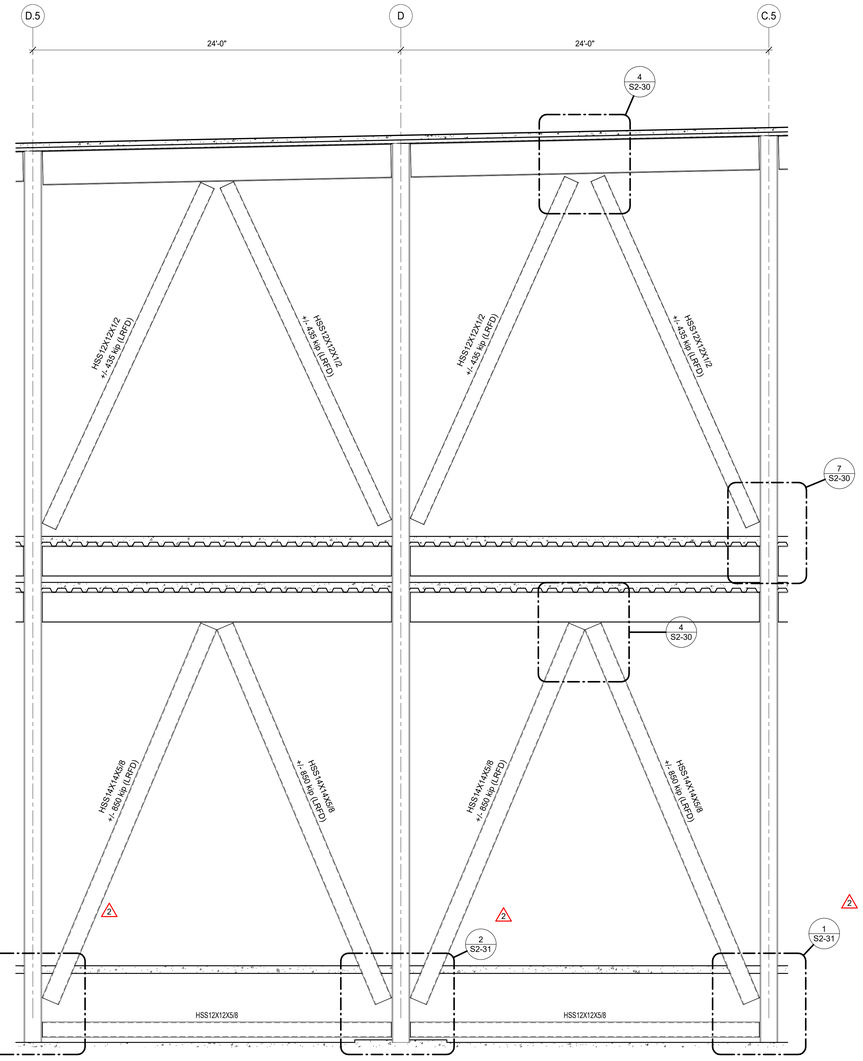
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City of Aurora Building Division
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Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



2 BRACED FRAME ELEVATION ALONG GRID 19
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID 18
1/4" = 1'-0"

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	ISSUES	
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	REVISIONS	
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2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

FRAME ELEVATIONS

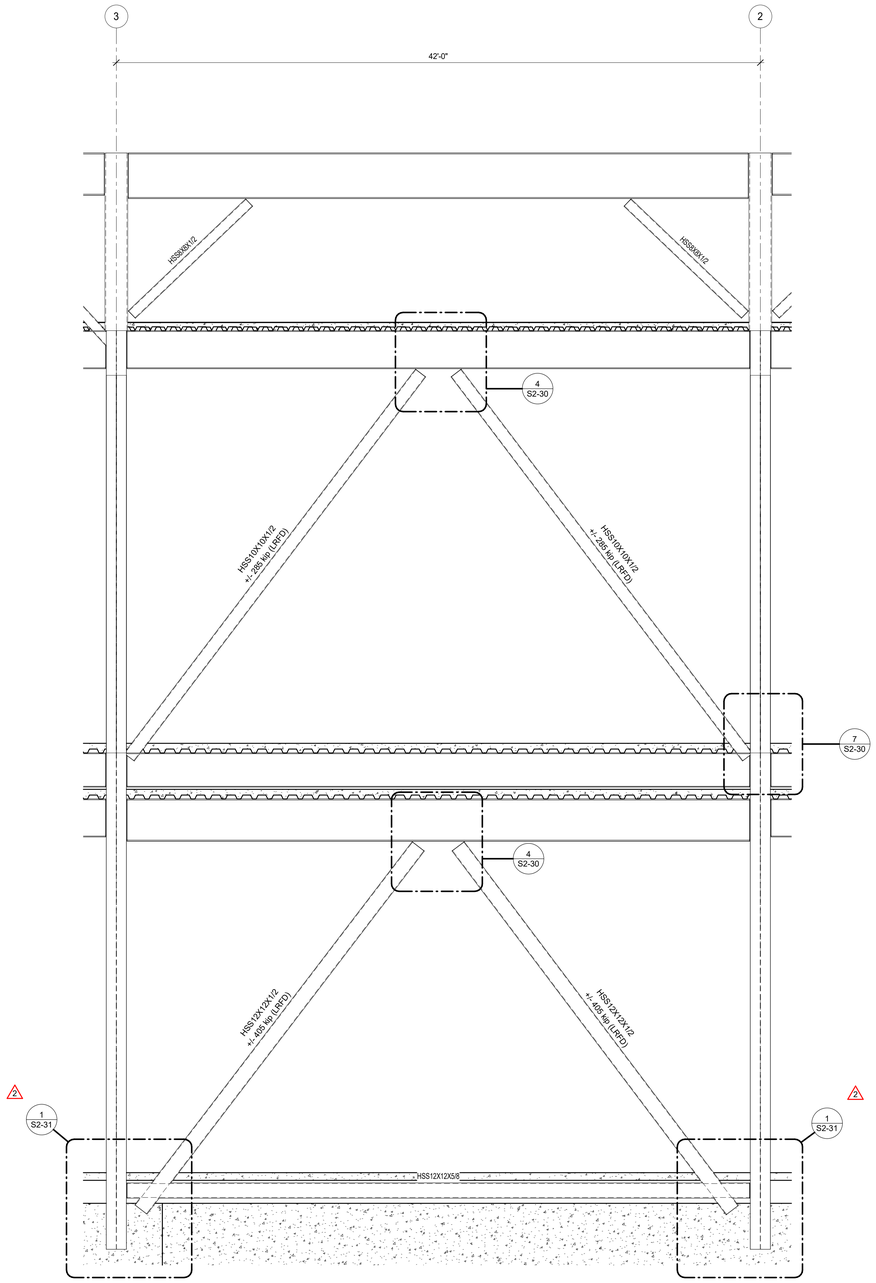
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DATE 09.10.2025
SHEET

S2-01

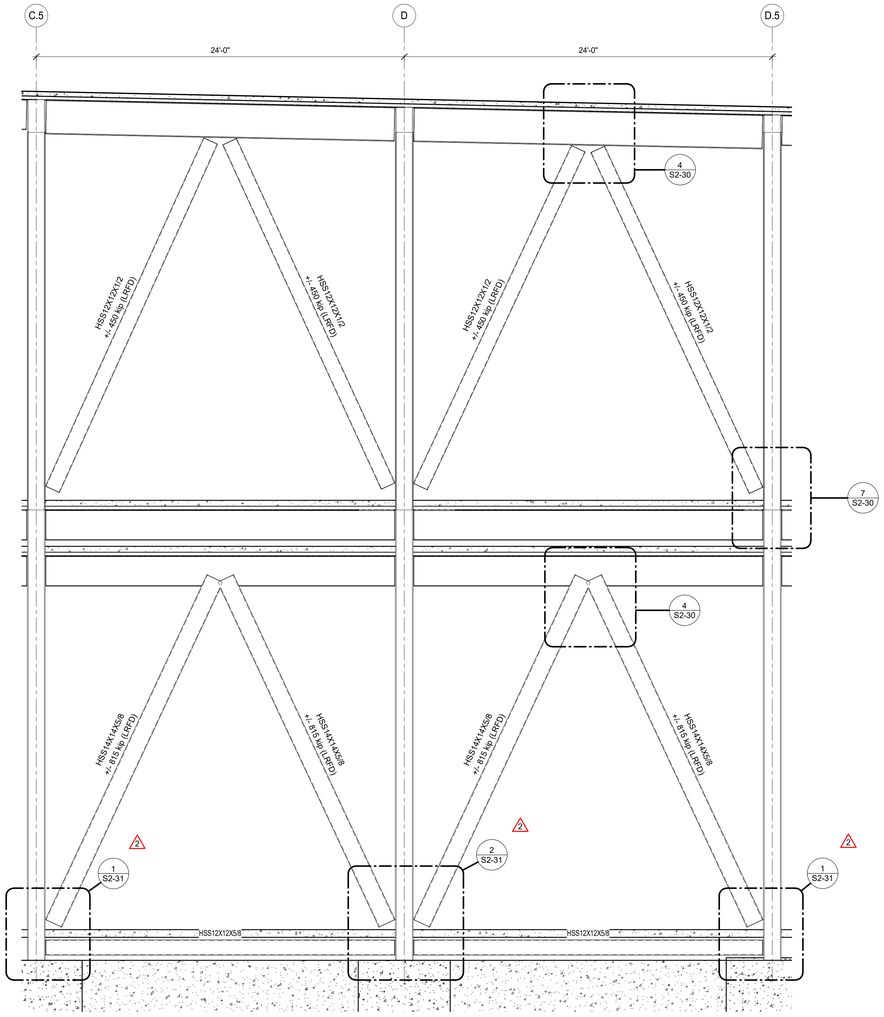
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City of Aurora Building Division
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2021 INTERNATIONAL CODES & 2023 NEC



2 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID 0
1/4" = 1'-0"

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	ISSUES	
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	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

FRAME ELEVATIONS

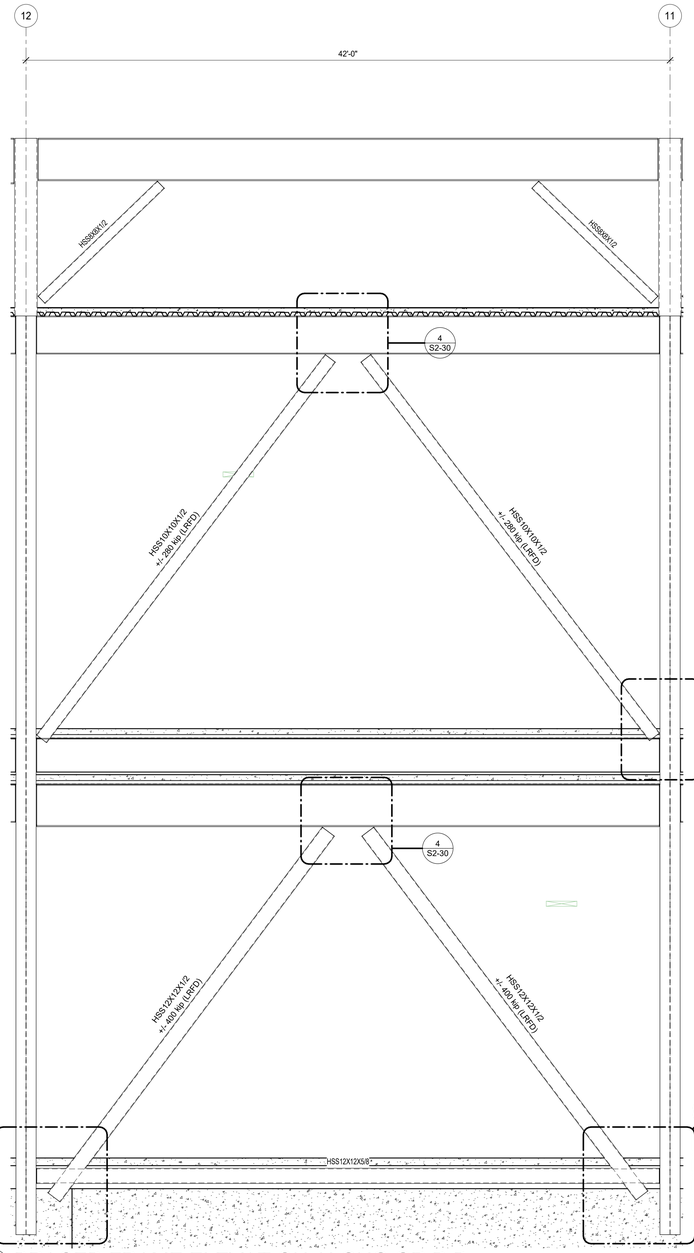
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DATE 09.10.2025
SHEET

S2-02

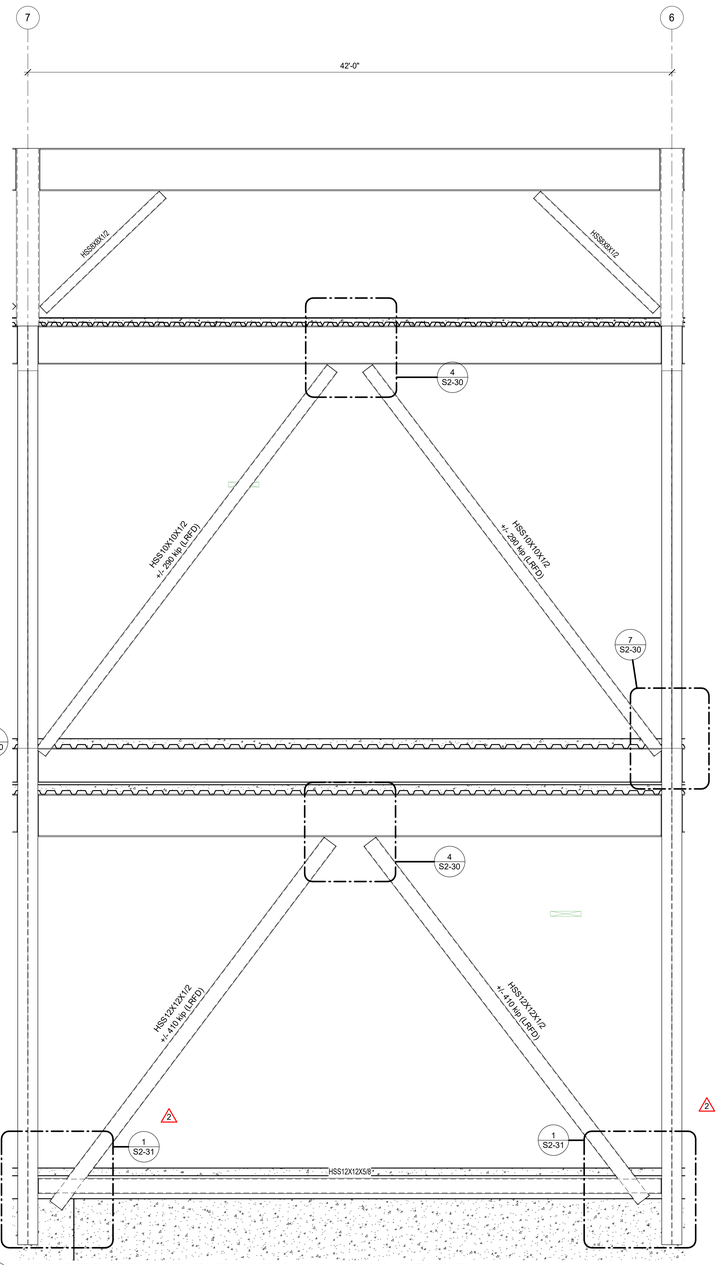
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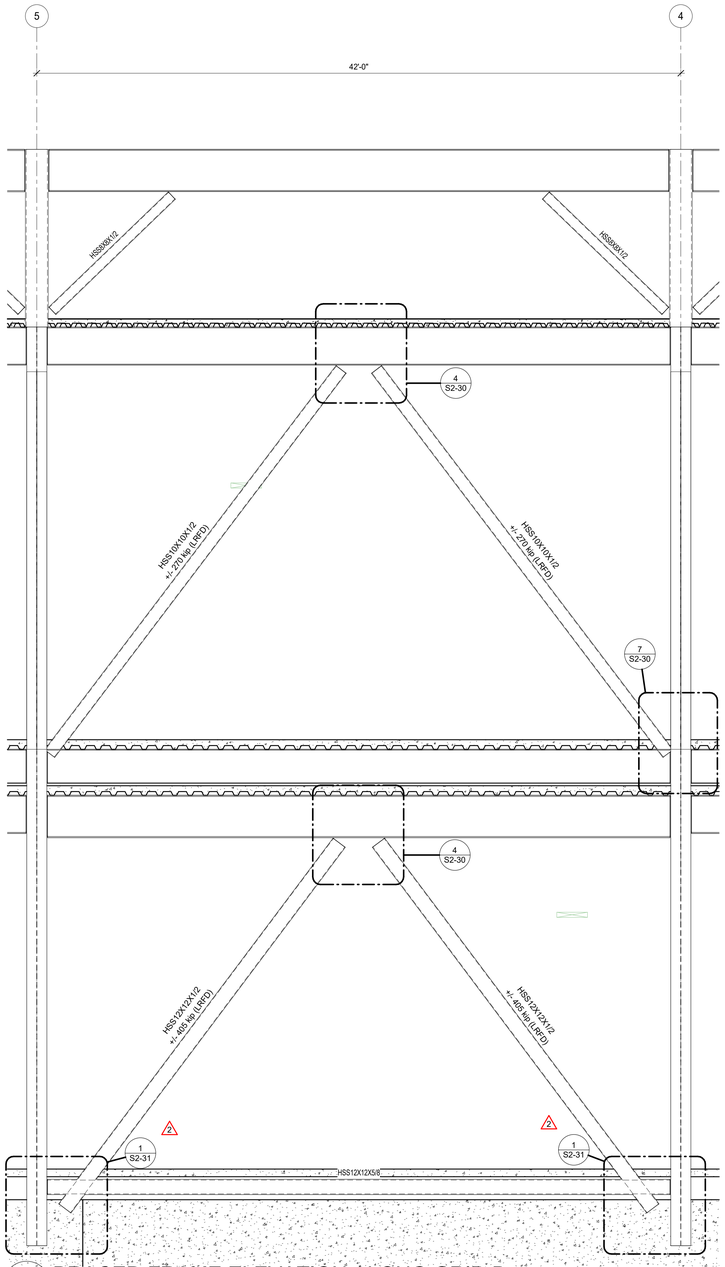
City of Aurora Building Division
Reviewed for Code Compliance
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Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



3 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"

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ISSUES		
1	12.20.24	DESIGN DEVELOPMENT
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REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

FRAME ELEVATIONS

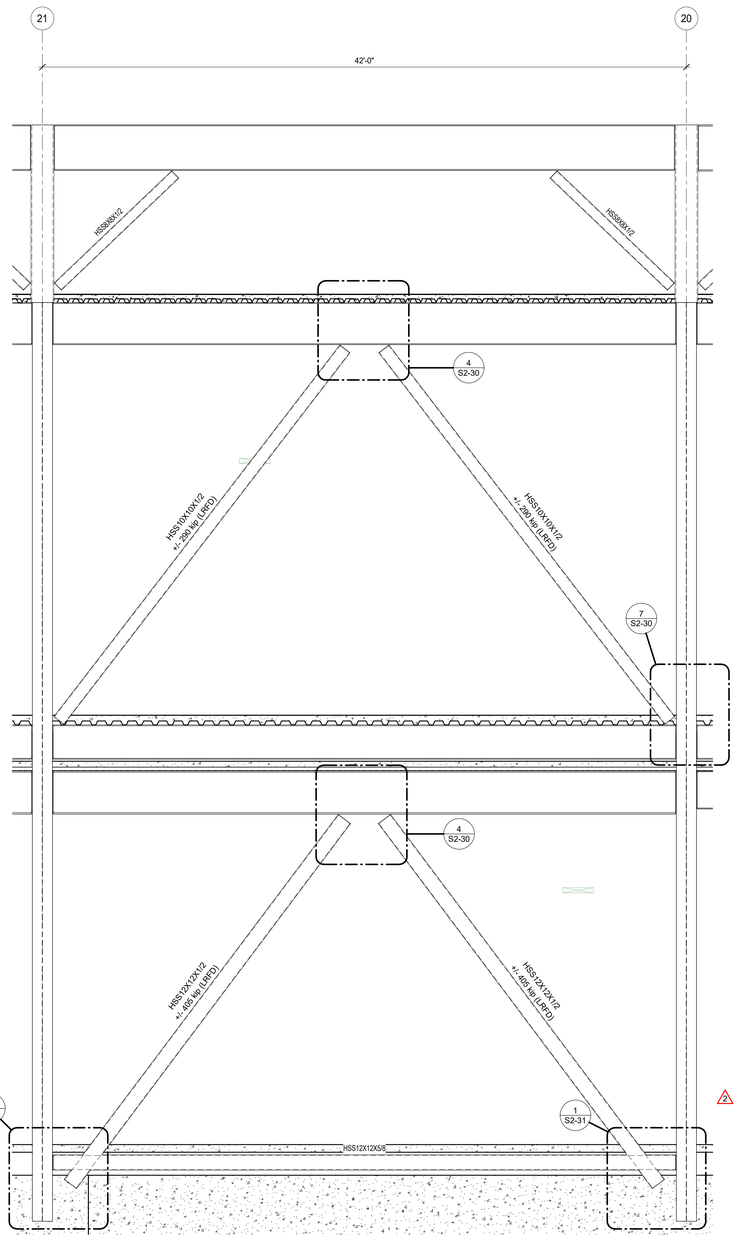
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S2-03

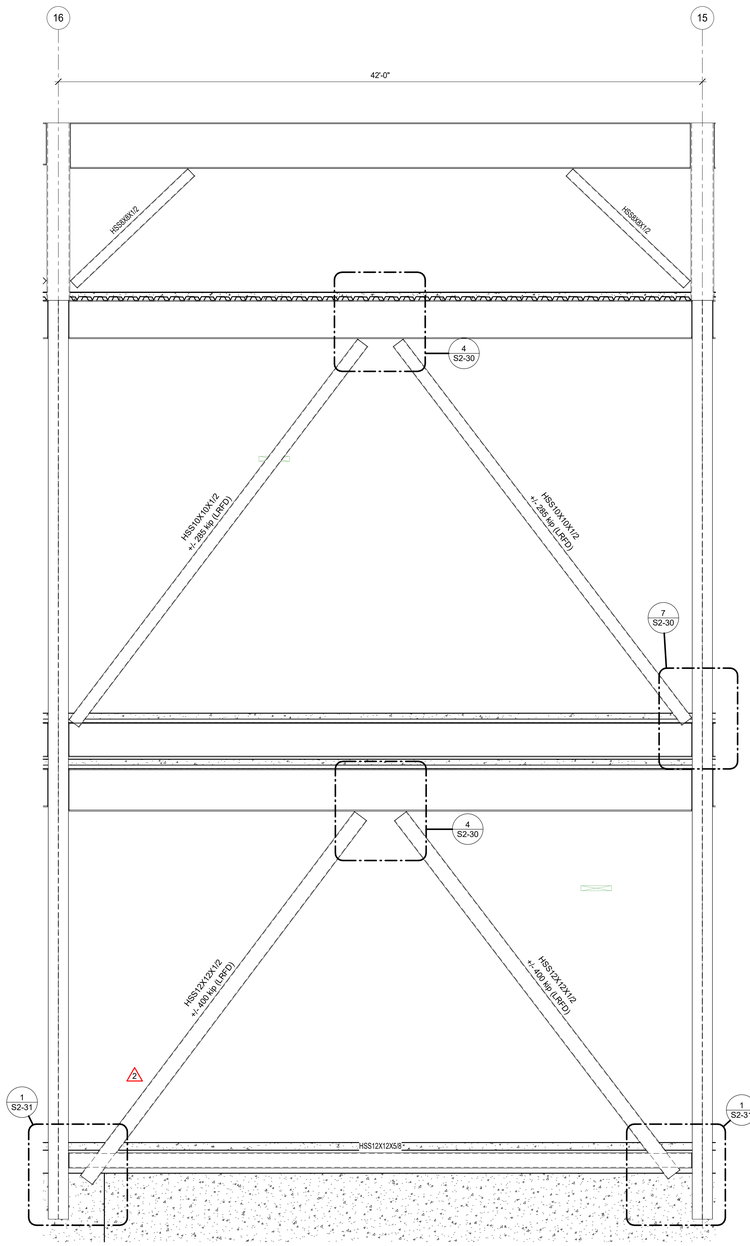
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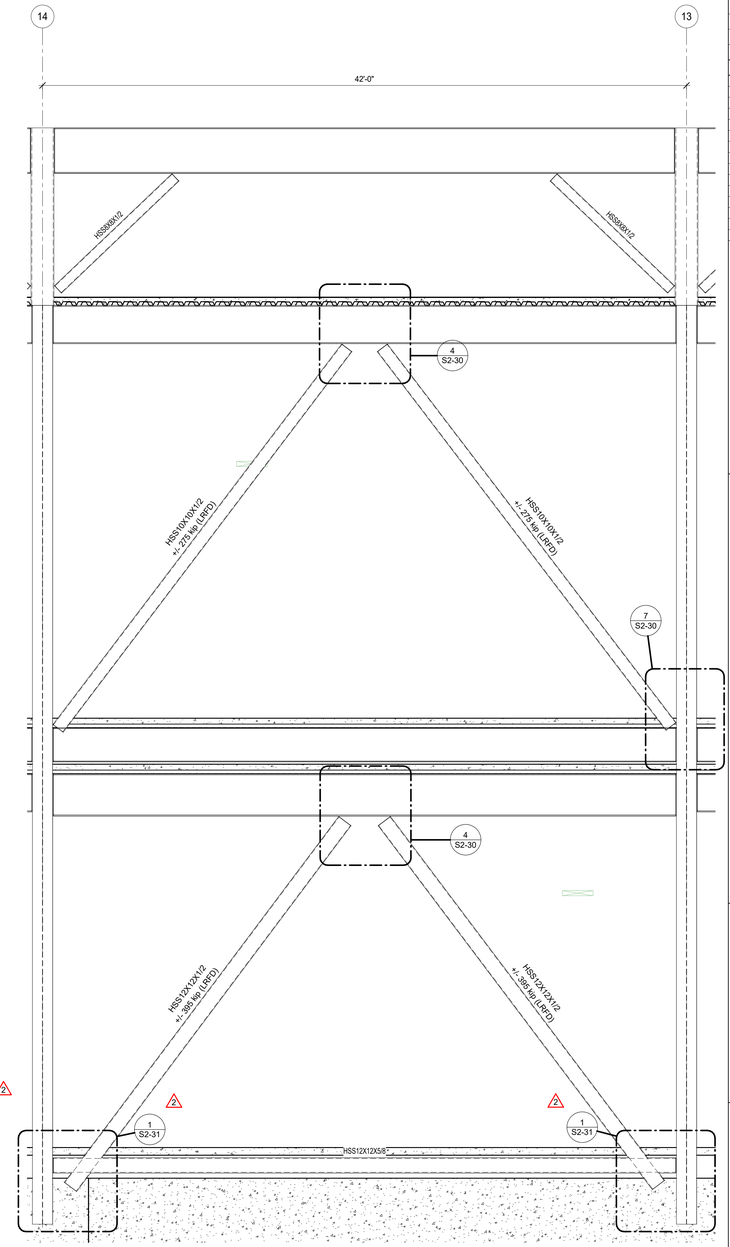
City of Aurora Building Division
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Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



3 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"

9/10/2025 5:07:09 PM S2-04 FRAME ELEVATIONS

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	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
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	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD

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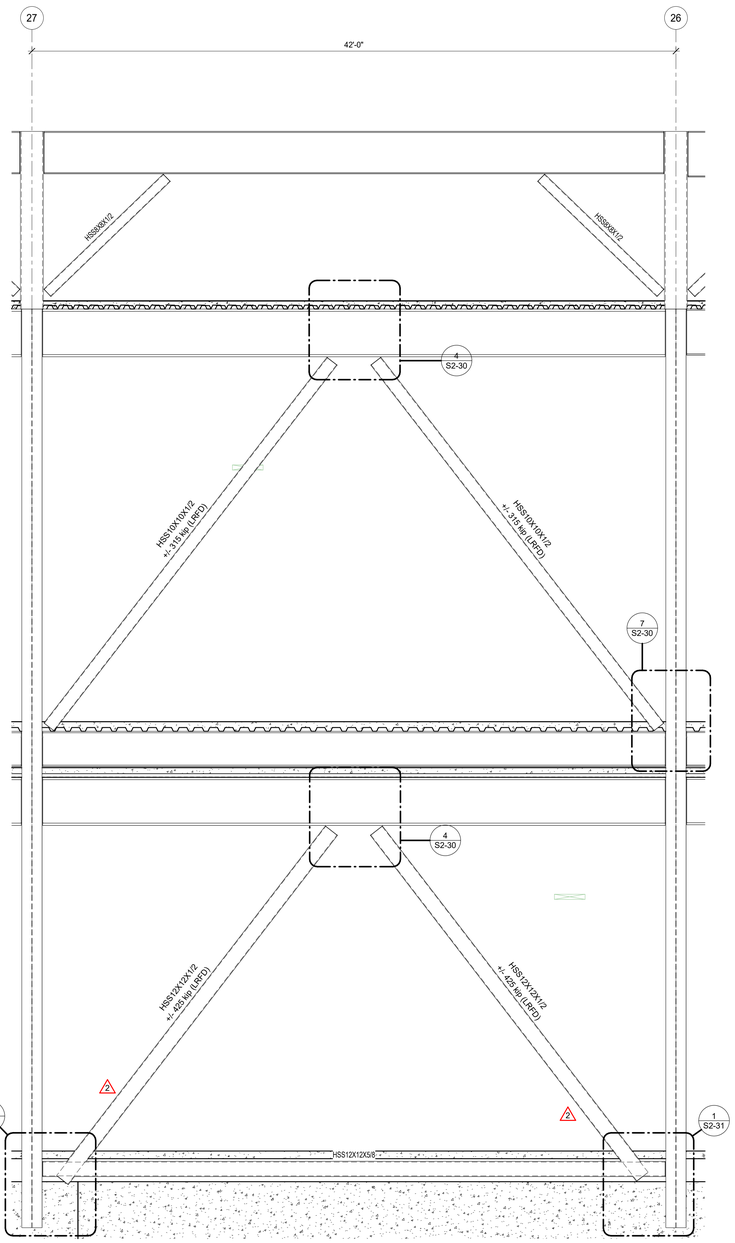
FRAME ELEVATIONS

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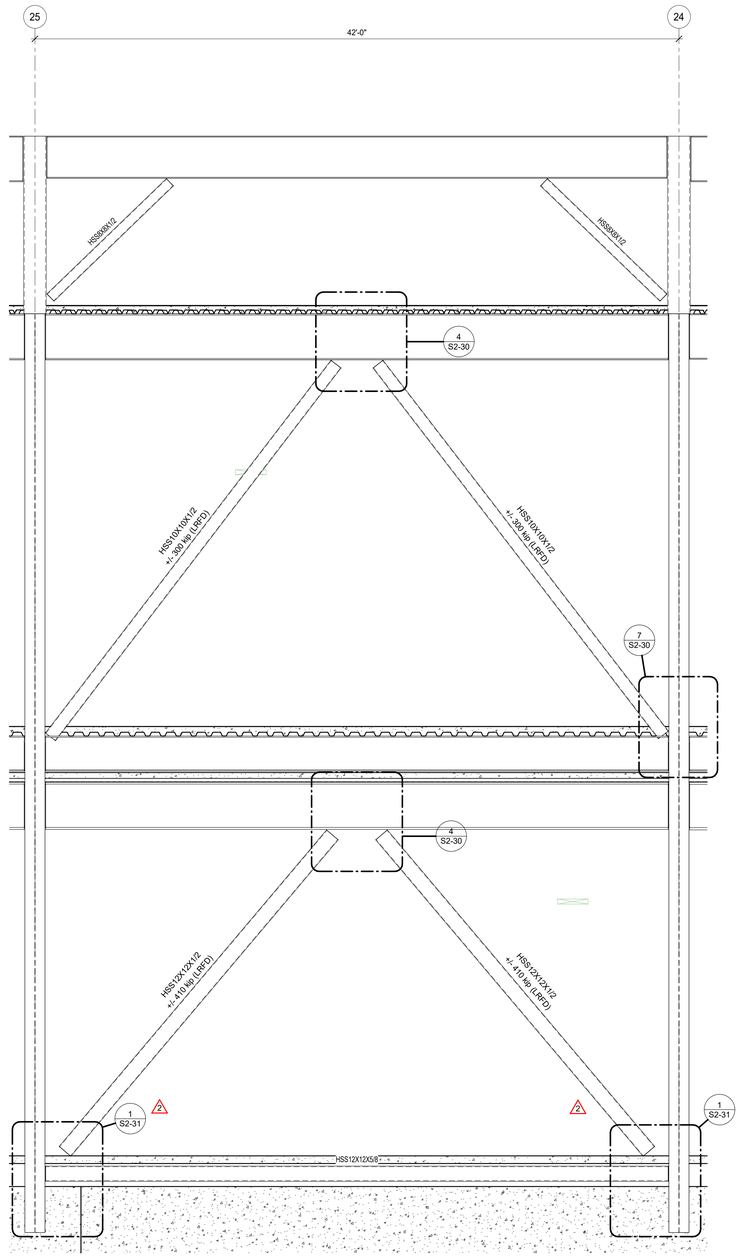
S2-04

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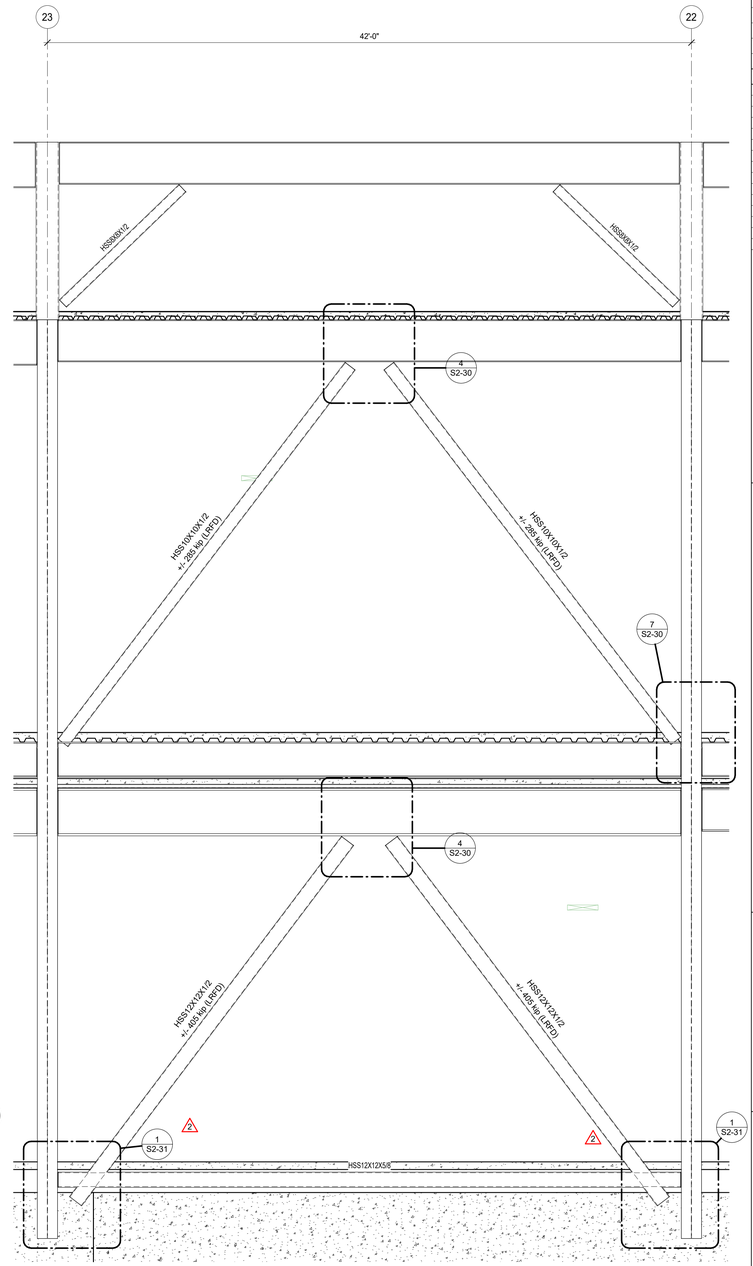
City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



3 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID B
1/4" = 1'-0"

9/10/2025 5:07:13 PM S2-05 FRAME ELEVATIONS

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ISSUES

1	12.20.24	DESIGN DEVELOPMENT
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REVISIONS

1	03.28.25	ADDENDUM 1
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COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

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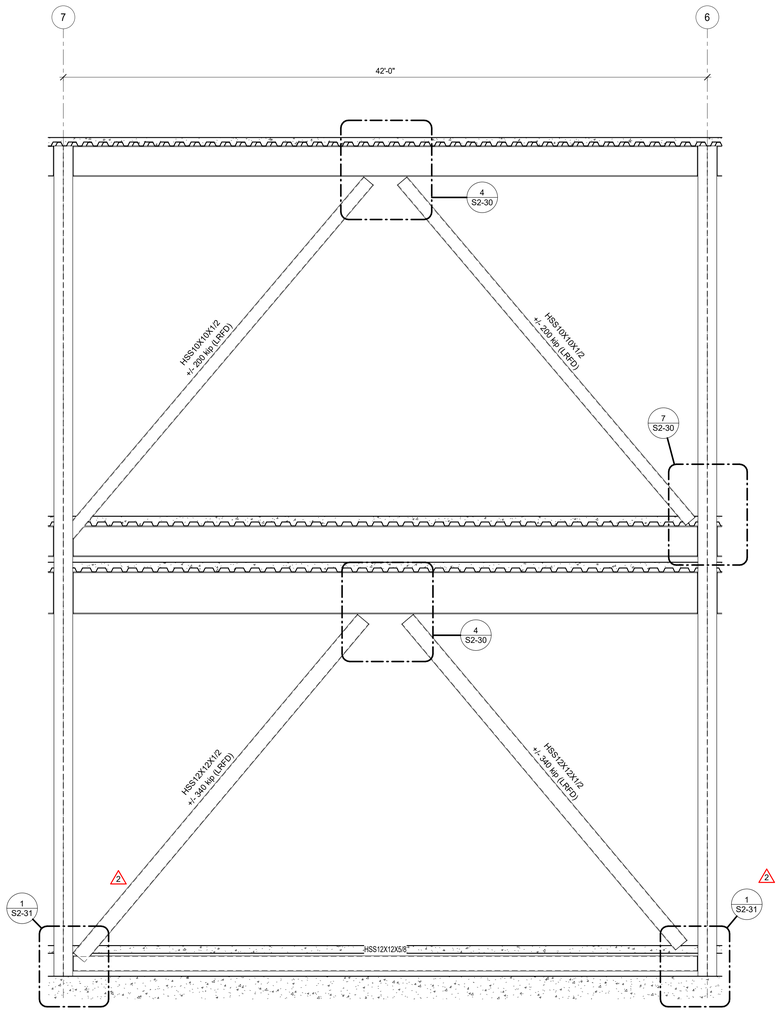
FRAME ELEVATIONS

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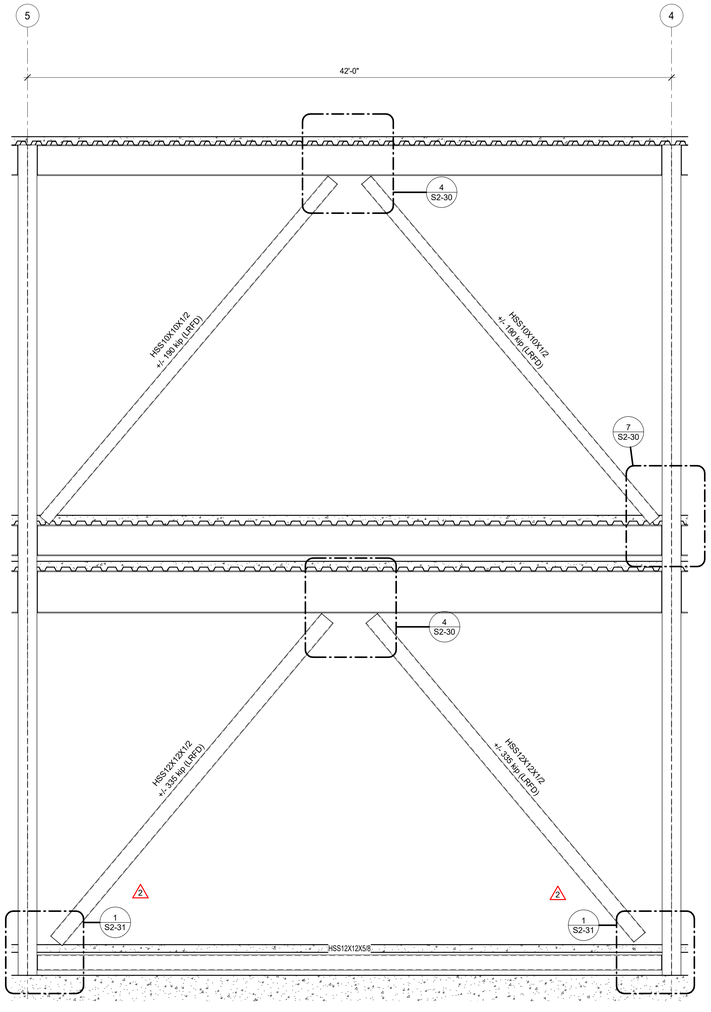
S2-05

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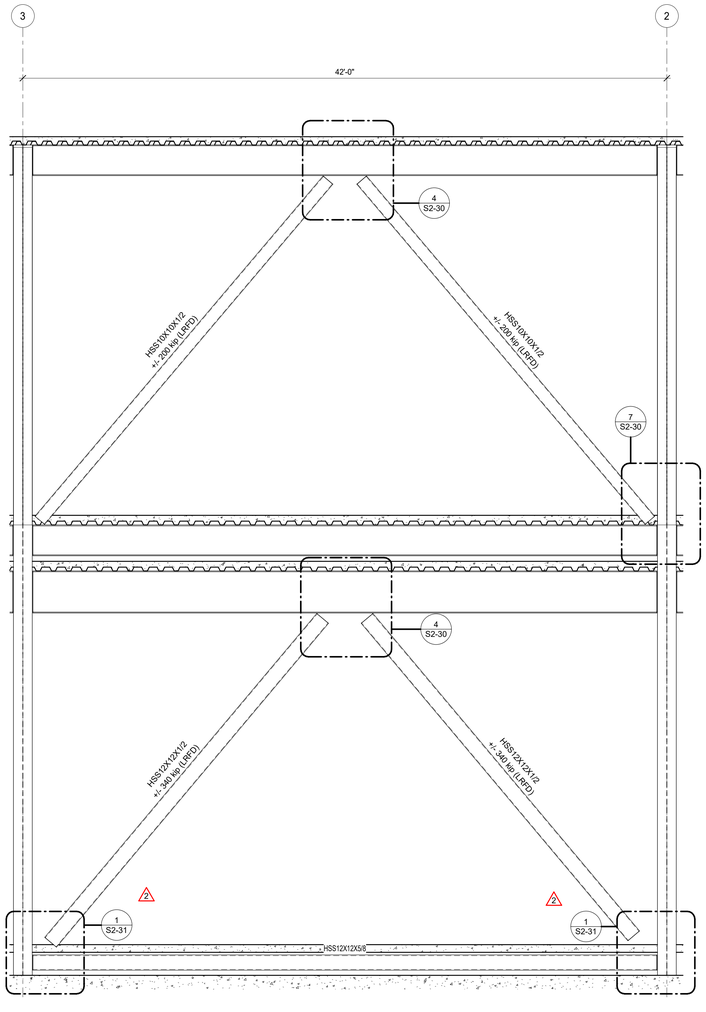
City of Aurora Building Division
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Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



3 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"

9/10/2025 5:07:16 PM S2-06 FRAME ELEVATIONS

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	ISSUES	
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	REVISIONS	
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PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

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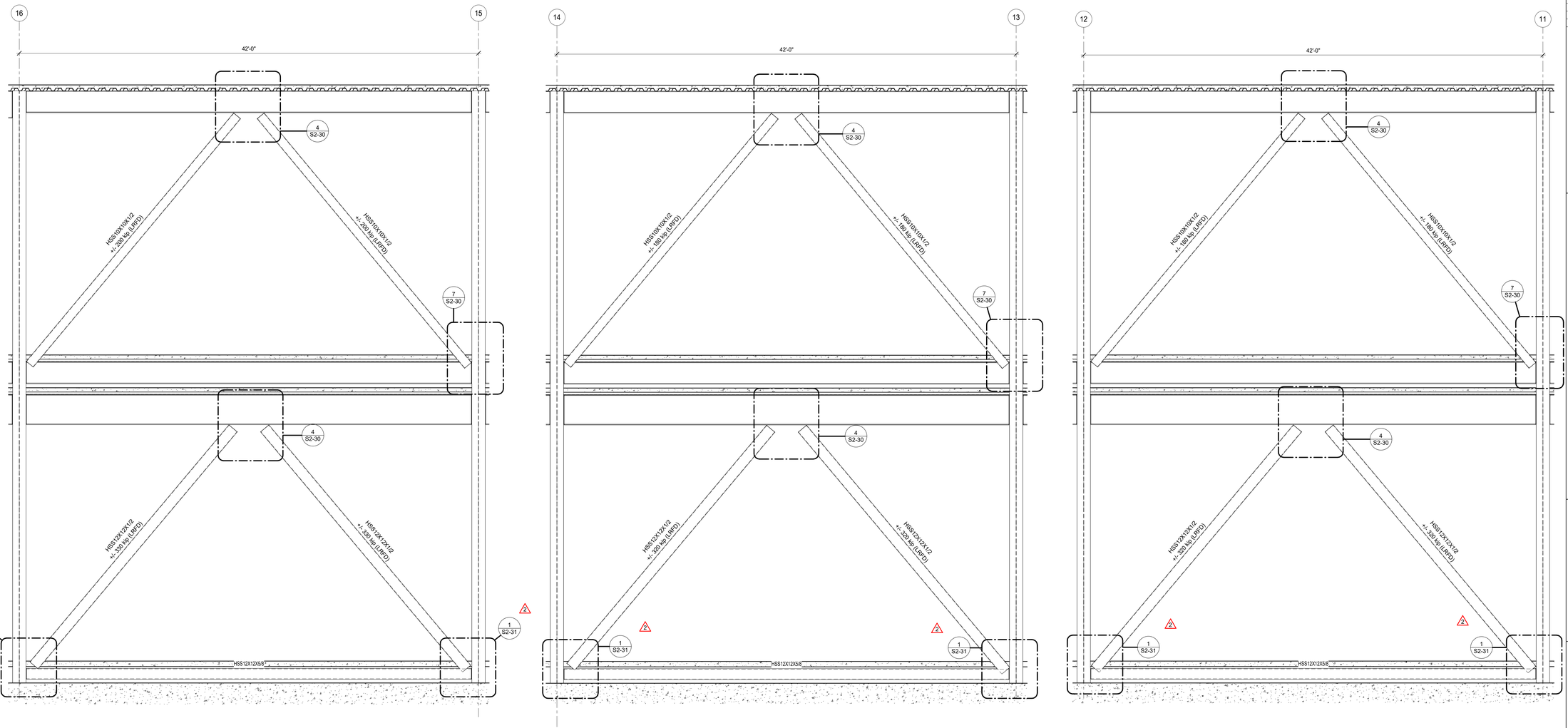
FRAME ELEVATIONS

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DATE 09.10.2025
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S2-06

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: ***Matt A.***
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



3 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"

2 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"

1 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"

9/10/2025 5:07:18 PM S2-07 FRAME ELEVATIONS

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401 N. Houston Street Dallas, TX
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SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

ISSUES		
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS		
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. HEYD
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

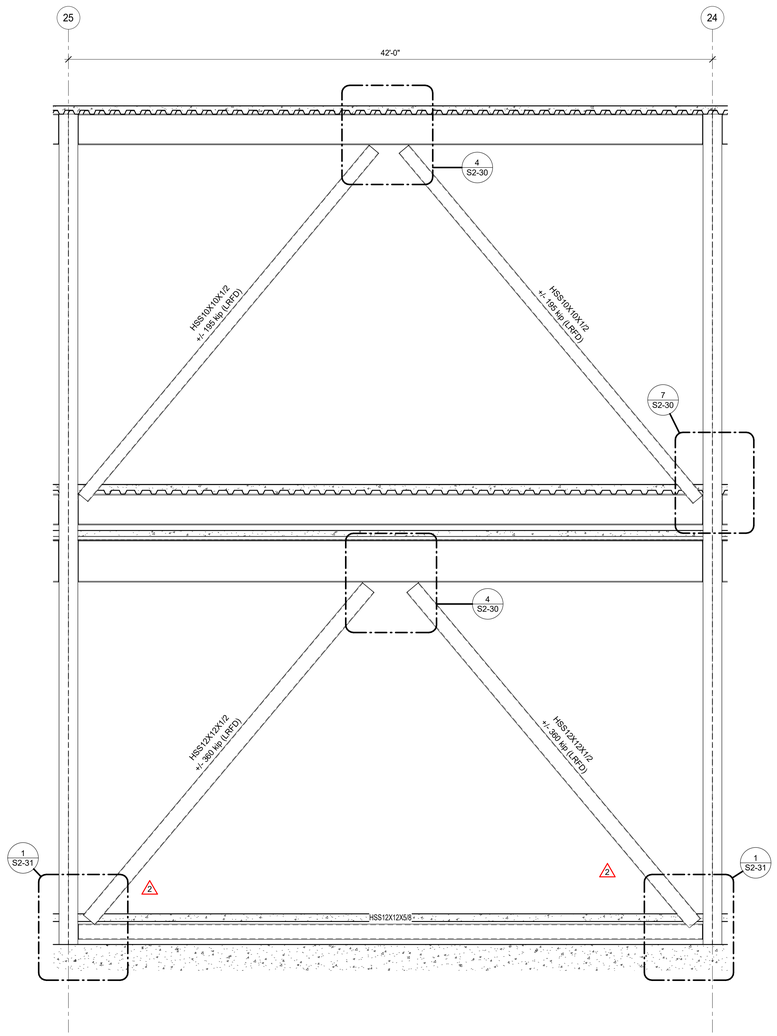
FRAME ELEVATIONS

JOB 24464.0000
DATE 09.10.2025
SHEET

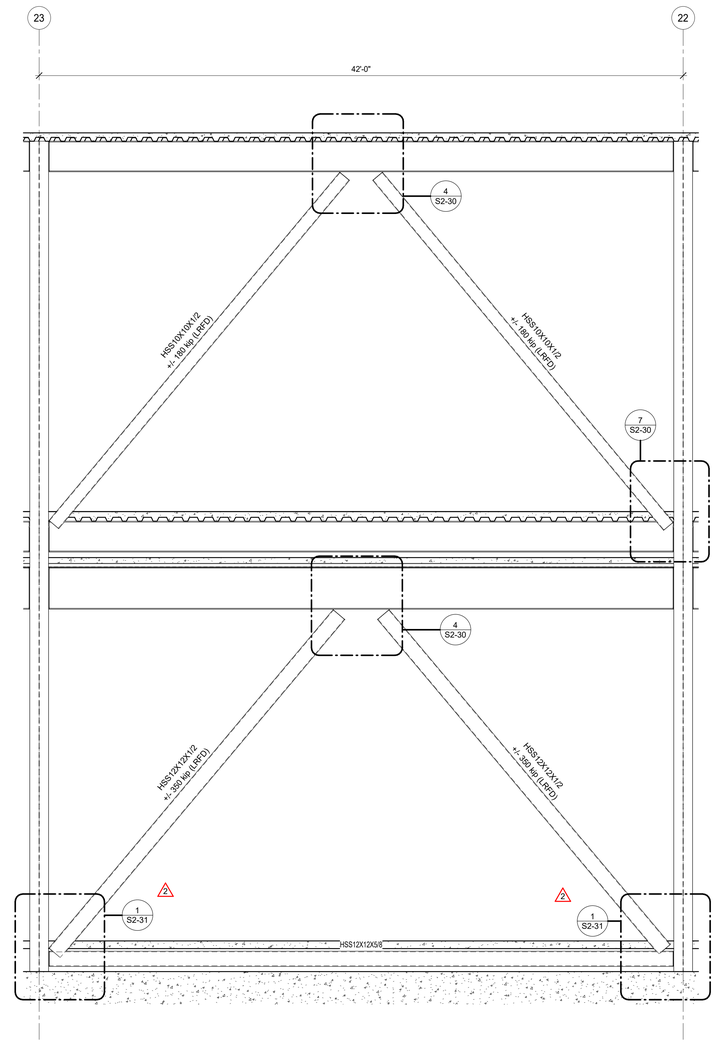
S2-07

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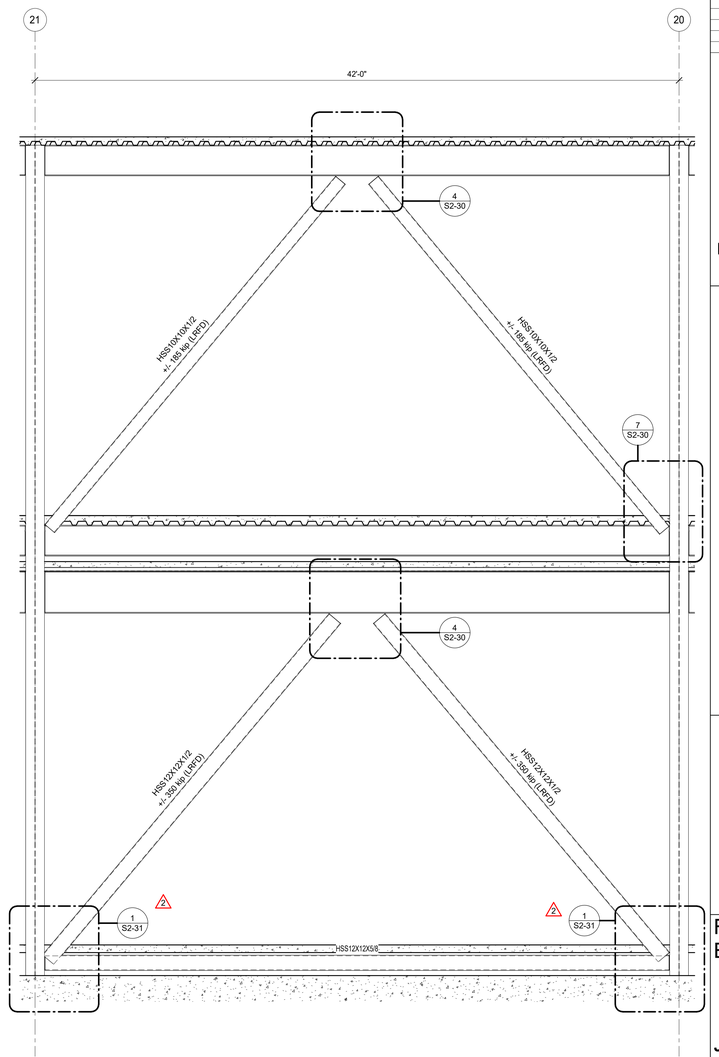
City of Aurora Building Division
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Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC



3 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"

9/10/2025 5:07:21 PM S2-08 FRAME ELEVATIONS

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COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

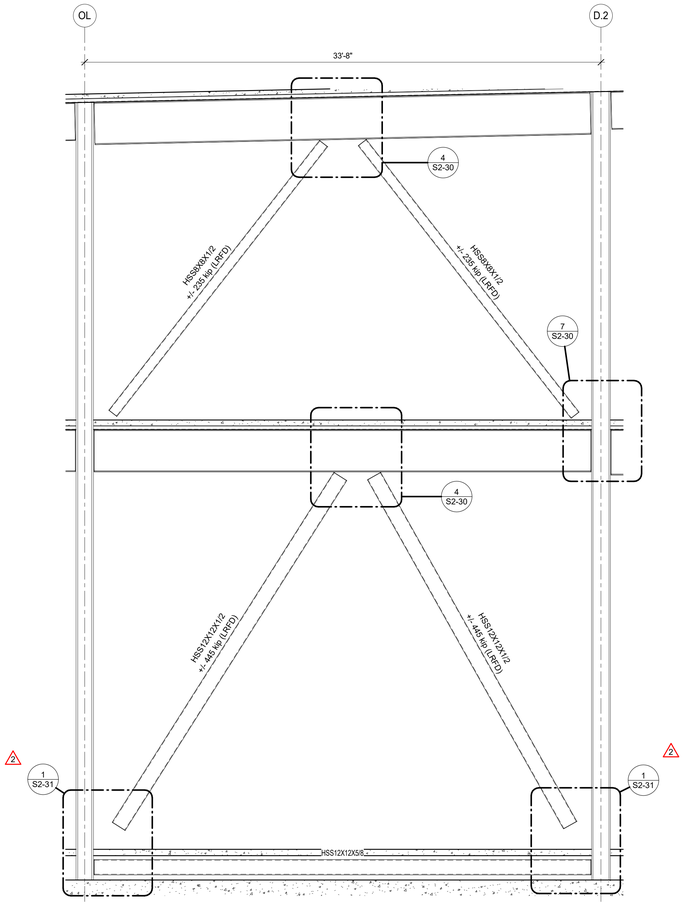
QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

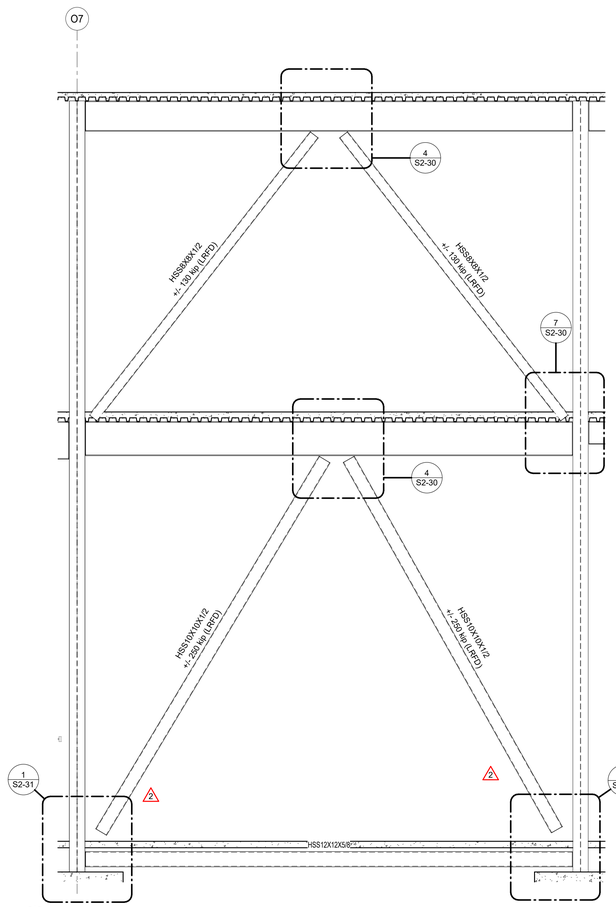
FRAME ELEVATIONS

JOB 24464.0000
DATE 09.10.2025
SHEET

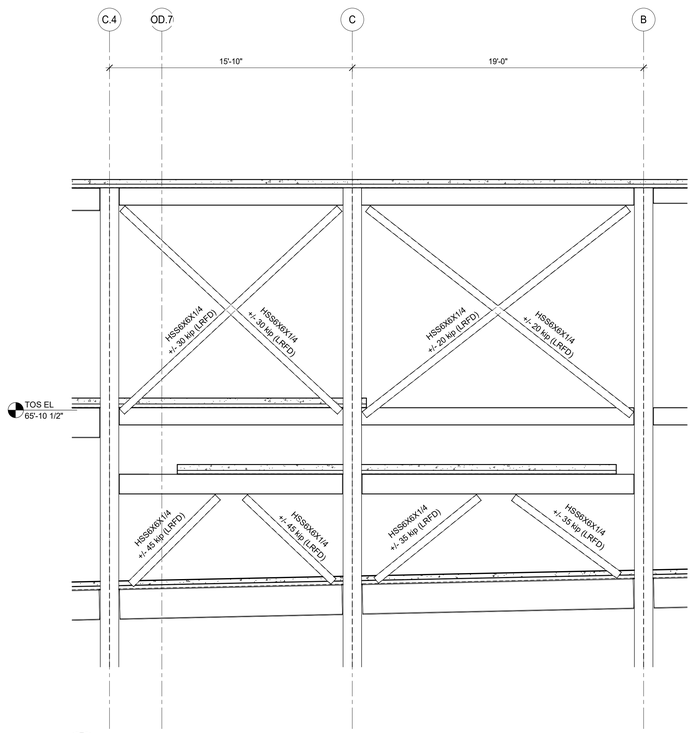
S2-08



4 BRACED FRAME ELEVATION ALONG GRID O7
1/4" = 1'-0"



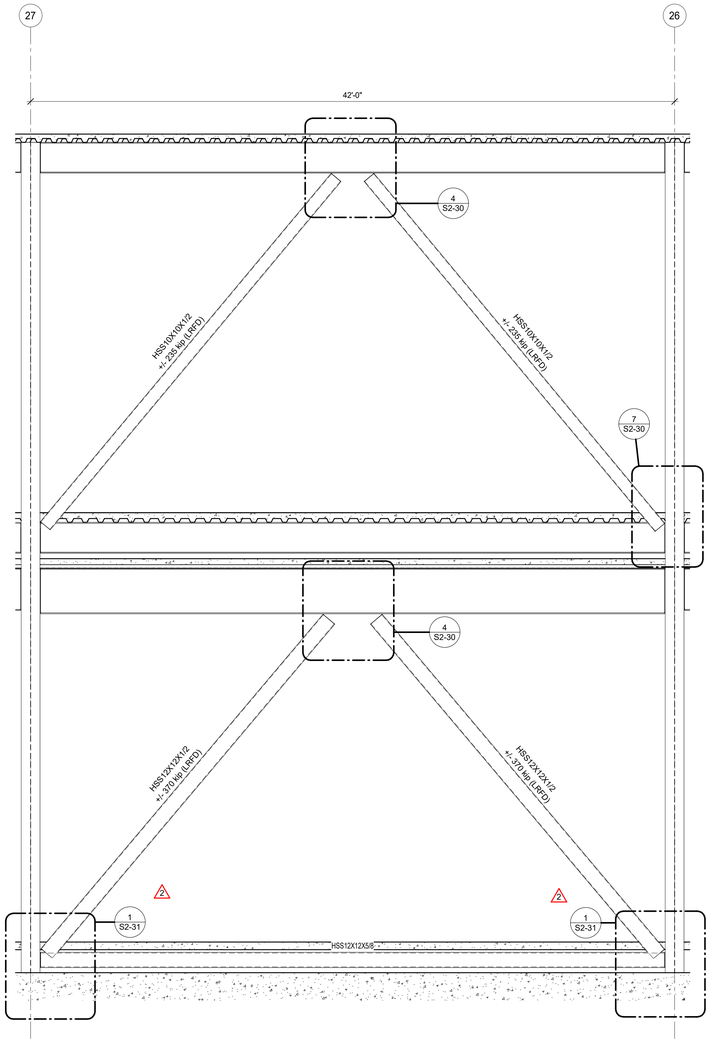
3 BRACED FRAME ELEVATION ALONG GRID OM
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID 30.6
1/4" = 1'-0"

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2021 INTERNATIONAL CODES & 2023 NEC



1 BRACED FRAME ELEVATION ALONG GRID F
1/4" = 1'-0"

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COLORADO LICENSED
NICHOLAS W. HEYD
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

FRAME ELEVATIONS

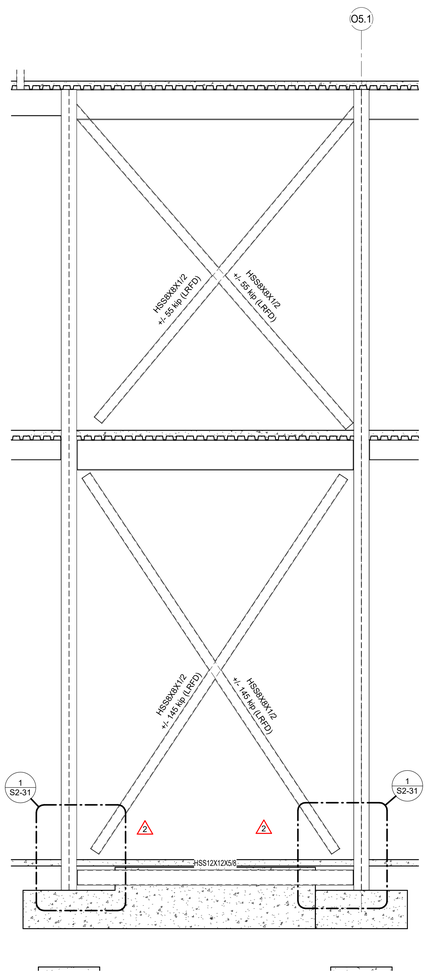
JOB 24464.0000
DATE 09.10.2025
SHEET

S2-09

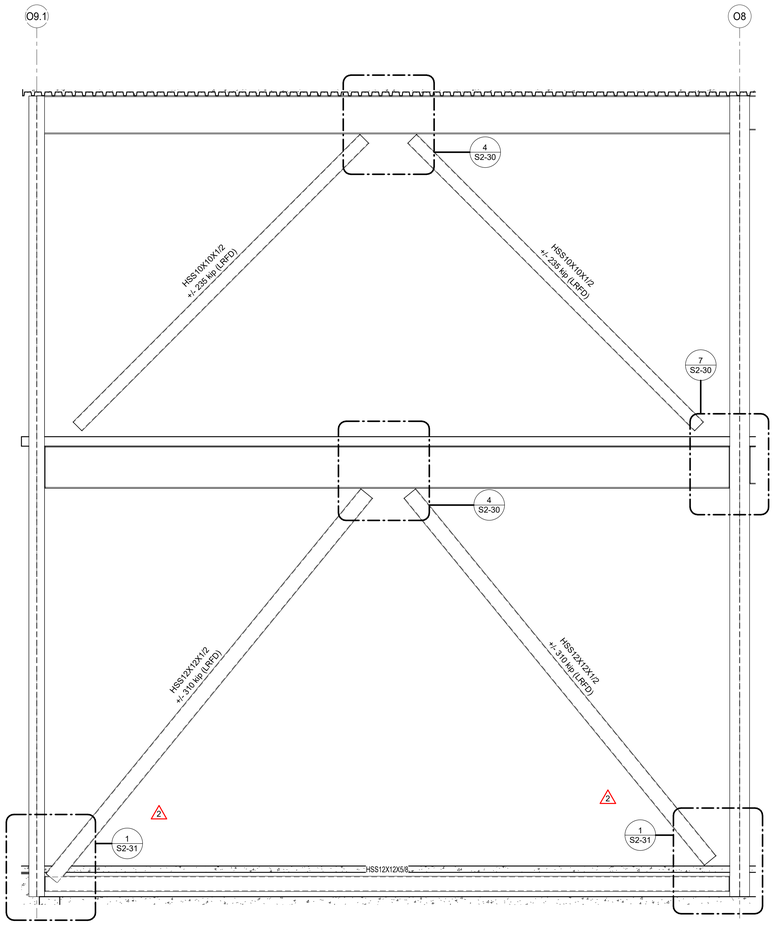
9/10/2025 5:07:24 PM S2-09 FRAME ELEVATIONS

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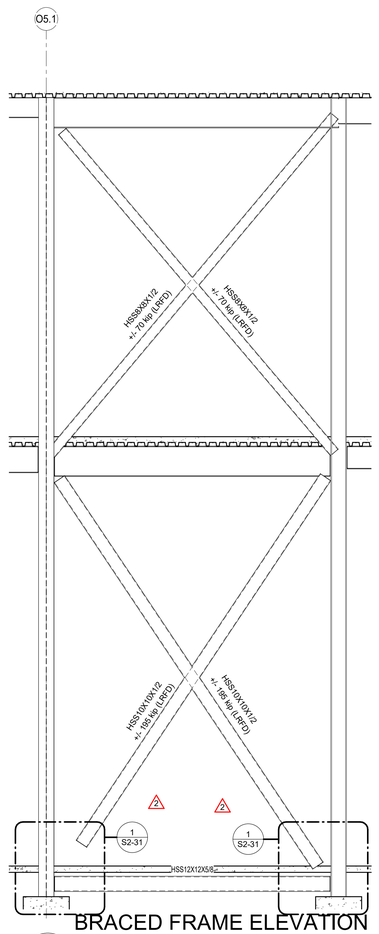
City of Aurora Building Division
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2021 INTERNATIONAL CODES & 2023 NEC



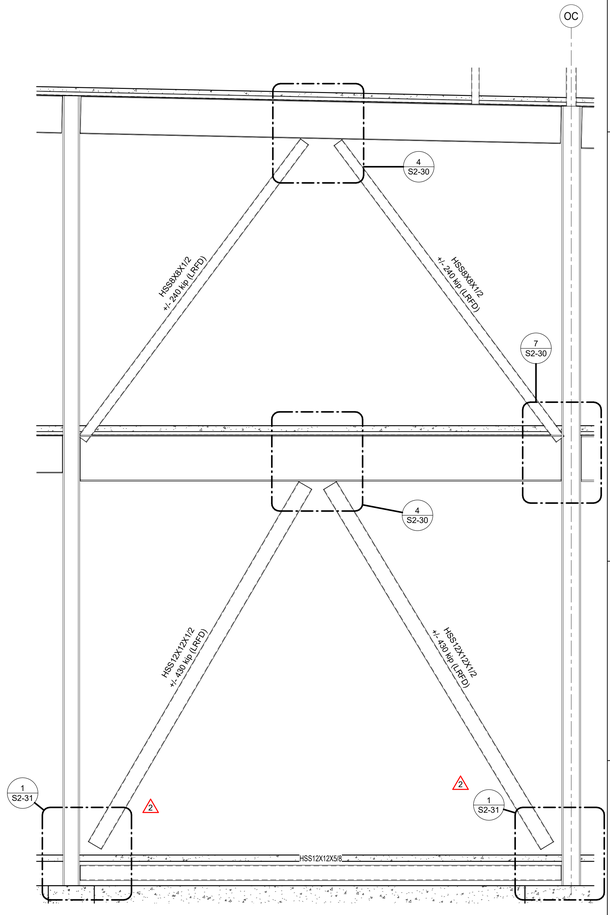
4 BRACED FRAME ELEVATION ALONG GRID OF
1/4" = 1'-0"



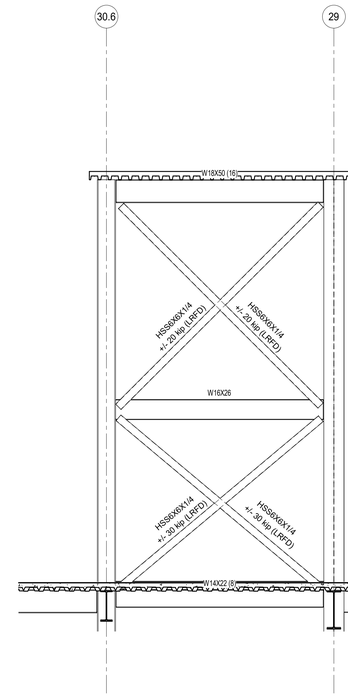
3 BRACED FRAME ELEVATION ALONG GRID OF
1/4" = 1'-0"



2 BRACED FRAME ELEVATION ALONG GRID OH
1/4" = 1'-0"



1 BRACE FRAME ELEVATION ALONG GRID O7
1/4" = 1'-0"



5 BRACE FRAME ELEVATION ALONG GRID D
1/4" = 1'-0"

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COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

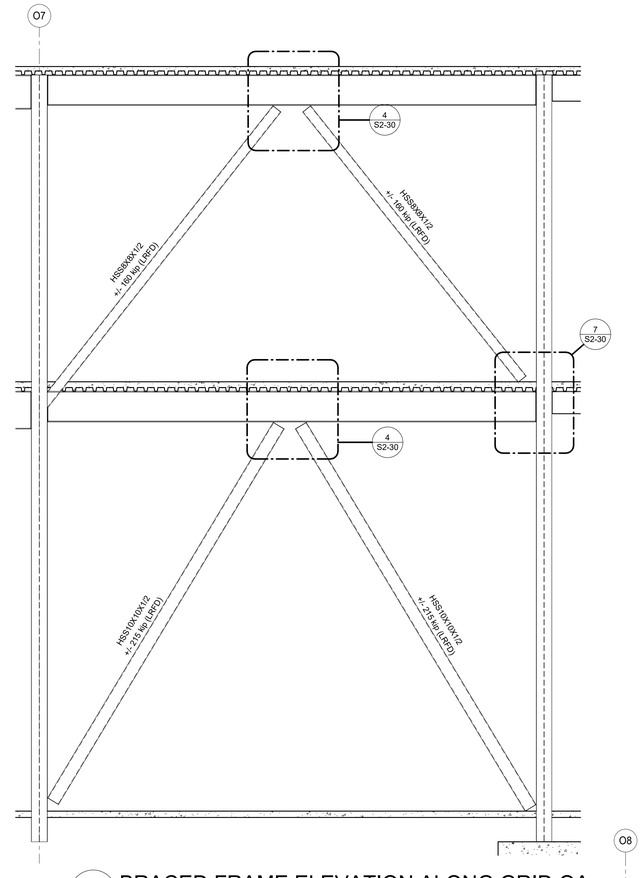
QTS

FRAME ELEVATIONS

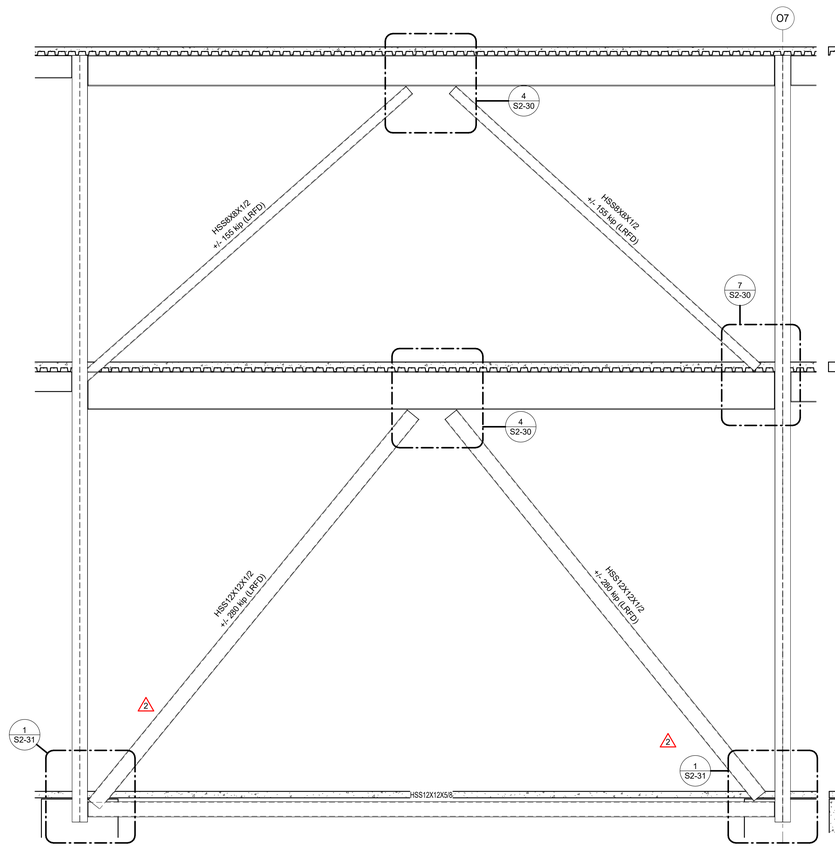
JOB 24464.0000
DATE 09.10.2025
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S2-10

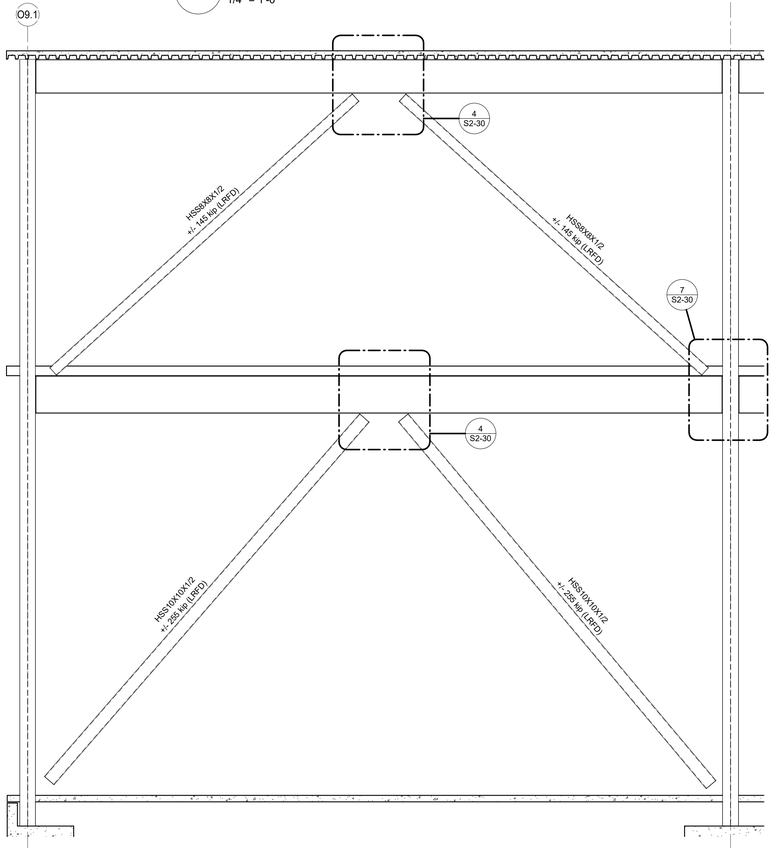
9/10/2025 5:07:29 PM S2-10 FRAME ELEVATIONS



4 BRACED FRAME ELEVATION ALONG GRID OA
1/4" = 1'-0"



2 Elevation 2 - a
1/4" = 1'-0"



1 BRACED FRAME ELEVATION ALONG GRID OA
1/4" = 1'-0"

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PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

FRAME ELEVATIONS

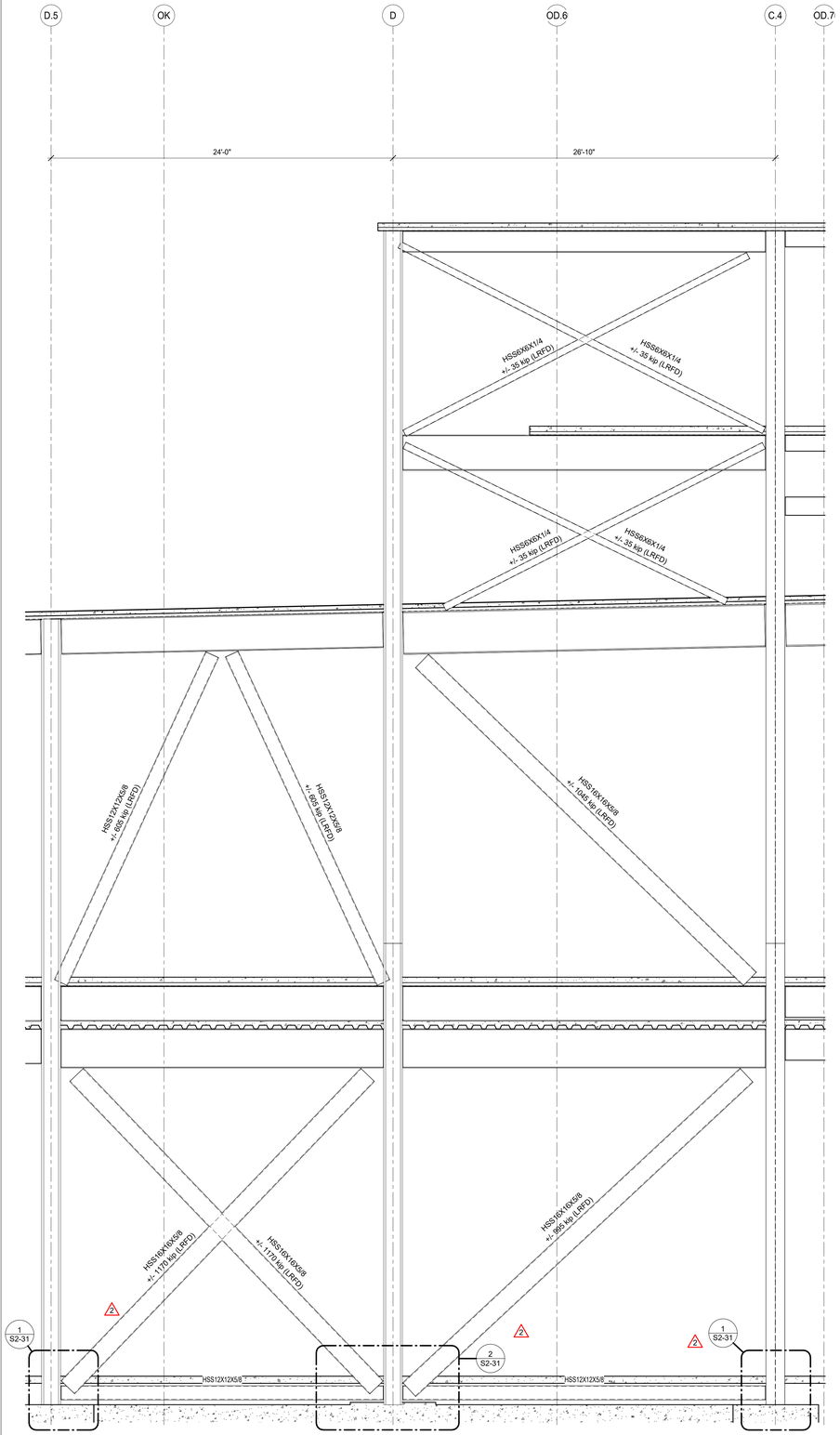
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DATE 09.10.2025
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S2-11

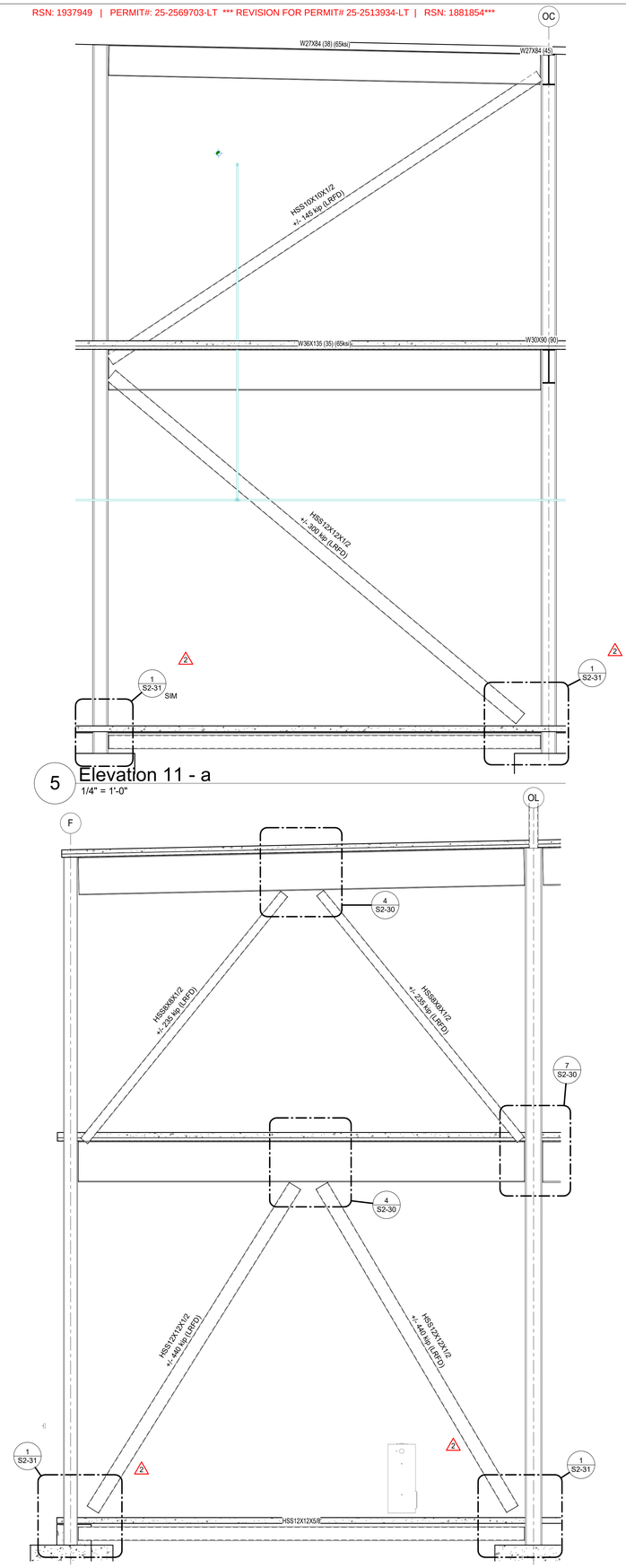
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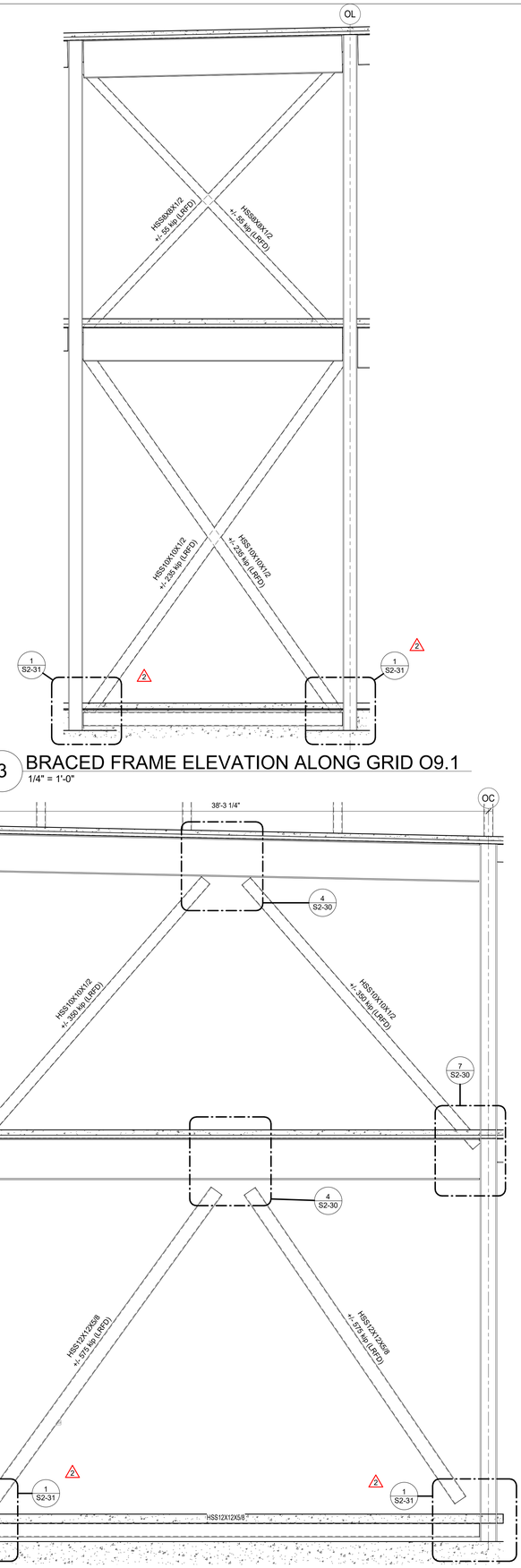


4 BRACED FRAME ELEVATION ALONG GRID 29
1/4" = 1'-0"



5 Elevation 11 - a
1/4" = 1'-0"

2 BRACED FRAME ELEVATION ALONG GRID O4
1/4" = 1'-0"



3 BRACED FRAME ELEVATION ALONG GRID O9.1
1/4" = 1'-0"

1 Elevation 5 - a
1/4" = 1'-0"

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DESIGN CONSULTING LLC

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COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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1140 N. GUN CLUB ROAD

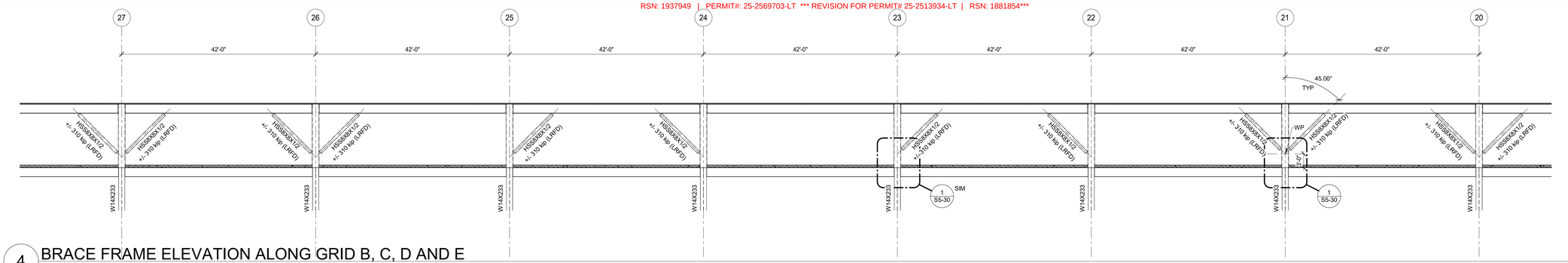
QTS

FRAME ELEVATIONS

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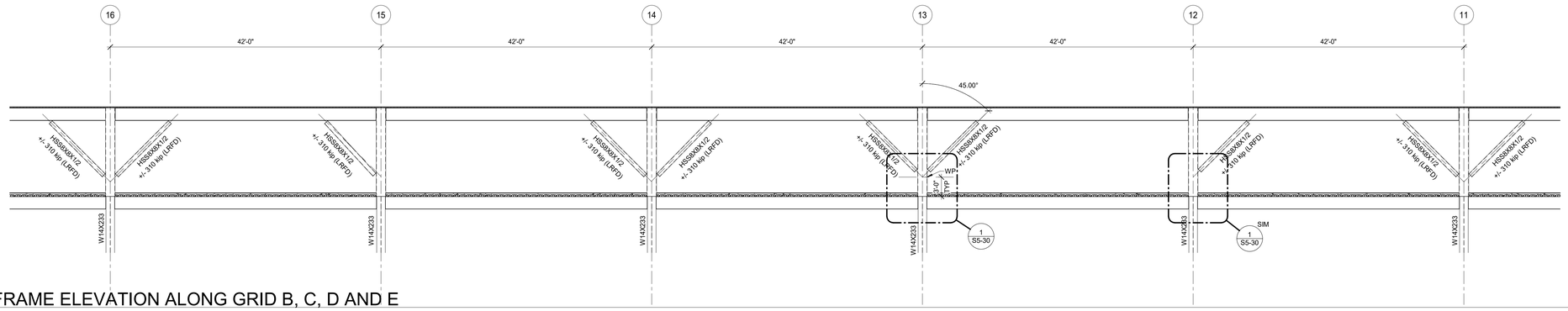
S2-12

9/10/2025 5:07:35 PM S2-12 FRAME ELEVATIONS



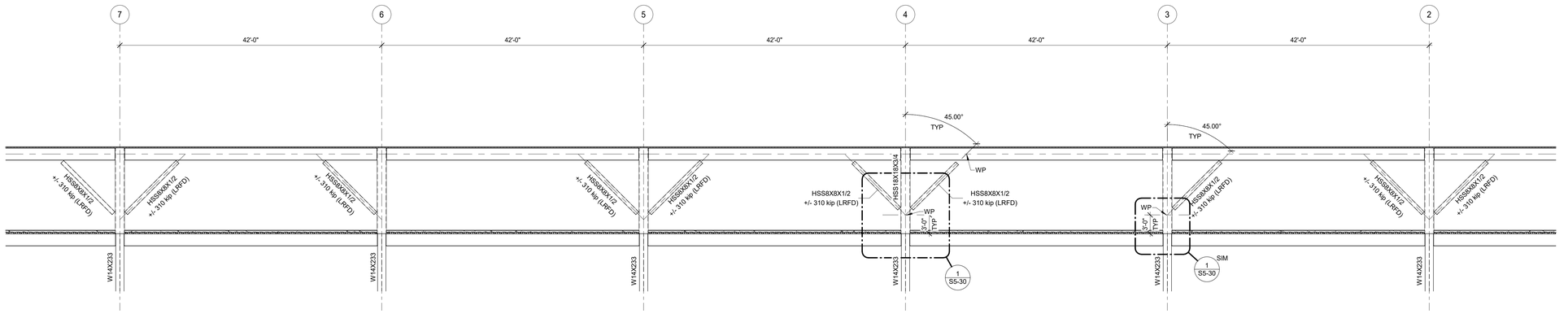
4 BRACE FRAME ELEVATION ALONG GRID B, C, D AND E

1" = 10'-0"



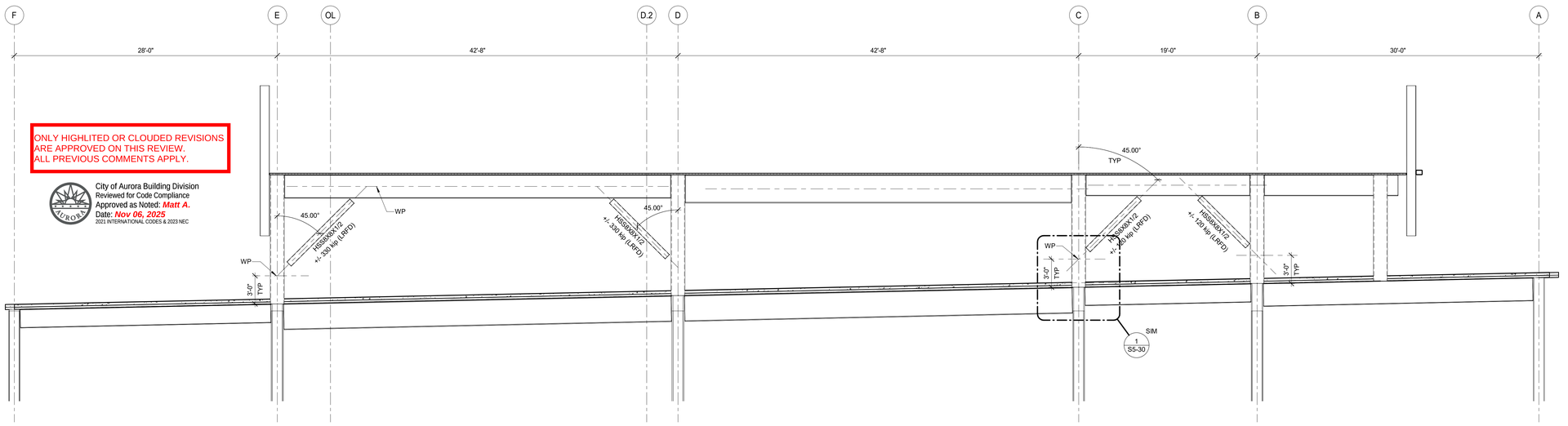
3 BRACE FRAME ELEVATION ALONG GRID B, C, D AND E

1/8" = 1'-0"



2 BRACE FRAME ELEVATION ALONG GRID B, C, D AND E

1/8" = 1'-0"



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1 BRACE FRAME ELEVATION ALONG GRID 1

3/16" = 1'-0"

9/10/2025 5:07:47 PM S2-13 FRAME ELEVATIONS

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COLORADO LICENSED NICHOLAS W. REYID 65236 PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

KEYPLAN

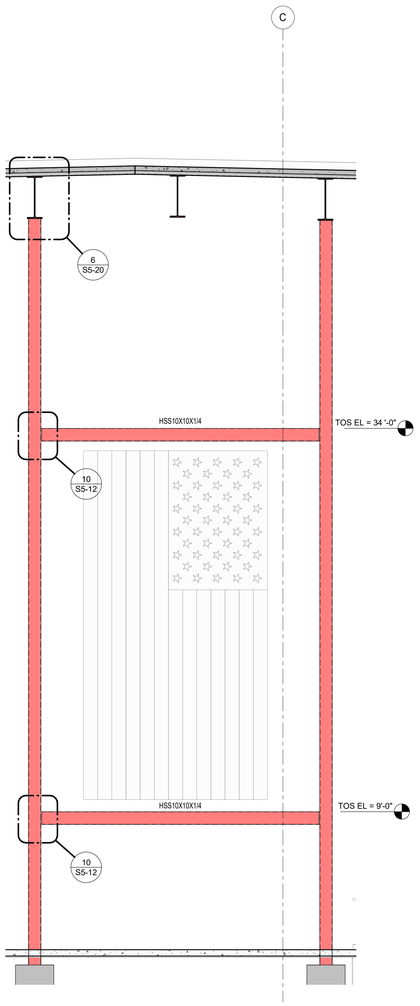
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T	R	P	M	K	H	F	D	B

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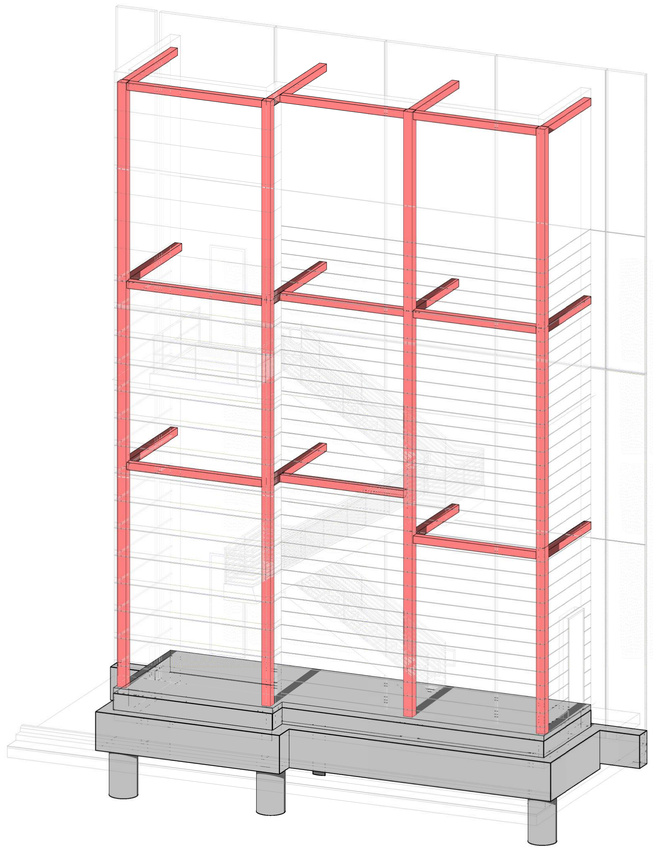
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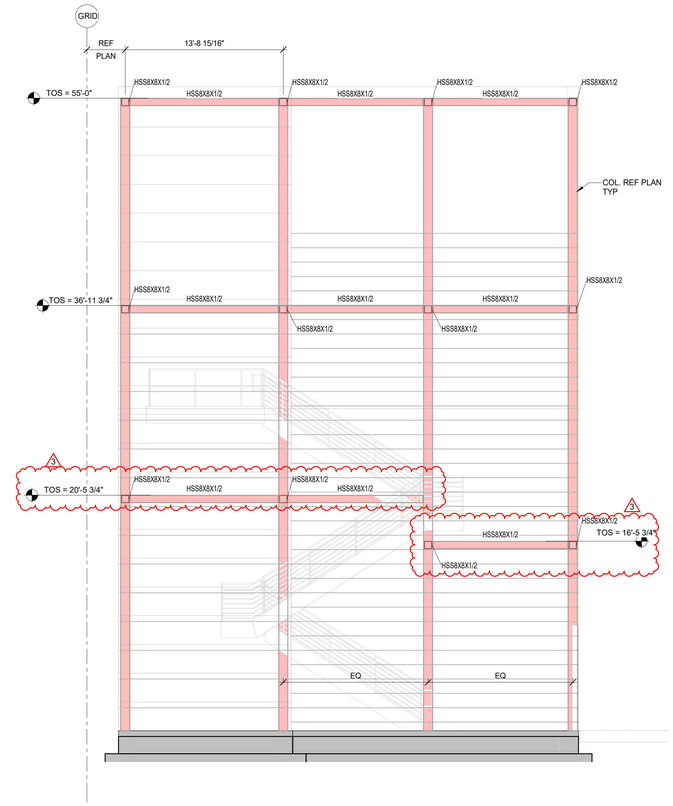
S2-13



2 FLAG SUPPORT ELEVATION
1/4" = 1'-0"



3 3D EXTERIOR STAIR



1 EXTERIOR STAIR ELEVATION
3/16" = 1'-0"

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QTS

EXT STAIR FRAMING ELEVATIONS

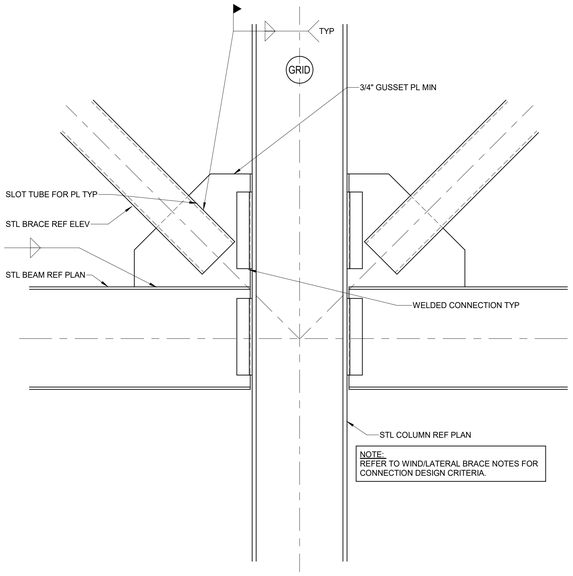
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S2-20

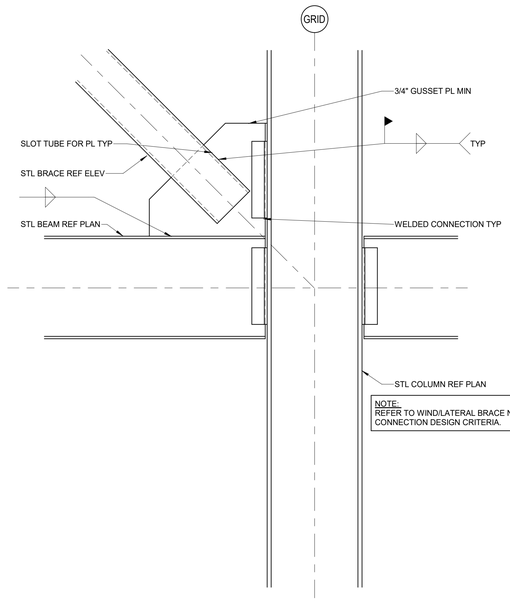
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WIND/LATERAL BRACE NOTES

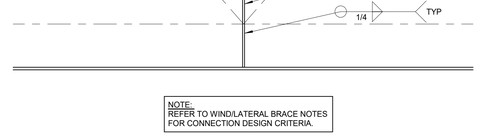
1. DESIGN CONNECTIONS FOR FORCES SHOWN ON BRACE ELEVATIONS. WHERE FORCES ARE NOT GIVEN, DESIGN FOR THE TENSILE CAPACITY OF THE DIAGONAL MEMBERS.
2. ALL WELDS AND GUSSET PLATES SHALL BE DESIGNED BY THE FABRICATOR FOR SHEAR, TENSION, AND ECCENTRICITIES OF FORCES. DESIGN CONNECTIONS USING THE UNIFORM FORCE METHOD PER AISC MANUAL, ASSUMING THE WORKING POINT (W.P.) IS LOCATED AT THE INTERSECTION OF COLUMN AND BEAM CENTERLINES. CHECK TENSION, FLEXURAL, AND RUPTURE CAPACITY OF GUSSET PLATES. AS A MINIMUM, WELDS OF GUSSET PLATES TO BASE PLATES TO BE DESIGNED FOR HORIZONTAL COMPONENT OF DIAGONAL BRACE FORCE. EXTEND BASE PLATE IF REQD TO MATCH WELD LENGTH.
3. ALL CALCULATIONS SHALL BE SUBMITTED WITH THE SHOP DRAWINGS, AND SHALL BE SEALED BY A REGISTERED ENGINEER VERIFYING THE CAPACITY OF EACH CONNECTION.
4. REF SPECIFIC BRACE ELEVATIONS FOR TYPE OF DIAGONAL MEMBERS.
5. ROD DIAGONALS TO HAVE SLEEVE NUTS TO ALLOW FOR TIGHTENING OF RODS.



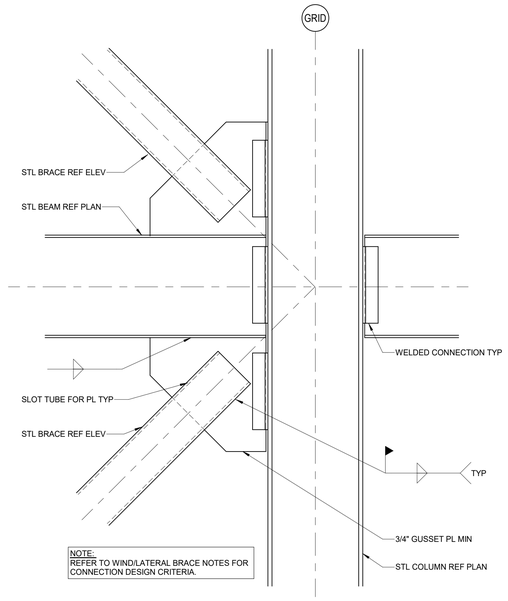
7 TYPICAL BRACE AT BEAM
3/4" = 1'-0"



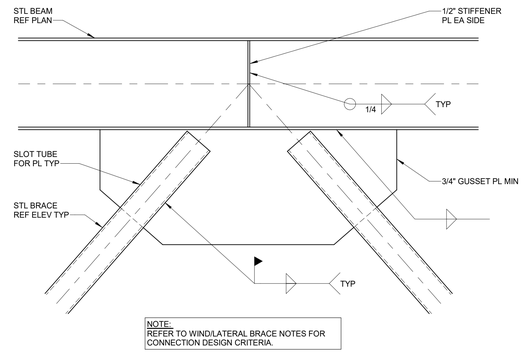
3 TYPICAL BRACE DETAIL
3/4" = 1'-0"



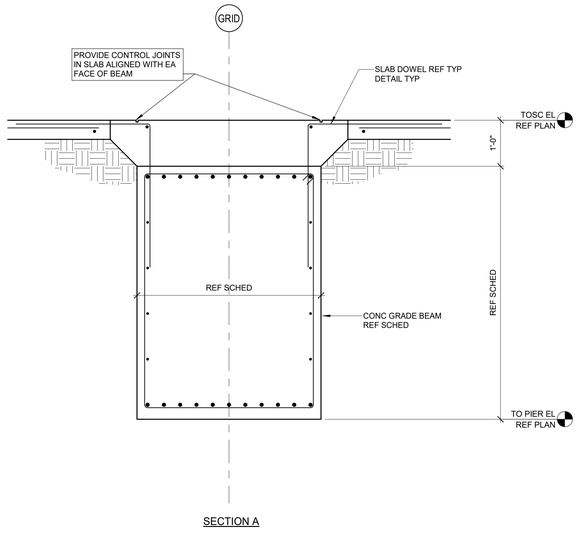
6 TYPICAL BRACE DETAIL
3/4" = 1'-0"



5 TYPICAL BRACE AT BEAM
3/4" = 1'-0"

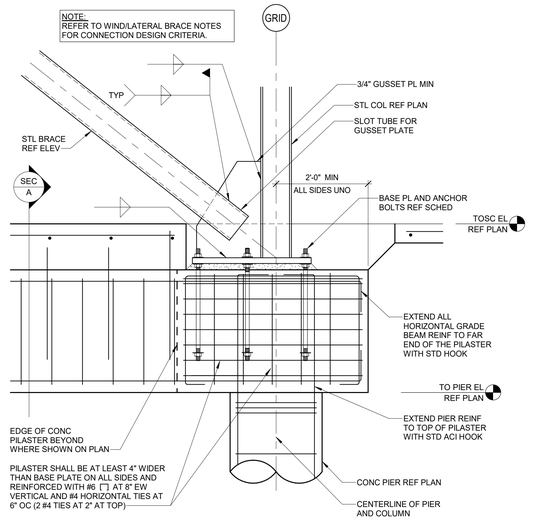


4 TYPICAL BRACE DETAIL
3/4" = 1'-0"

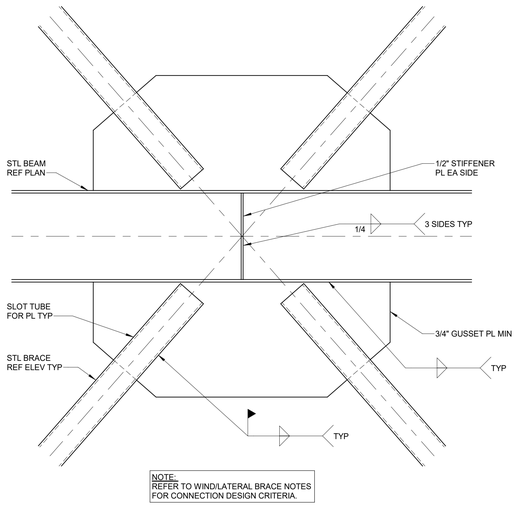


SECTION A

1 TYPICAL BRACED FRAME AT STRAP BEAM
3/4" = 1'-0"



2 TYPICAL BRACE DETAIL
3/4" = 1'-0"



ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW.
ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

TYPICAL BRACING DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

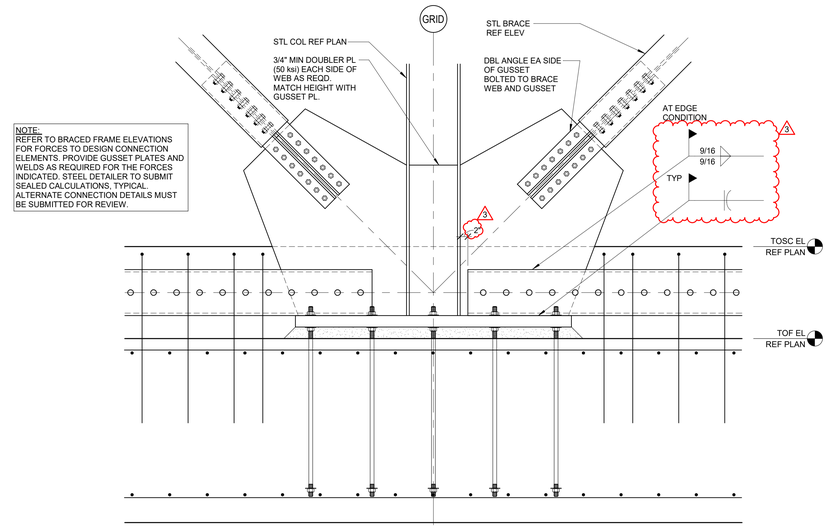
S2-30

9/10/2025 5:07:52 PM S2-30 TYPICAL BRACING DETAILS

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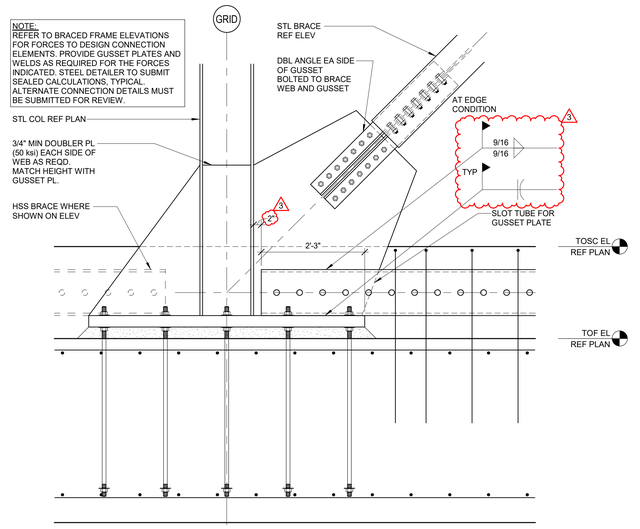
City of Aurora Building Division
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NOTE:
REFER TO BRACED FRAME ELEVATIONS FOR FORCES TO DESIGN CONNECTION ELEMENTS. PROVIDE GUSSET PLATES AND WELDS AS REQUIRED FOR THE FORCES INDICATED. STEEL DETAILER TO SUBMIT SEALED CALCULATIONS, TYPICAL. ALTERNATE CONNECTION DETAILS MUST BE SUBMITTED FOR REVIEW.



2 TYPICAL BRACED FRAME AT FOOTING - DATA HALL - HSSBFD1
3/4" = 1'-0"

NOTE:
REFER TO BRACED FRAME ELEVATIONS FOR FORCES TO DESIGN CONNECTION ELEMENTS. PROVIDE GUSSET PLATES AND WELDS AS REQUIRED FOR THE FORCES INDICATED. STEEL DETAILER TO SUBMIT SEALED CALCULATIONS, TYPICAL. ALTERNATE CONNECTION DETAILS MUST BE SUBMITTED FOR REVIEW.



1 TYPICAL BRACED FRAME AT FOOTING - DATA HALL- HSSBFD2
3/4" = 1'-0"

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75202
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SYSKA HENNESSY GROUP

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	REVISIONS	
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. KEVID
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AG&E Structural Engenuity

Date of issue:
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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

KEYPLAN

S Q N L J G E C A
T R P M K H F D B

N

TYPICAL BRACING DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

S2-31

9/10/2025 5:07:52 PM S2-31 TYPICAL BRACING DETAILS

STEEL REINF CLASS B TENSION LAP SPLICE SCHEDULE L_{st}

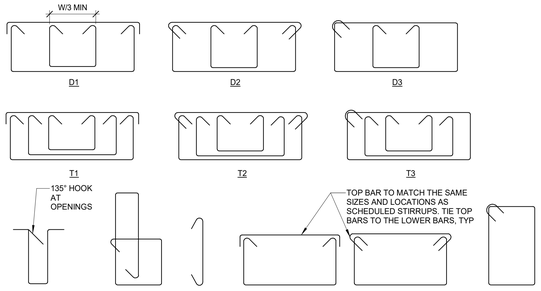
	3000 PSI CONC		4000 PSI CONC		5000 PSI CONC		6000 PSI CONC		7000 PSI CONC		8000 PSI CONC	
	TYP	OTHER	TYP	OTHER	TYP	OTHER	TYP	OTHER	TYP	OTHER	TYP	OTHER
#6 AND SMALLER BARS	74db	57db	64db	50db	58db	44db	53db	41db	49db	38db	46db	35db
#7 TO #11 BARS	93db	71db	81db	62db	72db	55db	66db	51db	61db	47db	57db	44db

NOTES:

1. THE CLASS B TENSION LAP SPLICE SCHEDULE SHOWN IS FOR NORMAL WEIGHT CONCRETE AND GRADE 60 NON EPOXY COATED REINFORCING.
2. TYPICAL IS FOR BEAM TOP AND INTERMEDIATE BARS, WALL TOP AND HORIZONTAL BARS AND SLAB/FOOTING TOP BARS FOR SLAB/FOOTING THICKNESS GREATER THAN 13".
3. FOR EPOXY COATED TYPICAL BARS, INCREASE SPLICE LENGTHS BY 31%. FOR EPOXY COATED OTHER BARS, INCREASE SPLICE LENGTHS BY 50%.
4. FOR LIGHTWEIGHT CONCRETE, INCREASE SPLICE LENGTHS BY 33%.
5. FOR GRADE 75 REBARS, INCREASE SPLICE LENGTHS BY 25%.
6. FOR #7 TO #11 SLAB OR WALL REINFORCING WITH CLEAR COVER LESS THAN 1 BAR DIAMETER, INCREASE SPLICE LENGTHS BY 50%.
7. SPLICE LENGTH FOR BARS OF DIFFERENT SIZE SHALL BE THE GREATER OF SPLICE LENGTH/1.3 OF THE LARGER BAR AND SPLICE LENGTH OF THE SMALLER BAR.
8. FOR CONCRETE STRENGTH IN BETWEEN THOSE SHOWN IN THE SCHEDULE, USE SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.
9. THE SCHEDULED LAP SPLICE LENGTH WITH THE APPLICABLE MODIFACTION FACTORS SHALL NOT BE LESS THAN 12".

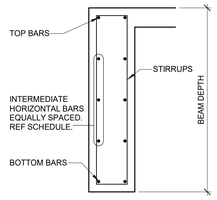
11 TYPICAL STEEL REINF CLASS B TENSION LAP SCHEDULE

3/4" = 1'-0"



10 STIRRUP TYPES AT GRADE BEAMS

3/4" = 1'-0"



INTERMEDIATE BAR SCHEDULE

BEAM DEPTH	NUMBER OF BARS EA FACE	SIZE OF BAR
> 36" - 48"	5	#3
49" - 54"	5	#4
55" - 60"	5	#5
61" - 66"	6	#5
67" - 78"	7	#5
79" AND ABOVE	AT 9" OC	#5

NOTES:

1. ALTERNATE LAP SPLICE INTERMEDIATE BARS BETWEEN SUPPORT AND MIDSPAN. PROVIDE STANDARD ACI HOOK AT DISCONTINUOUS ENDS.
2. BEAMS WITH DEPTH OF 36" OR LESS, DO NOT NEED INTERMEDIATE BARS UNLESS REQUIRED BY BEAM SCHEDULE.
3. WHERE "EXTRA INTERMEDIATE BARS" ARE SCHEDULED, PROVIDE SCHEDULED BARS IN LIEU OF THOSE REQUIRED BY THIS DETAIL.

9 TYPICAL INTERMEDIATE BAR SCHEDULE

3/4" = 1'-0"

CONCRETE GRADE BEAM SCHEDULE

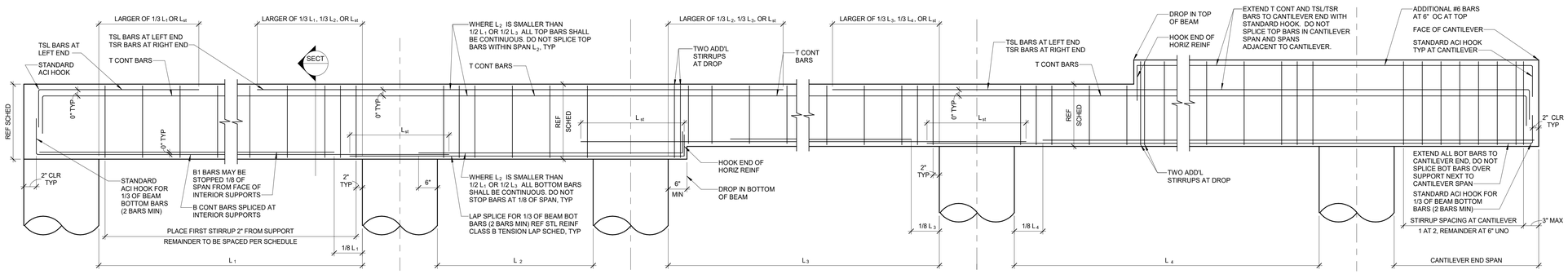
MARK	WIDTH	DEPTH	REINFORCEMENT			STIRRUPS		STIRRUP SPACING FROM EACH END
			TOP	BOTTOM	INTERMEDIATE EA FACE	SIZE	TYPE	
GB1	36"	36"	(6) #8	(6) #8	REF 9-S3-00	#4	S3	1@2", R@12", (R@9" AT CANT)
GB2	36"	42"	(6) #8	(6) #8	REF 9-S3-00	#4	S3	1@2", R@12", (R@9" AT CANT)
GB3	36"	60"	(7) #9	(7) #9	REF 9-S3-00	#4	S3	1@2", R@12", (R@9" AT CANT)
GB4	72"	66"	(15) #14	(15) #14	(10) #5	#5	T3	1@2", R@6", (R@6" AT CANT)
GB5	78"	48"	(16) #8	(16) #8	REF 9-S3-00	#4	S3	1@2", R@12", (R@9" AT CANT)
GB6	24"	24"	(6) #5	(6) #5	REF 9-S3-00	#4	S3	1@2", R@12", (R@9" AT CANT)
GB7	36"	66"	(8) #9	(8) #9	REF 9-S3-00	#4	S3	1@2", R@12", (R@9" AT CANT)

NOTES:

1. REF 7-S3-00 FOR ADDITIONAL INFORMATION.
2. FOR STIRRUP TYPE REF 10-S3-00.
3. REF TYPICAL STL REINF CLASS B TENSION LAP SPLICE SCHEDULE FOR ALL LAP REQUIREMENTS.

8 CONCRETE GRADE BEAM SCHEDULE

3/4" = 1'-0"



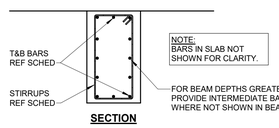
TYP END SUPPORT | **TYP INT SUPPORT** | **TYP INT SUPPORT** | **TYP INT SUPPORT** | **TYP CANTILEVER SUPPORT**

NOTES:

1. L_{st} = LAP SPLICE LENGTH - REF TYP STL REINF CLASS B TENSION LAP SCHED
2. SCHEDULED LEFT AND RIGHT END OF MEMBERS READ ON PLAN AS LEFT AND RIGHT OF THE SHEET OR BOTTOM AND TOP OF THE SHEET RESPECTIVELY AS SHOWN BELOW. SCHEDULED LEFT AND RIGHT END OF MEMBERS SHOWN DIAGONALLY ON PLAN READ AS LEFT AND RIGHT ON SHEET AS SHOWN BELOW:

L R | R L | L R

3. T CONT BARS MAY BE SPLICED AT MID-SPAN OF BEAM ONLY. EXCEPT AT CANTILEVER SPANS, SPANS ADJACENT TO CANTILEVER SPANS AND SPANS WITH LENGTH NOT GREATER THAN ONE HALF OF THE ADJACENT SPAN LENGTH.
4. ALL HOOKS SHOWN ARE STANDARD HOOKS (90° OR 180°).
5. SPLICES IN SCHEDULED BARS LESS THAN 60'-0" LENGTH ARE ONLY PERMITTED AT THE SPECIFIED LOCATIONS SHOWN. SPLICES IN SCHEDULED BARS MORE THAN 60'-0" LENGTH SHALL BE REFERRED TO ENGINEER FOR APPROVAL.



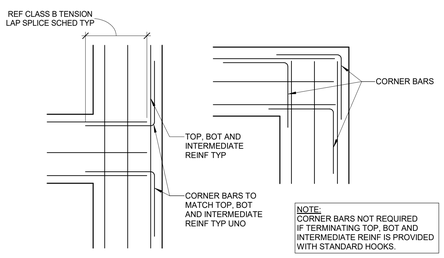
NOTE: BARS IN SLAB NOT SHOWN FOR CLARITY.

FOR BEAM DEPTHS GREATER THAN 36" PROVIDE INTERMEDIATE BARS PER 9-S3-00 WHERE NOT SHOWN IN BEAM SCHEDULE

SECTION

7 TYPICAL CONCRETE GRADE BEAM BAR PLACING DIAGRAM

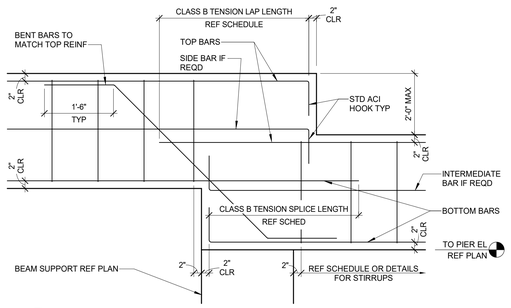
3/4" = 1'-0"



NOTE: CORNER BARS NOT REQUIRED IF TERMINATING TOP, BOT AND INTERMEDIATE REINF IS PROVIDED WITH STANDARD HOOKS.

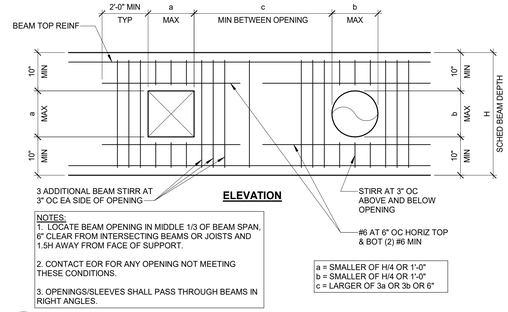
6 TYPICAL CORNER BAR REINFORCING

3/4" = 1'-0"



5 TYPICAL BEAM STEP

3/4" = 1'-0"



ELEVATION

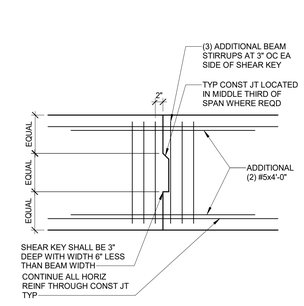
NOTES:

1. LOCATE BEAM OPENING IN MIDDLE 1/3 OF BEAM SPAN, 6" CLEAR FROM INTERSECTING BEAMS OR JOISTS AND 1.5H AWAY FROM FACE OF SUPPORT.
2. CONTACT EOR FOR ANY OPENING NOT MEETING THESE CONDITIONS.
3. OPENINGS/SLEEVES SHALL PASS THROUGH BEAMS IN RIGHT ANGLES.

a = SMALLER OF H/4 OR 1'-0"
b = SMALLER OF H/4 OR 1'-0"
c = LARGER OF 3a OR 3b OR 6"

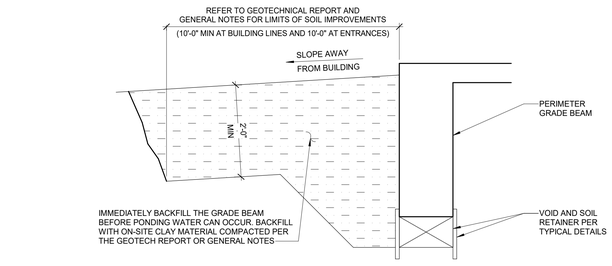
4 TYPICAL HORIZONTAL OPENING IN BEAM

3/4" = 1'-0"

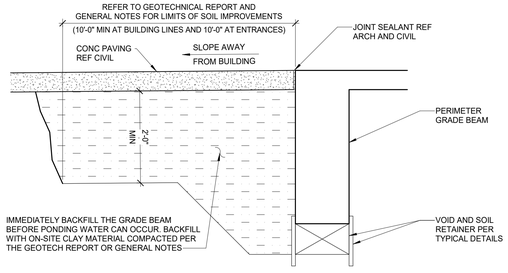


3 TYPICAL BEAM CONST JT

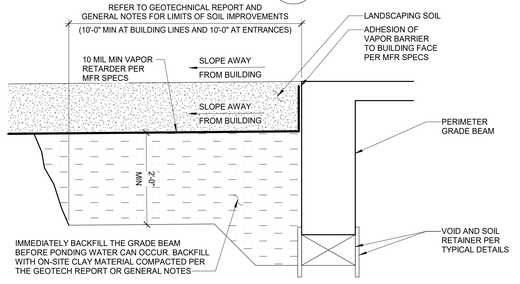
3/4" = 1'-0"



WITHOUT ADJACENT CONCRETE PAVING OR LANDSCAPING / IRRIGATION



WITH ADJACENT PERMANENT CONCRETE PAVING



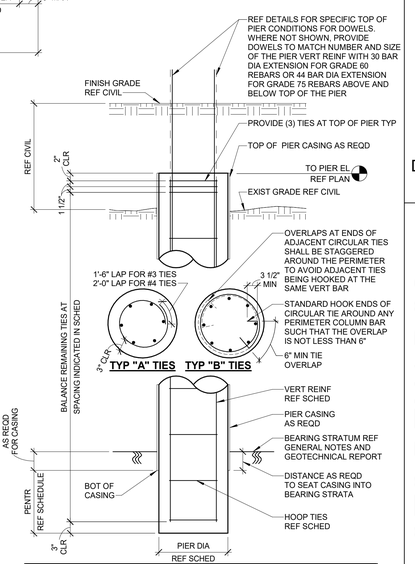
WITH ADJACENT LANDSCAPING

NOTES:

1. IF SELECT FILL OR ENGINEERED FILL IS UTILIZED BELOW SLAB, THE DEPTH OF GRADE BEAM OR PANEL MUST BE AT LEAST 6" BELOW THE BOT OF SUCH FILL.
2. THE DETAILS OF THE SOIL IMPROVEMENT PROCESS SUCH AS MOISTURE CONDITIONING OR WATER INJECTION ARE PRESENTED IN THE GEOTECHNICAL REPORT. USE OF IMPORTED SOIL IN THIS ZONE IS PROHIBITED.
3. ALL TREES SHALL BE PLANTED WITH PERMANENT ROOT BARRIERS.

2 TYPICAL BACKFILL DETAILS AT BUILDING PERIMETER

3/4" = 1'-0"



PIER SCHEDULE

MARK	PIER DIAMETER	PENTR	VERT REINF	HOOP TIES
P24	24"	20'-0"	(10) #8	#3 AT 12" OC
P30	30"	20'-0"	(12) #8	#4 AT 12" OC
P36	36"	25'-0"	(14) #8	#4 AT 12" OC
P42	42"	25'-0"	(14) #9	#4 AT 12" OC
P48	48"	30'-0"	(20) #9	#4 AT 12" OC
P54	54"	30'-0"	(20) #10	#4 AT 12" OC
P60	60"	35'-0"	(24) #10	#4 AT 12" OC
P66	66"	40'-0"	(22) #11	#4 AT 12" OC
P72	72"	45'-0"	(26) #11	#4 AT 12" OC
P72A	72"	55'-0"	(36) #14	#4 AT 12" OC

NOTE(S):

1. DEPTH OF BEARING STRATUM IS INDICATED FOR INFORMATION ONLY. ACTUAL DEPTH MAY VARY. REFER TO THE GEOTECHNICAL REPORT AND THE GRADING PLAN FOR DEPTH OF BEARING STRATUM BELOW FINISH FLOOR, AND ADJUST DEPTHS AS REQD. CONTRACTOR TO PROVIDE AN ALLOWANCE ON PER LINEAR FOOT BASIS FOR ADD AND DEDUCT. ALL PIERS SHALL EXTEND A MINIMUM OF 35FT BELOW TOP OF PIER ELEVATION, REFER TO GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
2. SPLICE VERTICAL REINFORCING IN PIERS WITH CLASS B TENSION LAP SPLICE. NO MORE THAN 50% OF THE BARS SHALL BE LAP SPLICED AT ONE LOCATION.
3. FOR PIERS LOCATED WITHIN TWO SHAFT DIAMETERS CLEAR SPACING, THE SECOND PIER SHALL BE DRILLED A MIN OF 48 HOURS AFTER THE FIRST.
4. REFER TO THE GEOTECHNICAL REPORT FOR PIER CASING REQUIREMENTS. IF CASING IS REQUIRED, CASING SHALL BE SIZED TO MAINTAIN THE PIER DIAMETER INDICATED IN SCHEDULE.
5. USE TYPE "A" TIES, TYP, UNO. USE TYPE "B" TIES FOR PIERS ABOVE GRADE (INCLUDING PIERS ABOVE GRADE IN CRAWL SPACE), IN SEISMIC DESIGN CATEGORY D, E, & F AND WHERE SPECIFIED IN SCHEDULE.
6. PROVIDE 3" HIGH x 3" DEEP SHEAR RINGS AT 16" OC AT BOTTOM 15' OF ALL PIERS.

1 TYPICAL PIER REINFORCING DIAGRAM AND SCHEDULE

3/4" = 1'-0"

CORGAN
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REVISIONS

2	05.30.25	ISSUE FOR CONSTRUCTION
B	05.05.25	COA - PERMIT COMMENTS R2

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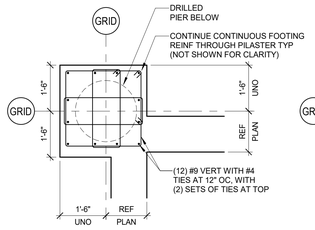
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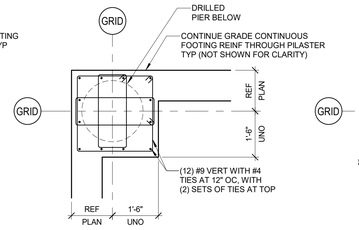
PIER, GRADE BEAM SCHEDULE AND TYPICAL FOUNDATION DETAILS

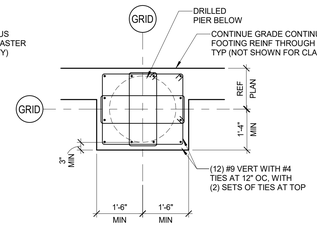
JOB 24464.0000
DATE 09.10.2025
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S3-00

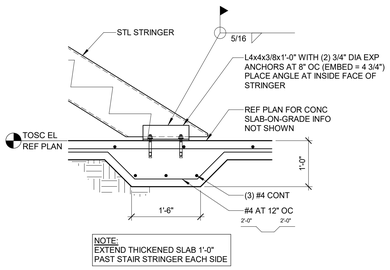
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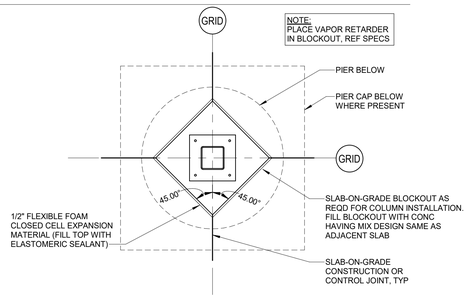


20 TYPICAL PILASTER DETAILS
1/2" = 1'-0"

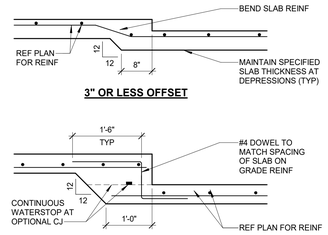


NOTE:
EXTEND THICKENED SLAB 1'-0" PAST STAIR STRINGER EACH SIDE

15 TYPICAL MTL STAIR AT SLAB-ON-GRADE
3/4" = 1'-0"

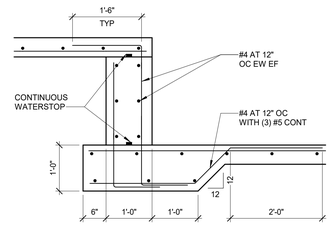


14 TYP COLUMN BLOCKOUT (SLAB-ON-GRADE ONLY)
3/4" = 1'-0"



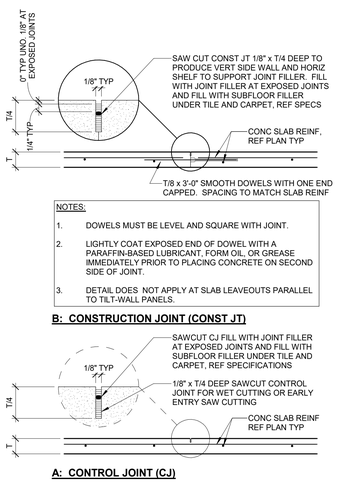
3" OR LESS OFFSET

3" TO 18" OFFSET



18" TO 36" OFFSET

9 TYPICAL OFFSETS IN SLAB-ON-GRADE
3/4" = 1'-0"

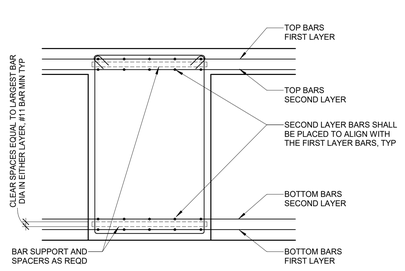


NOTES:
1. DOWELS MUST BE LEVEL AND SQUARE WITH JOINT.
2. LIGHTLY COAT EXPOSED END OF DOWEL WITH A PARAFFIN-BASED LUBRICANT, FORM OIL, OR GREASE IMMEDIATELY PRIOR TO PLACING CONCRETE ON SECOND SIDE OF JOINT.
3. DETAIL DOES NOT APPLY AT SLAB LEAVEOUTS PARALLEL TO TILT-WALL PANELS.

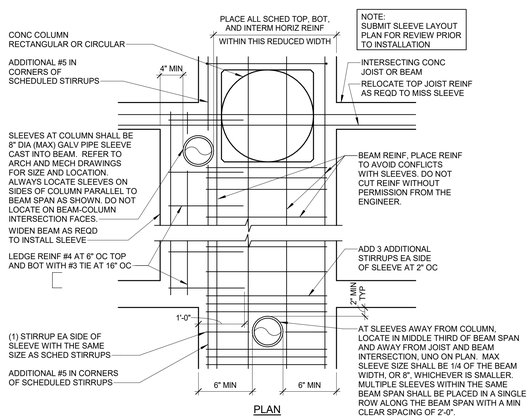
B: CONSTRUCTION JOINT (CONST JT)

A: CONTROL JOINT (CJ)

8 TYP SLAB-ON-GRADE JOINTS
3/4" = 1'-0"

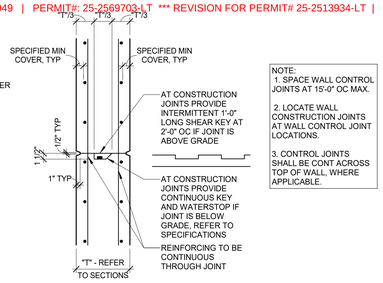


5 TYPICAL DETAIL OF MULTIPLE LAYERS OF BEAM REINFORCING
3/4" = 1'-0"



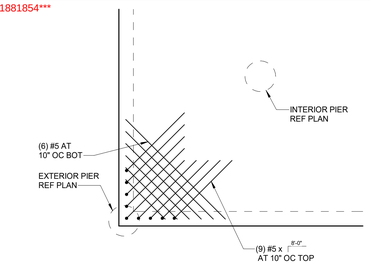
PLAN

4 TYPICAL VERTICAL SLEEVES IN CONCRETE BEAMS
3/4" = 1'-0"



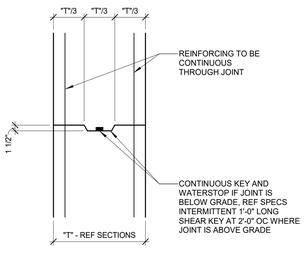
NOTE:
1. SPACE WALL CONTROL JOINTS AT 15'-0" OC MAX.
2. LOCATE WALL CONSTRUCTION JOINTS AT WALL CONTROL JOINT LOCATIONS.
3. CONTROL JOINTS SHALL BE CONT ACROSS TOP OF WALL, WHERE APPLICABLE.

18 TYPICAL VERTICAL CONTROL/ CONSTRUCTION JOINT IN WALL
3/4" = 1'-0"

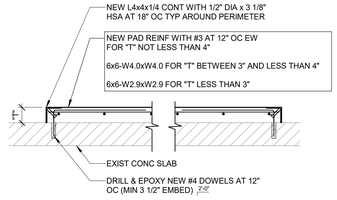


PLAN

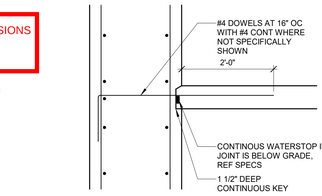
17 CORNER REINFORCING
NTS



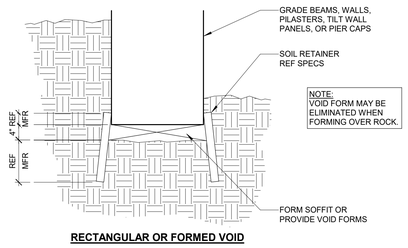
16 TYPICAL HORIZONTAL CONSTRUCTION JOINT IN WALL OR BEAM
3/4" = 1'-0"



13 TYPICAL HOUSEKEEPING PAD OR TOPPING SLAB
3/4" = 1'-0"

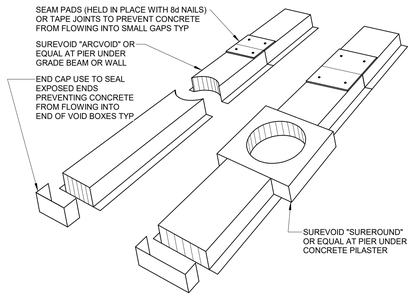


12 TYPICAL SLAB SHEAR KEY TO BEAM OR WALL
3/4" = 1'-0"

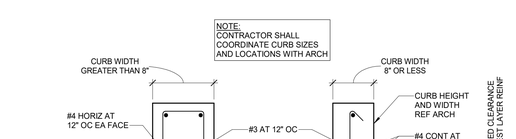


RECTANGULAR OR FORMED VOID

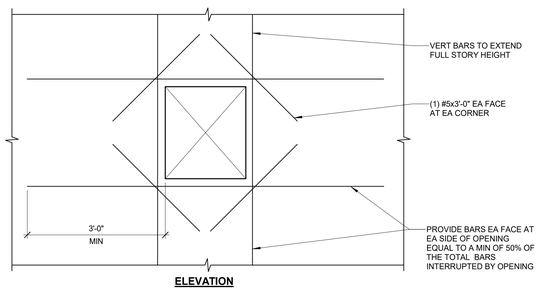
11 TYPICAL GRADE BEAMS/WALLS OVER VOID BOXES
3/4" = 1'-0"



10 TYPICAL VOID FORMS AT PIERS
3/4" = 1'-0"

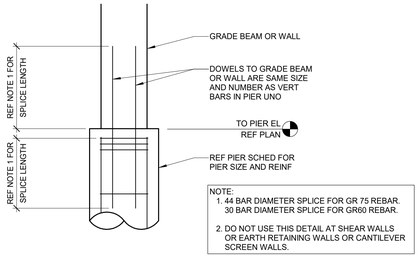


7 TYPICAL CONCRETE CURB
1" = 1'-0"



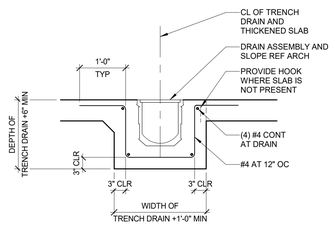
ELEVATION

6 TYPICAL OPENING IN CONCRETE WALL
3/4" = 1'-0"

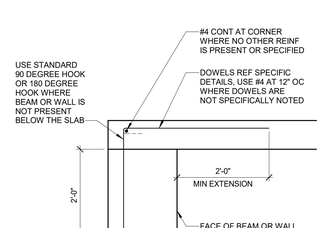


NOTE:
1. 44 BAR DIAMETER SPLICE FOR GR 75 REBAR. 30 BAR DIAMETER SPLICE FOR GR60 REBAR.
2. DO NOT USE THIS DETAIL AT SHEAR WALLS OR EARTH RETAINING WALLS OR CANTILEVER SCREEN WALLS.

3 TYPICAL DOWELS TO PIER AT CONCRETE WALL OR GRADE BEAM
3/4" = 1'-0"



2 TYPICAL TRENCH DRAIN AT SLAB-ON-GRADE
3/4" = 1'-0"



1 TYPICAL SLAB DOWEL
3/4" = 1'-0"

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW.
ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

CORGAN
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T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
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6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

TYPICAL FOUNDATION DETAILS

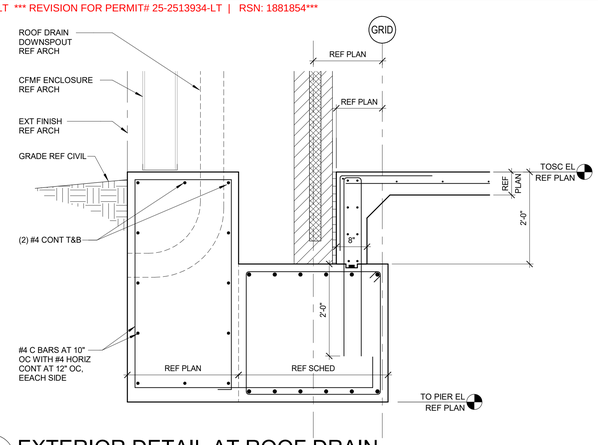
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S3-01

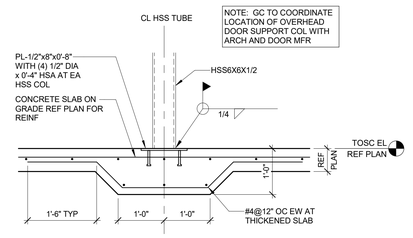
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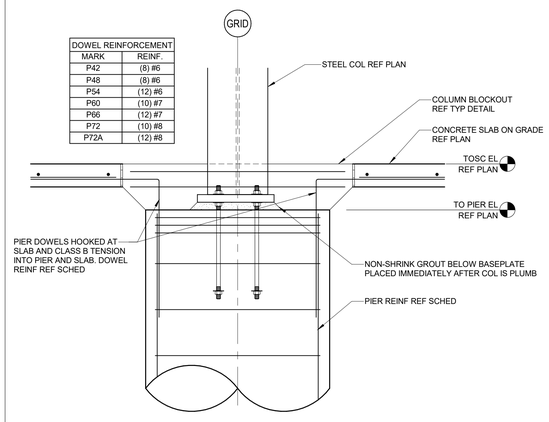
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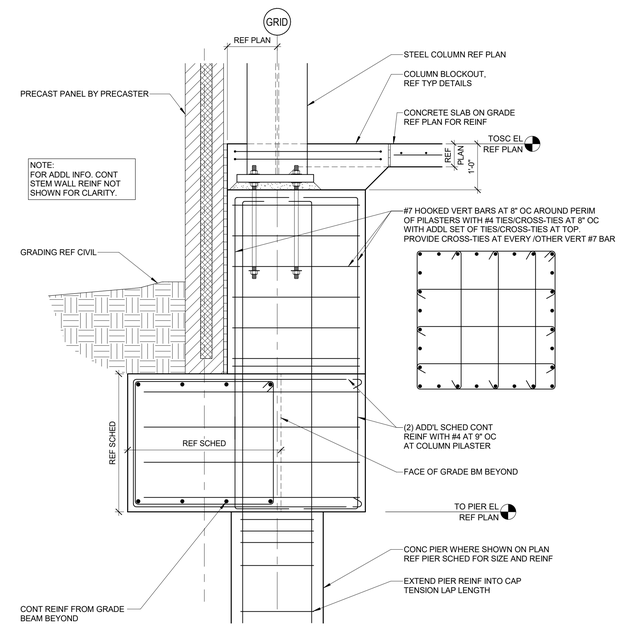
10 EXTERIOR DETAIL AT ROOF DRAIN
3/4" = 1'-0"



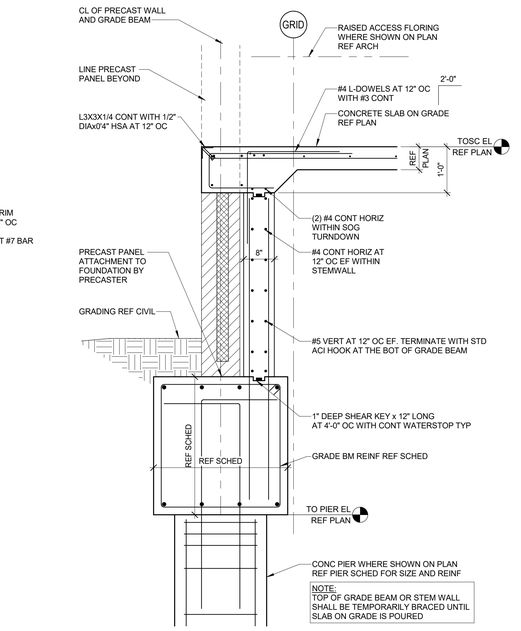
9 OVERHEAD DOOR SUPPORT AT FLOOR
3/4" = 1'-0"



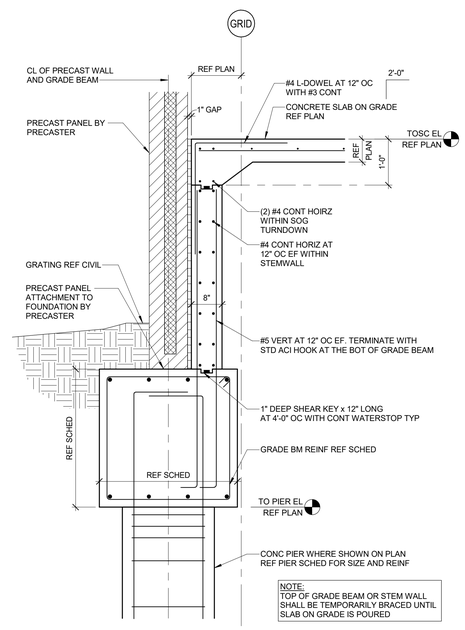
8 TYPICAL INTERIOR DETAIL AT COLUMN
3/4" = 1'-0"



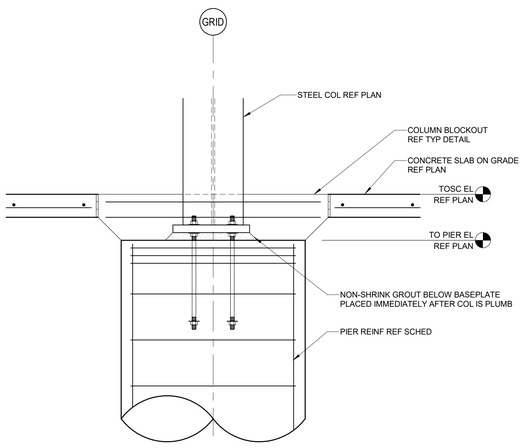
7 TYPICAL RECESSED EXTERIOR DETAIL COLUMN
3/4" = 1'-0"



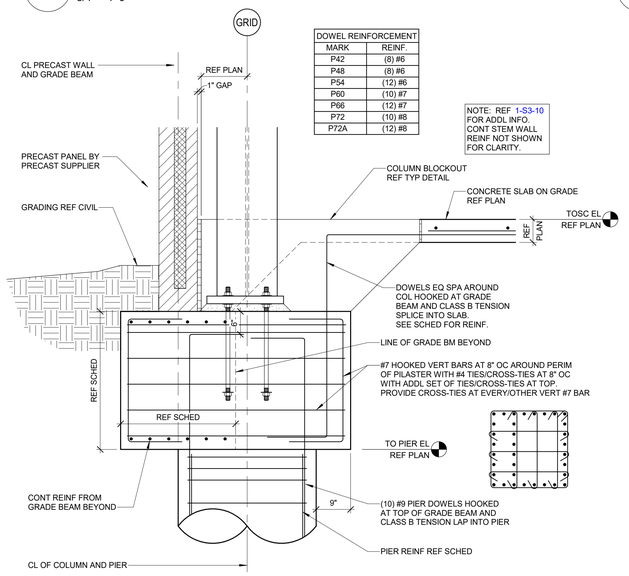
6 TYPICAL RECESSED EXTERIOR DETAIL AT PIER
3/4" = 1'-0"



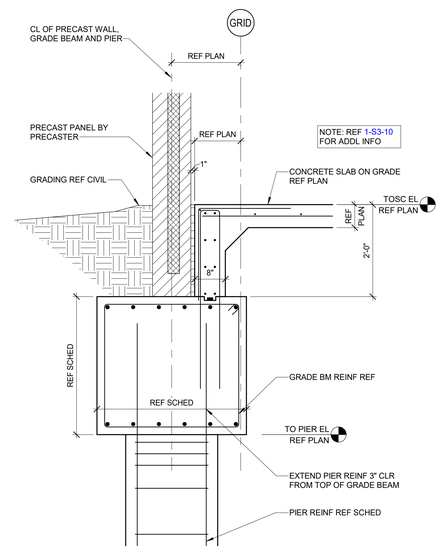
5 TYPICAL RECESSED EXTERIOR DETAIL
3/4" = 1'-0"



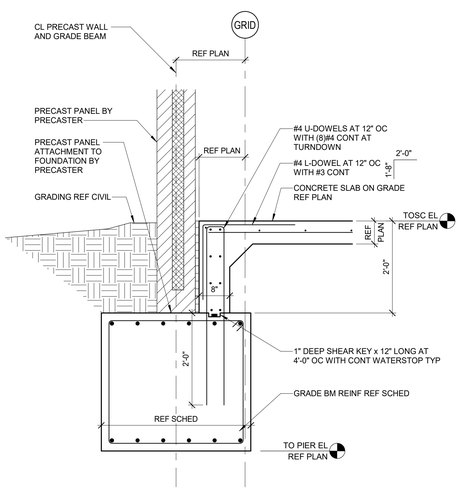
4 TYPICAL INTERIOR DETAIL AT COLUMN
3/4" = 1'-0"



3 TYPICAL EXTERIOR DETAIL AT COLUMN
3/4" = 1'-0"



2 TYPICAL EXTERIOR DETAIL AT PIER
3/4" = 1'-0"



1 TYPICAL EXTERIOR DETAIL
3/4" = 1'-0"

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	REVISIONS	
1	03.28.25	ADDENDUM 1

COLORADO LICENSED
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PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

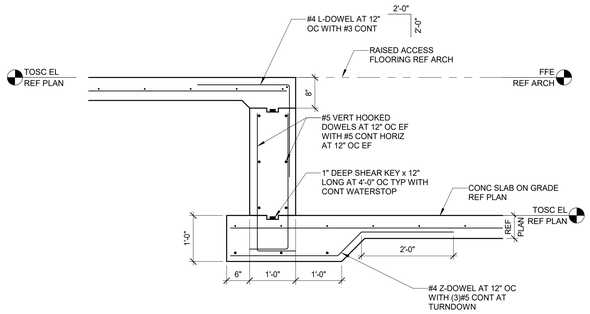
QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

FOUNDATION DETAILS

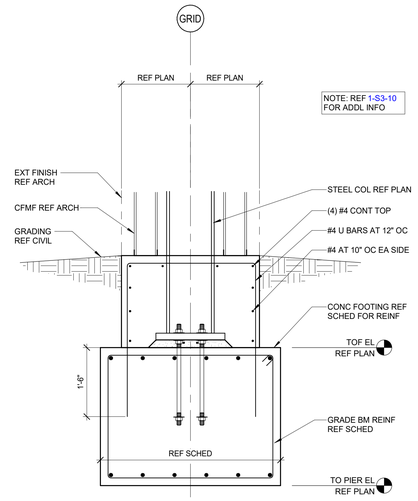
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S3-10

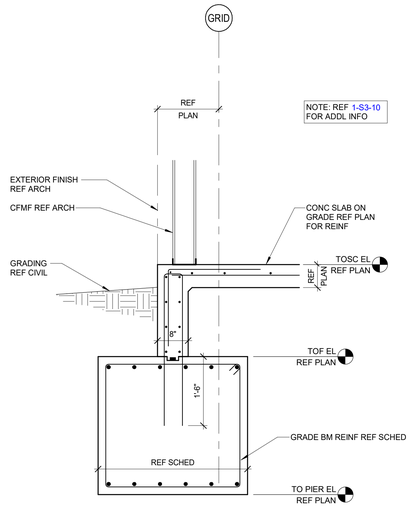
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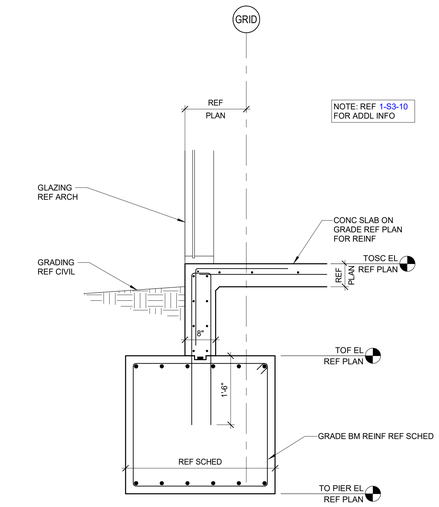
7 DETAIL12
3/4" = 1'-0"



6 SOG AT EXTERIOR
3/4" = 1'-0"



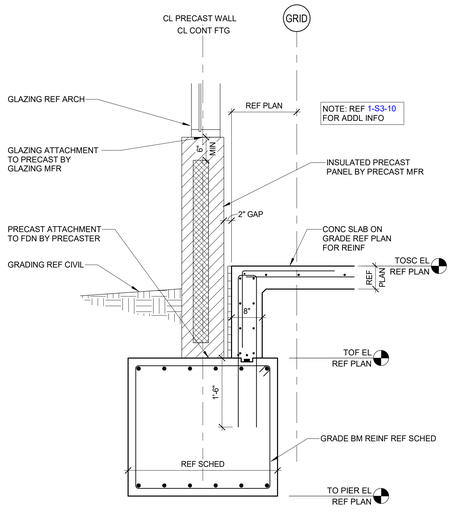
5 SOG AT EXTERIOR
3/4" = 1'-0"



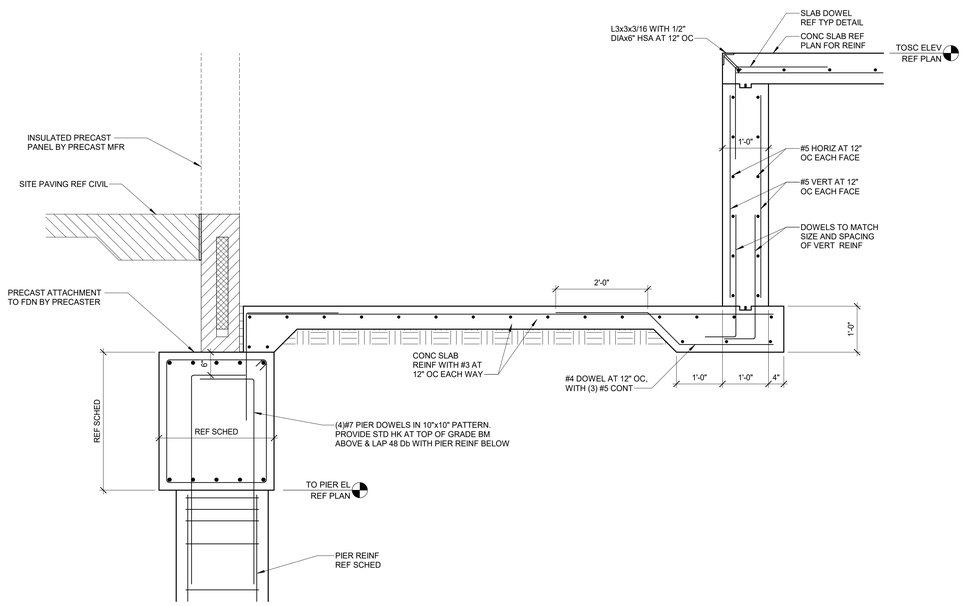
4 SOG AT EXTERIOR
3/4" = 1'-0"

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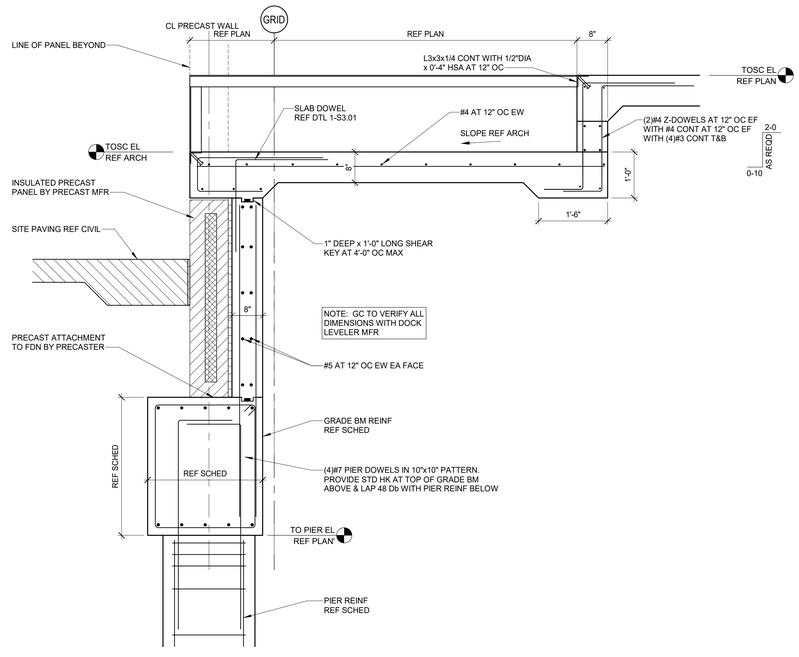
City of Aurora Building Division
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3 SOG AT PERIMETER OF BUILDING
3/4" = 1'-0"



2 DETAIL AT DOCK LIFT
3/4" = 1'-0"



1 DETAIL AT DOCK LEVELER
3/4" = 1'-0"

9/10/2025 5:07:57 PM S3-11 FOUNDATION DETAILS

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DESIGN CONSULTING LLC

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COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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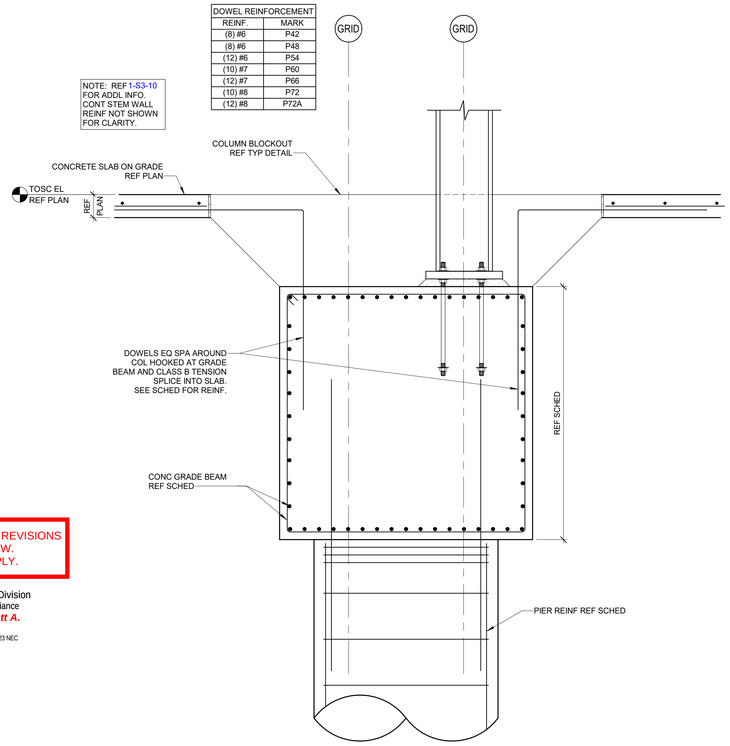
QTS DEN1 DC2
1140 N. GUN CLUB ROAD

FOUNDATION DETAILS

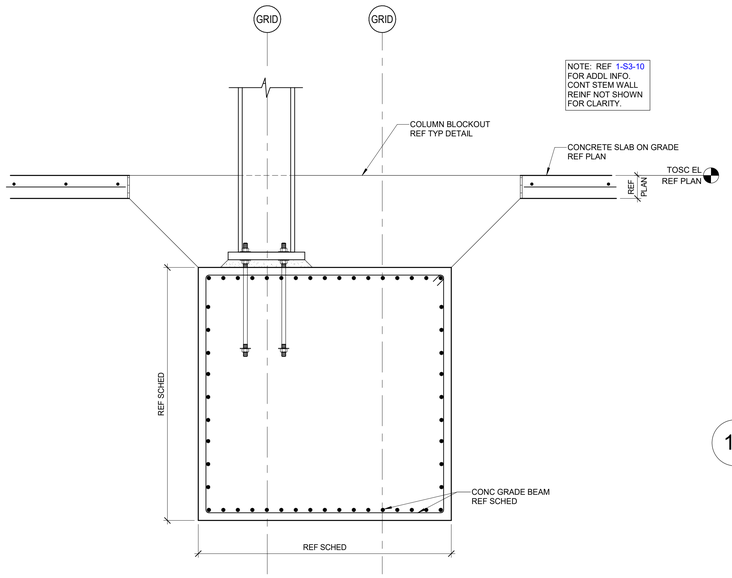
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DATE 09.10.2025
SHEET

S3-11

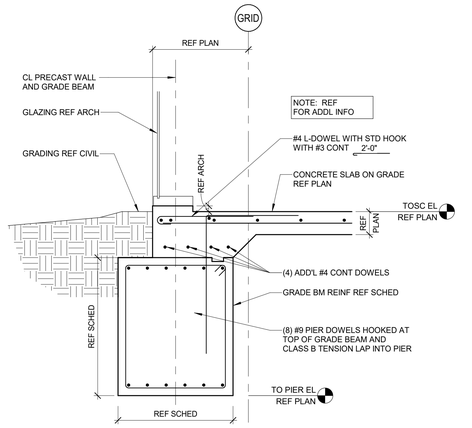
DOWEL REINFORCEMENT	
REINF.	MARK
(8) #6	P42
(8) #6	P48
(12) #6	P54
(10) #7	P60
(12) #7	P66
(10) #8	P72
(12) #8	P72A



12 DETAIL AT COLUMN/PIER
3/4" = 1'-0"



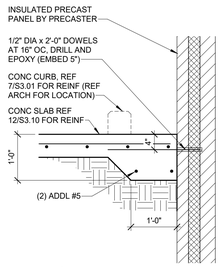
11 DETAIL AT COLUMN
3/4" = 1'-0"



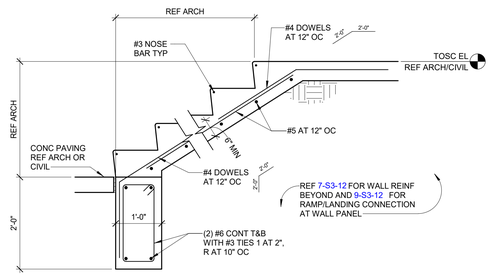
10 DETAIL
3/4" = 1'-0"

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ALL PREVIOUS COMMENTS APPLY.

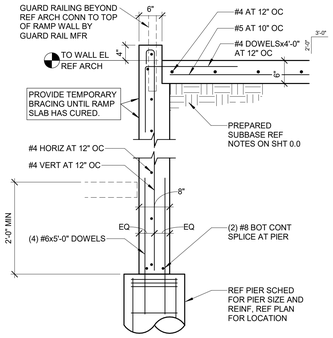
City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



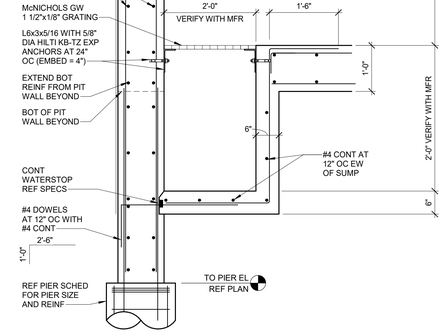
9 RAMP/LANDING AT PANEL
3/4" = 1'-0"



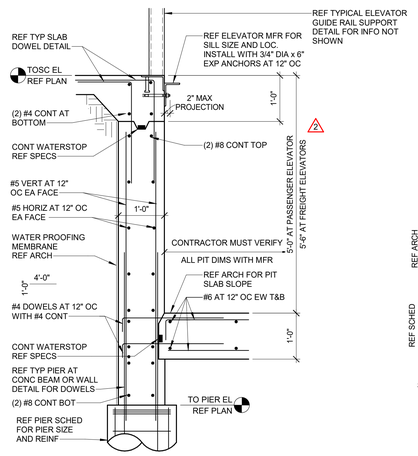
8 CONCRETE STAIR DETAIL
3/4" = 1'-0"



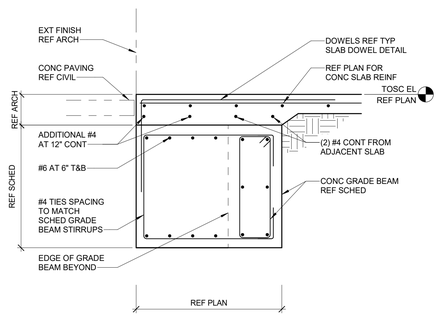
7 RAMP/LANDING AT PIER
3/4" = 1'-0"



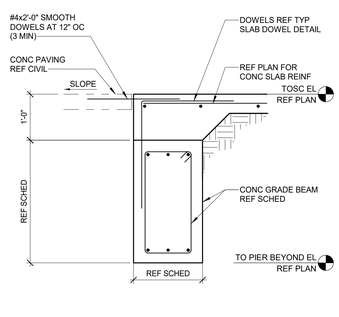
6 ELEVATOR SUMP PIT
3/4" = 1'-0"



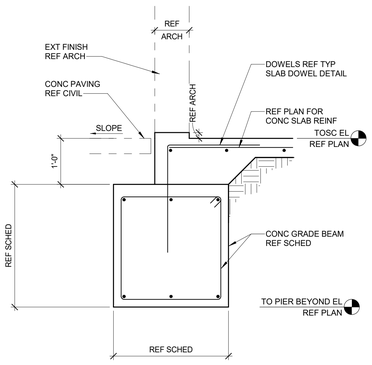
5 ELEVATOR PIT
3/4" = 1'-0"



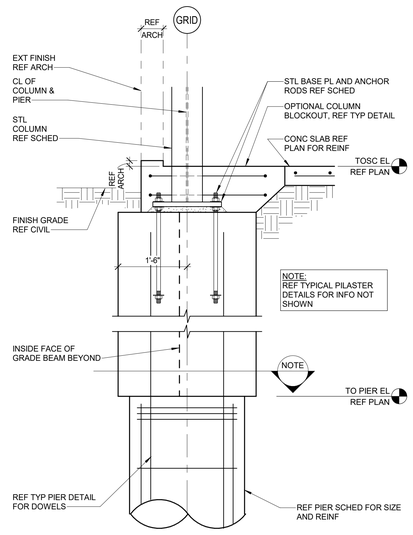
4 WIDENED GRADE BEAM
3/4" = 1'-0"



3 GRADE BEAM AT DOOR OPENING
3/4" = 1'-0"



2 EXTERIOR AT GRADE BEAM
3/4" = 1'-0"



1 COLUMN AT PILASTER
3/4" = 1'-0"

CORGAN
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AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS

1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED NICHOLAS W. KEVID 65236 PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

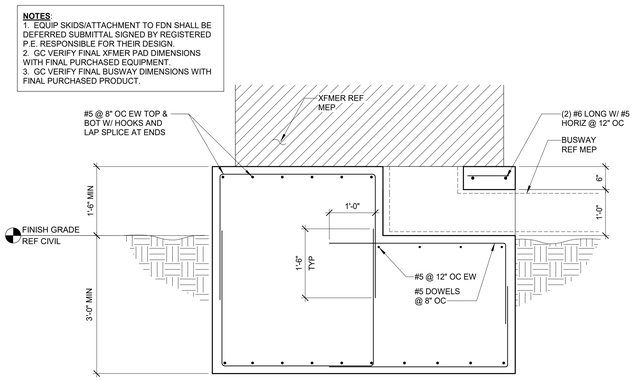
QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

FOUNDATION DETAILS

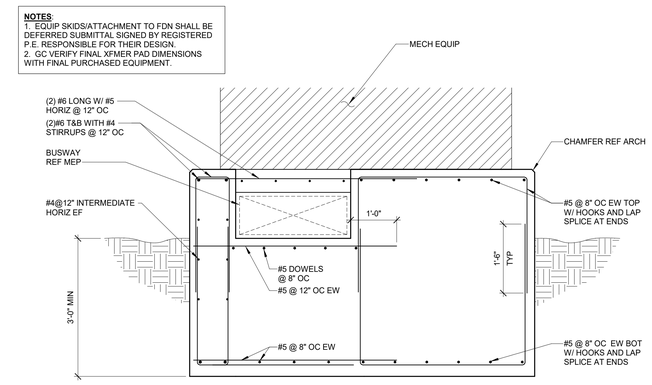
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S3-12

9/10/2025 5:07:58 PM S3-12 FOUNDATION DETAILS



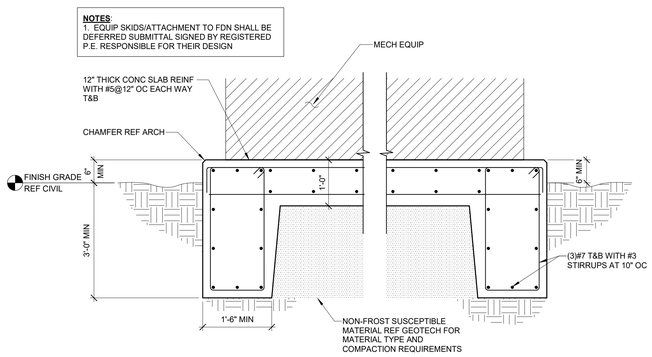
7 TRANSFORMER PAD DETAIL
3/4" = 1'-0"



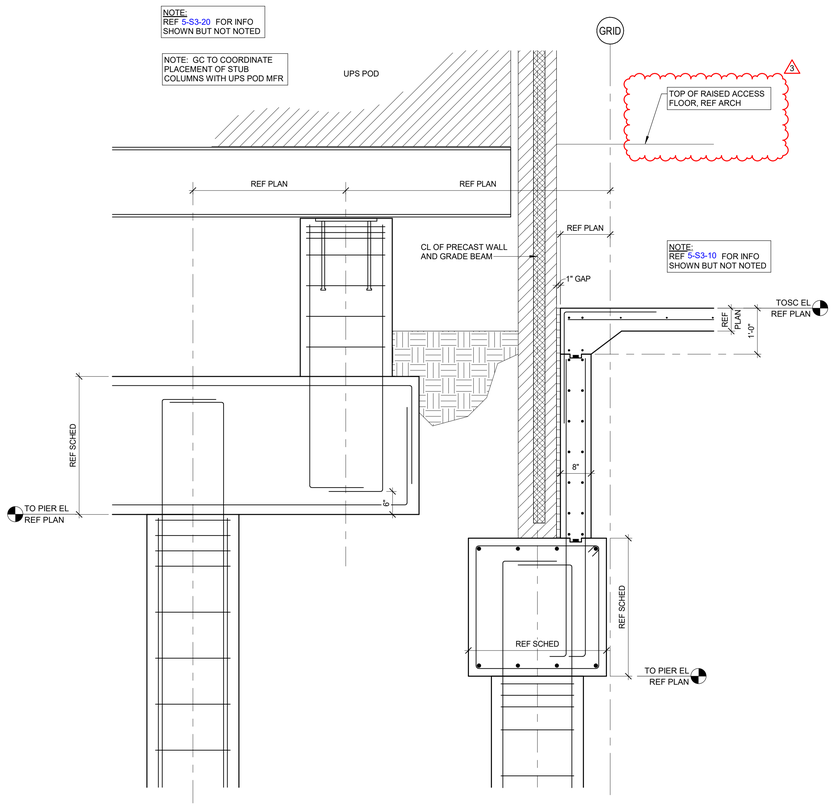
4 TRANSFORMER PAD DETAIL
3/4" = 1'-0"

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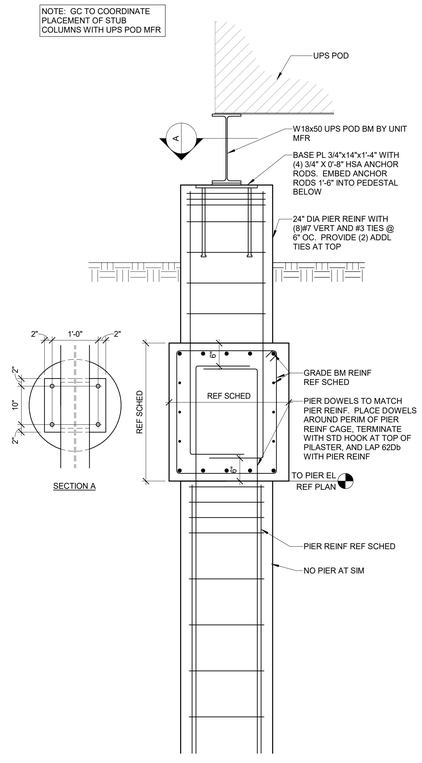
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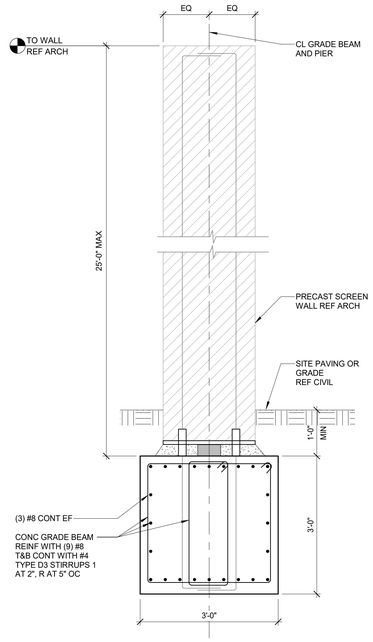
3 GENERATOR PAD DETAIL
3/4" = 1'-0"



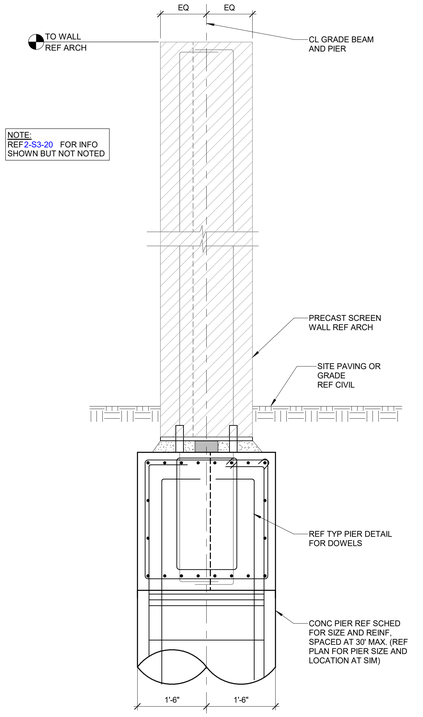
6 UPS POD PILASTER AT GRADE RECESSED EXTERIOR
3/4" = 1'-0"



5 UPS POD PILASTER AT GRADE
3/4" = 1'-0"



2 PRECAST SCREENWALL AT GRADE BEAM
3/4" = 1'-0"



1 PRECAST SCREENWALL AT PIER
3/4" = 1'-0"

9/10/2025 5:07:59 PM S3-20 FOUNDATION DETAILS

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DESIGN CONSULTING LLC

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10		

	REVISIONS	
3	09.10.25	IFC.1

COLORADO LICENSED
NICHOLAS W. HEYD
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

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1140 N. GUN CLUB ROAD
QTS

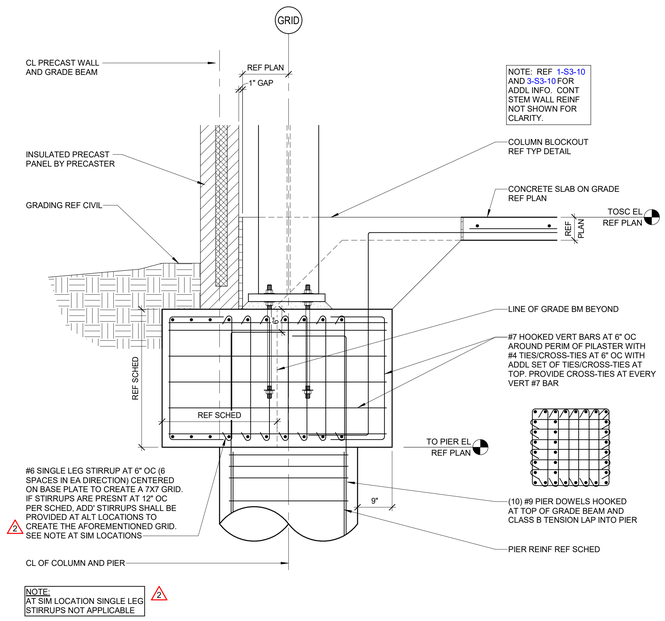
FOUNDATION DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

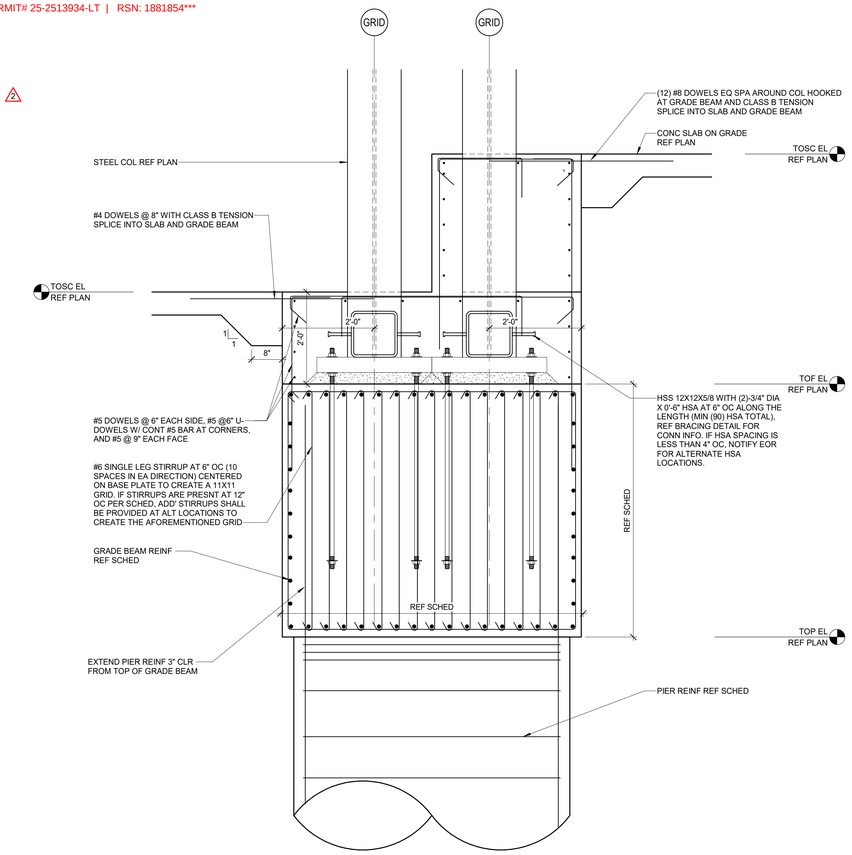
S3-20

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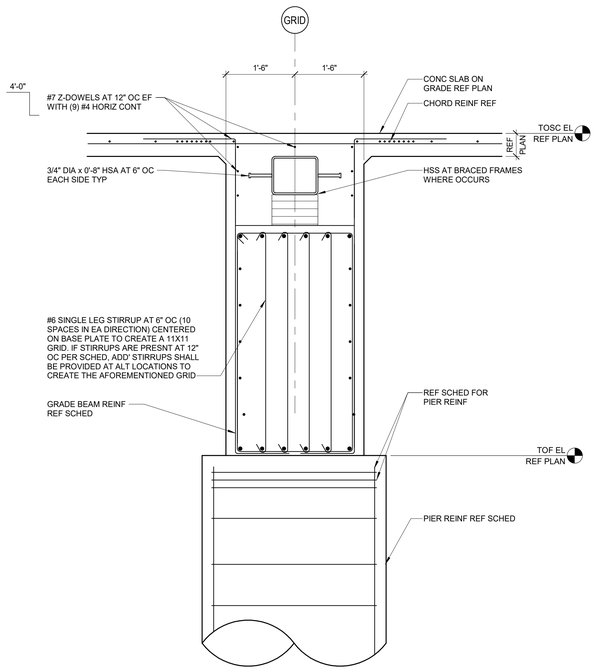


5 EXTERIOR DETAIL AT BRACE FRAME COL ALONG GRID F
3/4" = 1'-0"

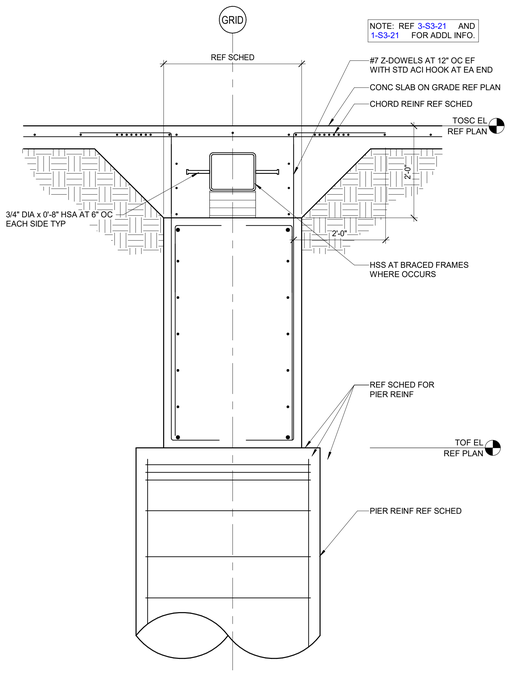


4 TYPICAL INTERIOR DETAIL AT COLUMN
3/4" = 1'-0"

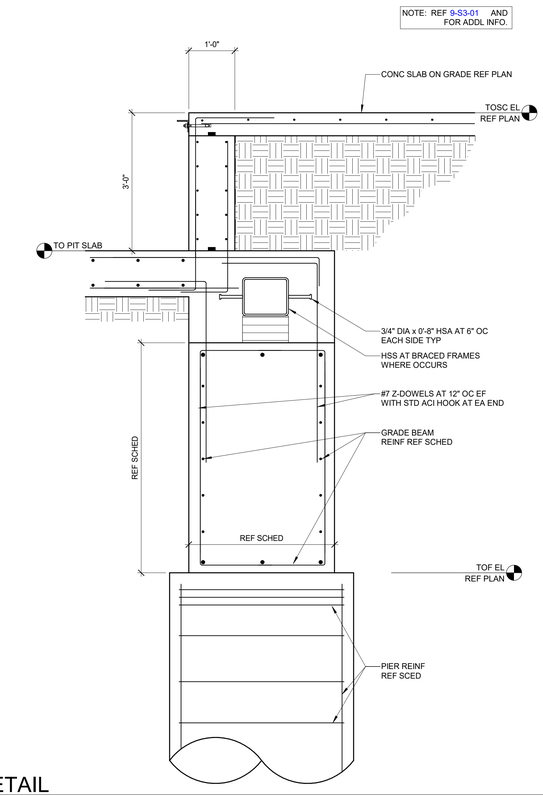
2



3 DETAIL
3/4" = 1'-0"



2 DETAIL
3/4" = 1'-0"



1 DETAIL
3/4" = 1'-0"

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QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

KEYPLAN

S	Q	N	L	J	G	E	C	A	
T	R	P	M	K	H	F	D	B	

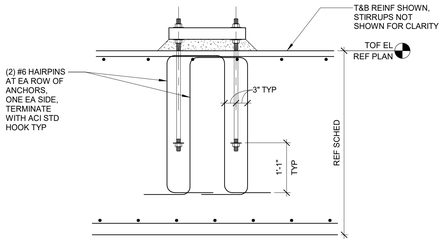
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FOUNDATION DETAILS

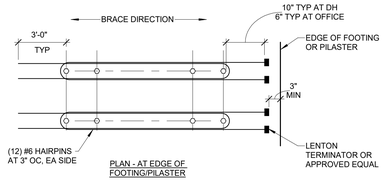
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DATE 09.10.2025
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S3-21

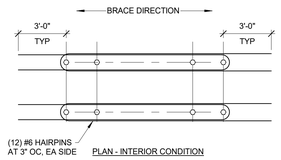
9/10/2025 5:07:59 PM S3-21 FOUNDATION DETAILS



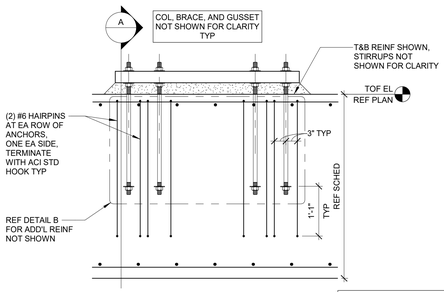
DETAIL A

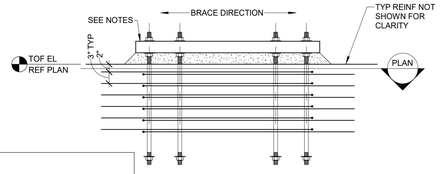


PLAN - AT EDGE OF FOOTING/PILASTER



PLAN - INTERIOR CONDITION

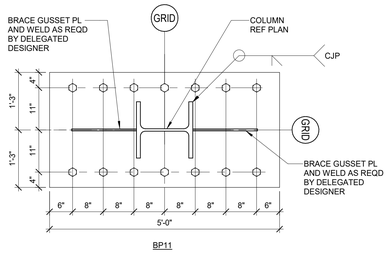




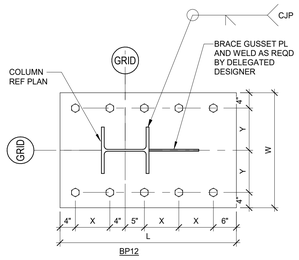
DETAIL B

NOTE:
1. PROVIDE:
•OVERSIZE HOLES IN BASE PLATE
•STANDARD HOLES IN WASHERS
2. PLATE WASHERS MUST BE WELDED TO THE BASE PLATE.
3. SEE "ANCHOR ROD DETAIL" ON 1-S5-00 FOR WELD SIZE.

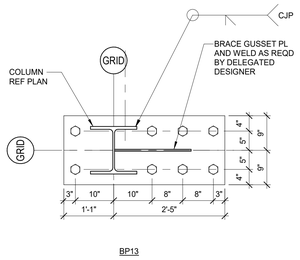
5 ANCHOR ROD REINFORCING
3/4" = 1'-0"



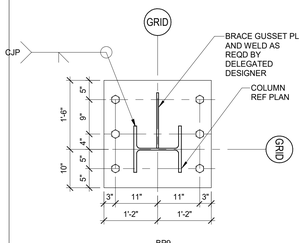
BP11



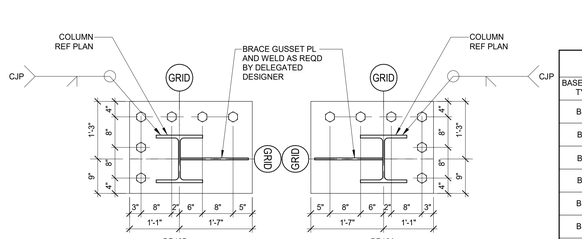
BP12



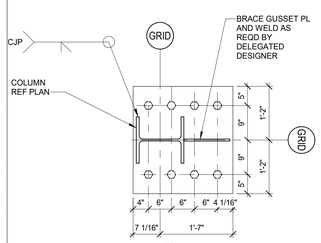
BP13



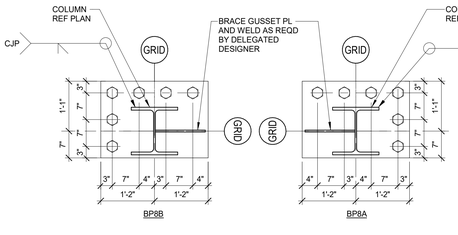
BP9



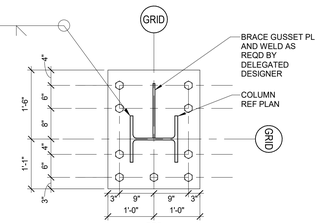
BP10B BP10A



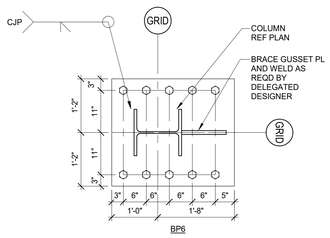
BP7



BP8B BP8A



BP5



BP6

2 COLUMN BASE PLATE AND ANCHOR ROD DETAILS
3/4" = 1'-0"

BASE PLATE AND ANCHOR ROD SCHEDULE AND DETAILS

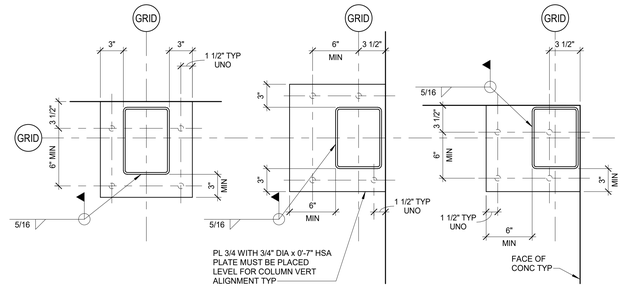
BASE PLATE TYPE	SIZE	(L)	(W)	(T)	(X)	(Y)	BASE PLATE TYPE	ANCHORS RODS	ANCHOR ROD GRADE	COMMENTS
BP-1	W12X106/120 (PERIMETER) W14X90/99/109 (PERIMETER)	18"	18"	1 1/4"	3"	4"	BP3	(4) 3/4" DIA x 1'-6"	GR 36	-
BP-2	W12X79/136/152 (PERIMETER)	18"	18"	1 1/2"	3"	4"	BP3	(4) 3/4" DIA x 1'-6"	GR 36	-
BP-3	W14X132/145 (PERIMETER)	18"	18"	1 3/4"	3"	4"	BP3	(4) 3/4" DIA x 1'-6"	GR 36	-
BP-4	W12X72/79/106 (INTERIOR) W14X90/99 (INTERIOR)	24"	24"	1 1/2"	9"	9"	BP2	(4) 3/4" DIA x 1'-6"	GR 36	-
BP-5	W12X136/152 (INTERIOR) W14X132 (INTERIOR)	24"	24"	1 3/4"	9"	9"	BP2	(4) 3/4" DIA x 1'-6"	GR 36	-
BP-6	W14X145/159/176/193 (INTERIOR)	24"	24"	2 1/4"	9"	9"	BP2	(4) 3/4" DIA x 1'-6"	GR 36	-
BP-7	W14X211/233 (INTERIOR)	28"	28"	2 1/2"	10"	10"	BP2	(4) 3/4" x 1'-6"	GR 36	-
BP-8	W14X257/311 (INTERIOR)	36"	36"	3"	12"	12"	BP2	(4) 3/4" x 1'-6"	GR 36	-
BP-9	W12X79 (ELEVATOR)	14"	18"	1 1/4"	3"	4"	BP3	(4) 3/4" x 1'-6"	GR 36	-
BP-10	W14X90 (ELEVATOR)	16"	18"	1 1/4"	3 1/2"	4"	BP3	(4) 3/4" x 1'-6"	GR 36	-
BP-11	W14X233 (INTERIOR)	34"	32"	4"	13"	8"	BP4	(8) 2" x 2'-6"	GR 105	REF NOTE 5 & 5/S5.00
BP-12	HSS6X6X1/2	12"	12"	1"	4"	4"	BP1	(4) 3/4" x 1'-6"	GR 36	-
BP-13	HSS10X10X1/2	18"	18"	1"	7"	7"	BP1	(4) 3/4" x 1'-6"	GR 36	-
BF-1	WF12(BF) WF14(BF)	REF 2-S5-00		2 1/2"	REF 2-S5-00		BP5	(9) 1" x 2'-0"	GR 105	REF 5/S5.00
BF-2	WF12(BF) WF14(BF)	REF 2-S5-00		2 1/4"	REF 2-S5-00		BP6	(10) 1" x 2'-0"	GR 105	REF 5/S5.00
BF-3	WF12(BF)	REF 2-S5-00		2 1/4"	REF 2-S5-00		BP7	(8) 1" x 2'-0"	GR 105	-
BF-4	WF12(BF)	REF 2-S5-00		3"	REF 2-S5-00		BP8A&B	(6) 1 3/4" x 2'-0"	GR 105	REF NOTE 5 & 5/S3-21
BF-5	WF14(BF)	REF 2-S5-00		2 3/4"	REF 2-S5-00		BP9	(6) 1" x 2'-0"	GR 105	-
BF-6	WF14(BF)	REF 2-S5-00		3"	REF 2-S5-00		BP10A&B	(6) 1 1/2" x 2'-0"	GR 105	REF NOTE 5 & 5/S3-21
BF-7	WF14(BF)	REF 2-S5-00		3"	REF 2-S5-00		BP11	(14) 1" x 2'-0"	GR 105	REF 5/S5.00
BF-8	WF14(BF)	46"	32"	4"	9"	12"	BP12	(10) 2 1/2" x 4'-0"	GR 105	REF NOTE 5 & 4/S3-21
BF-9	WF12(BF)	REF 2-S5-00		2 1/2"	REF 2-S5-00		BP13	(8) 1 1/2" x 2'-0"	GR 105	REF NOTE 5 & 5/S5.00
BF-10	WF14(BF)	46"	30"	4"	9"	11"	BP12	(10) 2 1/2" x 4'-0"	GR 105	REF NOTE 5 & 4/S3-21

NOTES:
1. BASE PLATE SIZES ARE BASED ON COLUMN SIZES UNLESS DENOTED BY A SPECIFIC "BPX" IN THE COLUMN SCHEDULE OR ON THE PLAN.
2. ALL BASE PLATES SHALL BE ASTM A572 GR 50, UNLESS NOTED OTHERWISE.
3. ALL ANCHOR RODS SHALL BE ASTM F1554 GRADE AS NOTED.
4. SIZE WELDS PER AISC MINIMUM FILLET REQUIREMENTS, 5/16" MINIMUM.
5. ADDITIONAL WASHERS PER SCHEDULE TO BE PLACED AT EMBEDED END OF ANCHOR RODS.
6. BF = BRACE FRAME

1 COLUMN BASE PLATE AND ANCHOR ROD SCHEDULE AND DETAILS
1 1/2" = 1'-0"

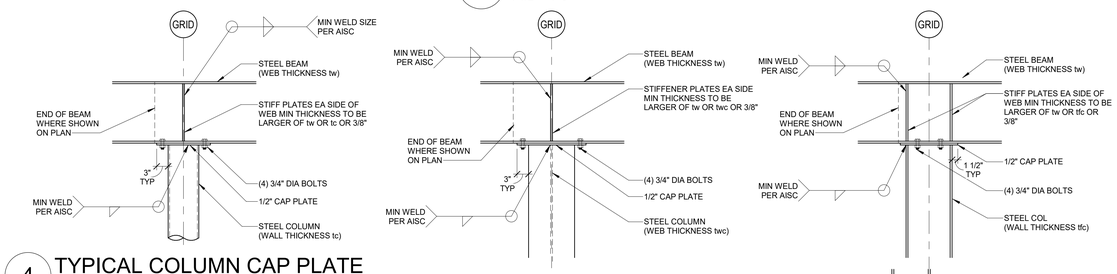
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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: ***Nov 06, 2025***
2021 INTERNATIONAL CODES & 2023 NEC

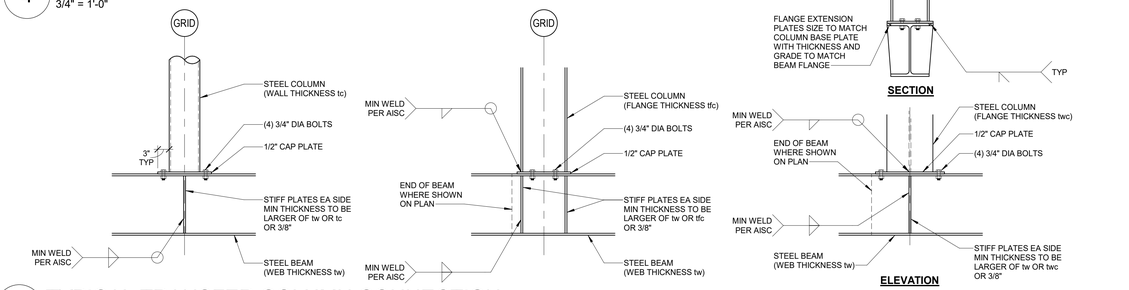


PLAN A PLAN B PLAN C

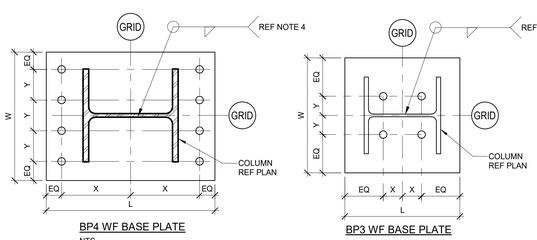
6 TYPICAL COLUMN BASE PLATE AT EDGE OF CONCRETE
1 1/2" = 1'-0"



4 TYPICAL COLUMN CAP PLATE
3/4" = 1'-0"

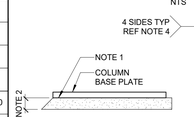


3 TYPICAL TRANSFER COLUMN CONNECTION
3/4" = 1'-0"



BP4 WF BASE PLATE
NTS

BP3 WF BASE PLATE
NTS



NOTES:
1. GROUT MAY BE BEVELED OR FORMED BUT SHALL EXTEND A MIN OF 1 1/2" BEYOND FACE OF BASE PLATE.
2. GROUT THICKNESS SHALL BE 2" FOR COLUMN WITH UP TO 1 1/4" DIA ANCHOR RODS. FOR COLUMNS WITH ANCHOR RODS GREATER THAN 1 1/4" DIA, USE 3" GROUT THICKNESS.

GROUT PLACEMENT
NTS

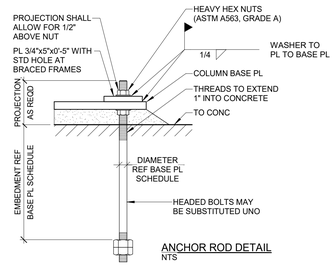
BP1 HSS BASE PLATE
NTS

BP2 WF BASE PLATE
NTS

ANCHOR ROD HOLE & WASHER SCHEDULE

ANCHOR ROD DIAMETER	MAX HOLE DIAMETER	MIN WASHER SIZE	MIN WASHER THICKNESS
3/4"	1 1/4"	2"	1/4"
7/8"	1 1/2"	2 1/2"	5/16"
1"	1 3/4"	3"	3/8"
1 1/4"	2"	3"	1/2"
1 1/2"	2 1/4"	3 1/2"	1/2"
1 3/4"	2 1/2"	4"	5/8"
2"	3 1/4"	5"	3/4"
2 1/2"	3 3/4"	5 1/2"	7/8"

NOTE(S):
1. CIRCULAR OR SQUARE WASHERS MEETING THE SIZE SHOWN ARE ACCEPTABLE.



ANCHOR ROD DETAIL
NTS

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ISSUES

1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

REVISIONS

1	03.28.25	ADDENDUM 1
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED NICHOLAS W. REVID 65236 PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

TYPICAL STEEL COLUMN DETAILS AND BASE PLATE SCHEDULE

JOB 24464.0000
DATE 09.10.2025
SHEET

S5-00

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COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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QTS DEN1 DC2
1140 N. GUN CLUB ROAD

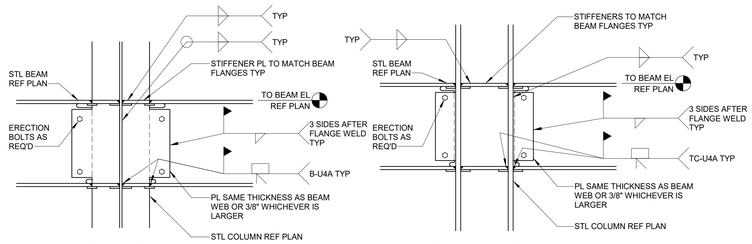
QTS

3 TYPICAL BEAM SLOPE CHANGE
3/4" = 1'-0"

HSS COLUMN

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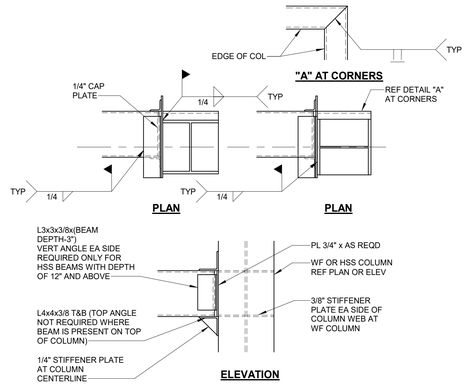
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WEAK AXIS - WF COLUMN

STRONG AXIS - WF COLUMN

2 TYPICAL BEAM TO COLUMN MOMENT CONNECTION
3/4" = 1'-0"



ELEVATION

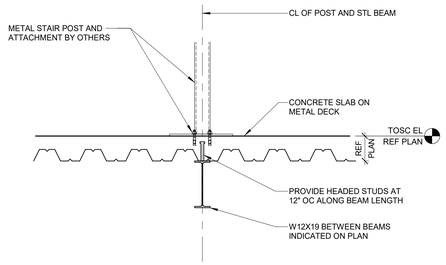
1 TYPICAL HSS BEAM TO COL CONN
3/4" = 1'-0"

TYPICAL CONNECTION DETAILS

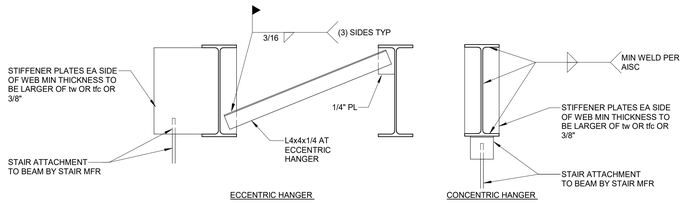
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S5-01

9/10/2025 5:08:02 PM S5-01 TYPICAL CONNECTION DETAILS



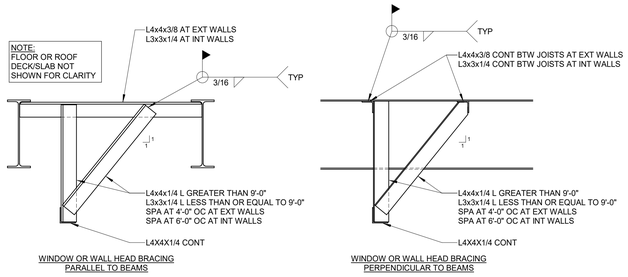
4 TYPICAL STAIR POST SUPPORT
3/4" = 1'-0"



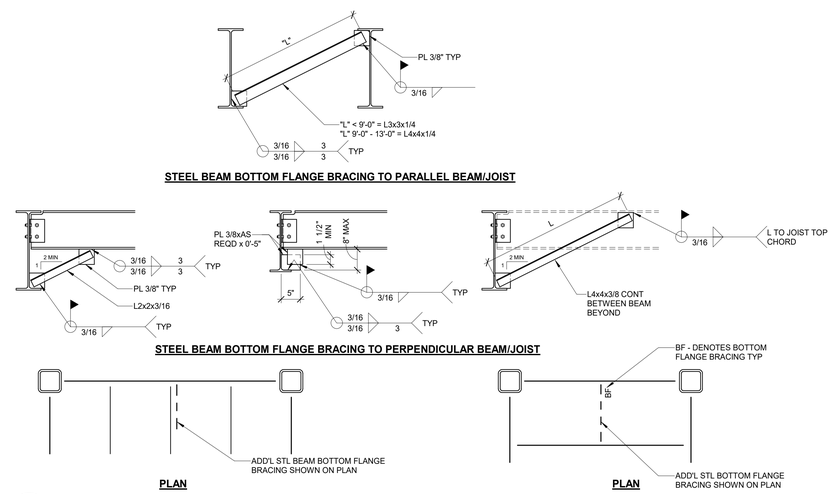
3 TYPICAL STAIR HANGER ATTACHMENT TO BEAMS
3/4" = 1'-0"

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2 TYPICAL WINDOW OR WALL HEAD BRACING AT BEAMS
3/4" = 1'-0"



1 TYPICAL STEEL BEAM BOTTOM FLANGE BRACING
3/4" = 1'-0"

9/10/2025 5:08:03 PM S5-02 TYPICAL STEEL FRAMING SECTIONS AND DETAILS

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NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

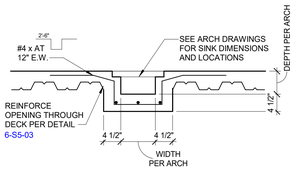
QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

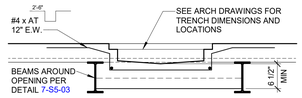
TYPICAL STEEL FRAMING SECTIONS AND DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

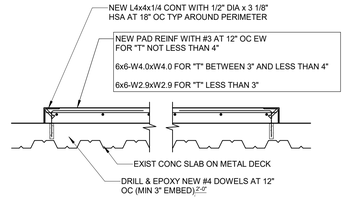
S5-02



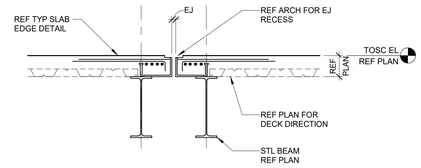
14 TYPICAL DETAIL TRENCH DRAIN AT COMPOSITE FLOOR
3/4" = 1'-0"



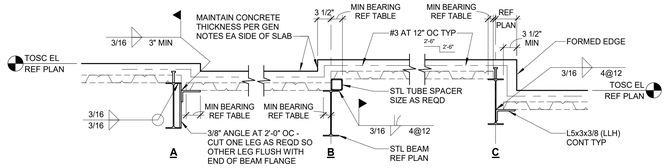
13 TYPICAL DETAIL TRENCH DRAIN AT COMPOSITE FLOOR
3/4" = 1'-0"



12 TYPICAL HOUSEKEPING PAD AT CONC SLAB ON METAL DECK
3/4" = 1'-0"



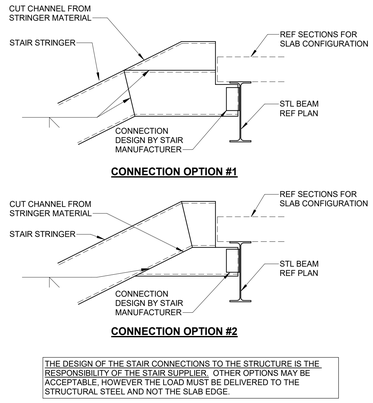
11 TYPICAL SLAB EXPANSION JOINT
3/4" = 1'-0"



NORMAL WEIGHT CONCRETE				
MINIMUM BEARING LENGTH REQUIRED				
	SLAB DEPTH ABOVE FLUTES			
DECK TYPE	2 1/2	3	3 1/2	4 1/2
2 VLI 18	1 1/2	1 1/2	1 1/2	2

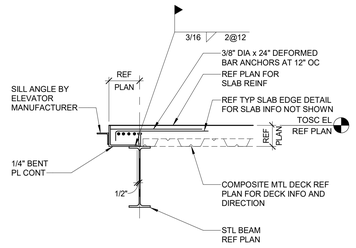
NOTE:
MINIMUM BEARING FOR ALL DECK TYPES AND SLAB THICKNESSES IS 1 1/2" UNLESS LISTED IN THE TABLE ABOVE.

10 TYPICAL FLOOR DECK SUPPORTS AT SLAB OFFSETS
3/4" = 1'-0"

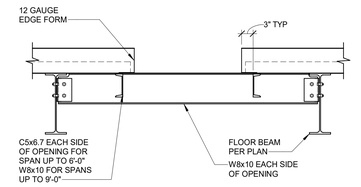


THE DESIGN OF THE STAIR CONNECTIONS TO THE STRUCTURE IS THE RESPONSIBILITY OF THE STAIR SUPPLIER. OTHER OPTIONS MAY BE ACCEPTABLE, HOWEVER THE LOAD MUST BE DELIVERED TO THE STRUCTURAL STEEL AND NOT THE SLAB EDGE.

9 TYPICAL CONNECTION OPTIONS FOR STAIR STRINGER TO STRUCTURAL STEEL
3/4" = 1'-0"



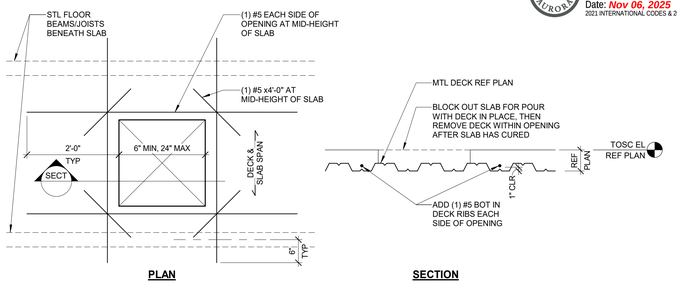
8 TYPICAL SLAB EDGE AT ELEVATOR SILL, STAIR STRINGER AND HANDRAILS
3/4" = 1'-0"



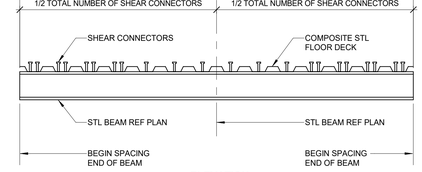
7 TYPICAL SLAB OPENING WIDTH OVER 2'-0" PERPENDICULAR TO DECK SPAN
3/4" = 1'-0"

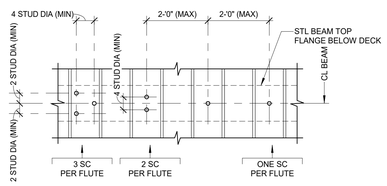
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6 TYPICAL SLAB OPENING WIDTH FROM 6" WIDE TO 24" WIDE
3/4" = 1'-0"

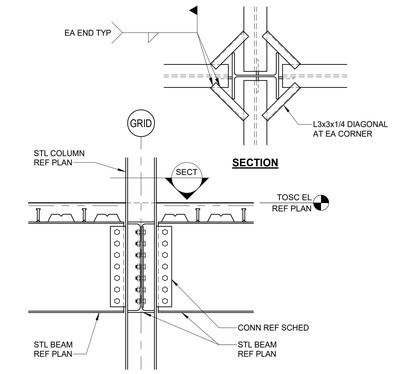




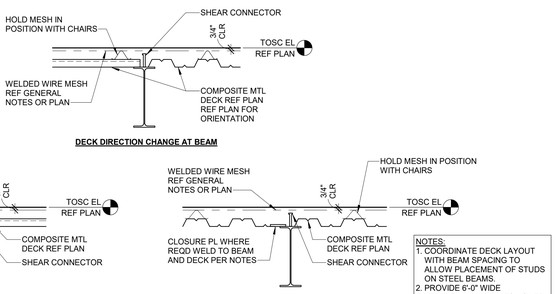
NOTES FOR TYPICAL SHEAR CONNECTOR PLACING DETAILS

1. REF GENERAL NOTES FOR SHEAR CONNECTOR SIZE AND TYPE
2. SPACE ONE-HALF OF TOTAL NUMBER OF SHEAR CONNECTORS EVENLY BETWEEN END OF BEAM AND MIDSPAN, UNLESS NOTED OTHERWISE.
3. WHERE DECK FLUTES ARE PARALLEL TO BEAM SPAN, MINIMUM SPACING SHALL BE 6 STUD DIAMETERS ALONG LONGITUDINAL AXIS OF BEAM AND 4 STUD DIAMETERS TRANSVERSE TO LONGITUDINAL AXIS OF BEAM.
4. WHERE DECK FLUTES ARE PERPENDICULAR TO BEAM SPAN, PLACE CONNECTORS AS FOLLOWS:
   A. IF THERE ARE MORE SHEAR CONNECTORS THAN DECK FLUTES, BEGIN PLACING CONNECTORS AT END SPANS OF BEAM AND PLACE ONE CONNECTOR IN EACH DECK FLUTE UNTIL REACHING MIDSPAN. PLACE REMAINING CONNECTORS (IN EXCESS OF ONE) TWO PER FLUTE, STARTING AGAIN AT ENDS OF BEAM UNTIL TOTAL NUMBER OF SHEAR CONNECTORS HAVE BEEN PLACED. FOR EXAMPLE, IF THERE ARE 30 CONNECTORS AND 24 FLUTES, PLACE TWO CONNECTORS PER FLUTE IN THREE FLUTES AT EACH END AND ONE PER FLUTE IN THE MIDDLE 18 FLUTES.
   B. IF THERE ARE MORE DECK FLUTES THAN CONNECTORS, BEGIN PLACING AT END OF BEAM AND PLACE ONE CONNECTOR IN EVERY OTHER FLUTE UNTIL REACHING MIDSPAN. PLACE REMAINING CONNECTORS IN EMPTY FLUTES, BEGINNING AT ENDS OF BEAM, UNTIL NUMBER OF CONNECTORS HAVE BEEN PLACED.
   C. WHERE TWO OR MORE CONNECTORS OCCUPY THE SAME FLUTE, PLACE CONNECTORS AS INDICATED IN PLAN VIEW.
5. DETERMINE WHERE CONNECTORS ARE TO BE LOCATED PRIOR TO ATTACHMENT.
6. REMOVE CERAMIC FERRULES FROM CONNECTORS AND DECK BEFORE PLACING CONCRETE.
7. MAXIMUM SPACING OF SHEAR CONNECTORS ON BEAMS (OTHER THAN CANTILEVER BEAMS AND BEAMS WITHOUT CONCRETE ON TOP) SHALL BE 24" ON CENTER. PROVIDE ADDITIONAL SHEAR CONNECTORS AS REQUIRED TO COMPLY WITH THE MAXIMUM SPACING WHERE SHEAR CONNECTORS HAVE NOT BEEN SPECIFIED ON PLAN OR THE NUMBER OF SHEAR CONNECTORS SPECIFIED ON PLAN IS LESS THAN REQUIRED TO MEET THE MAXIMUM SPACING.

5 TYPICAL SHEAR CONNECTOR PLACEMENT DETAIL
3/4" = 1'-0"

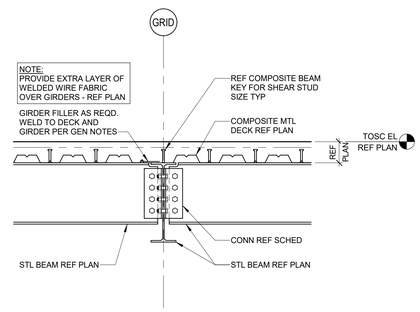


4 TYPICAL COLUMN DETAIL
3/4" = 1'-0"

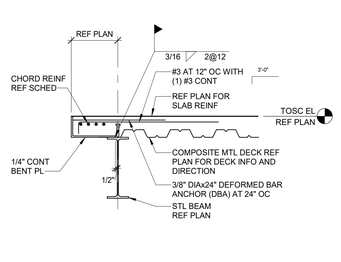


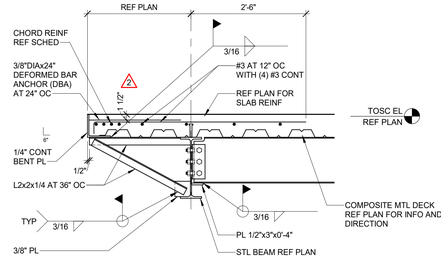
NOTES:
1. COORDINATE DECK LAYOUT WITH BEAM SPACING TO ALLOW PLACEMENT OF STUDS ON STEEL BEAMS.
2. PROVIDE 6'-0" WIDE ADDITIONAL WIRE MESH OVER ALL GIRDERS NOT SHOWN REF PLAN.

3 TYPICAL DECK AT BEAM
3/4" = 1'-0"



2 TYPICAL BEAM TO BEAM CONNECTION
3/4" = 1'-0"





1 TYPICAL SLAB EDGE DETAIL
3/4" = 1'-0"

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10		

REVISIONS		
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engeunity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

TYPICAL STEEL FRAMING SECTIONS AND DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

S5-03

9/10/2025 5:08:04 PM S5-03 TYPICAL STEEL FRAMING SECTIONS AND DETAILS

CORGAN

401 N. Houston Street Dallas, TX
75202
T: 214.748.2000

AG/E

SYSKA HENNESSY GROUP

TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	

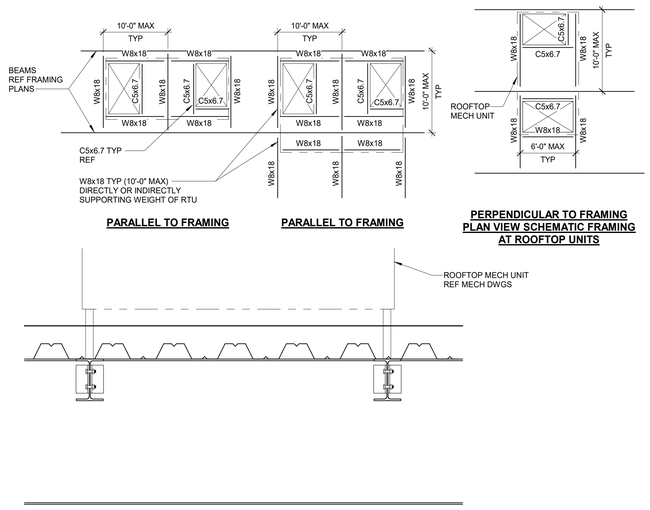
COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

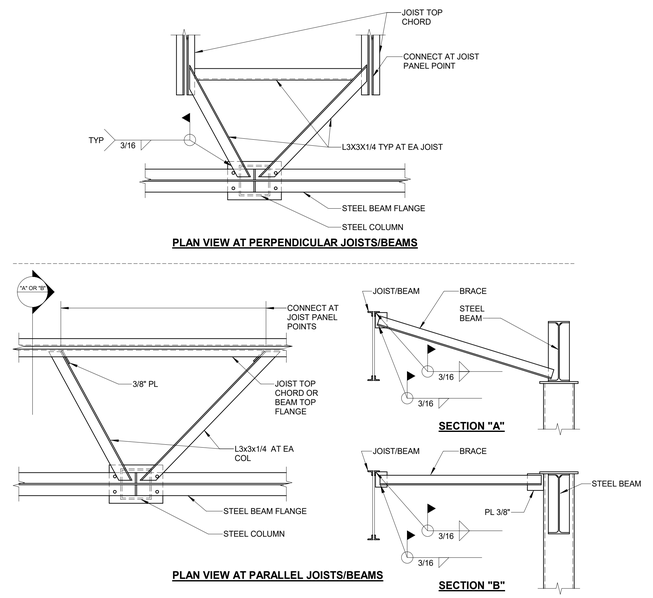
ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW.
ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



PARALLEL TO FRAMING **PARALLEL TO FRAMING** **PERPENDICULAR TO FRAMING**
PLAN VIEW SCHEMATIC FRAMING AT ROOFTOP UNITS

2 TYPICAL FRAMING AT ROOFTOP UNITS
1" = 1'-0"



PLAN VIEW AT PERPENDICULAR JOISTS/BEAMS
PLAN VIEW AT PARALLEL JOISTS/BEAMS
SECTION "A"
SECTION "B"

1 TYPICAL COLUMN BRACE AT JOISTS/BEAMS
3/4" = 1'-0"

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

TYPICAL STEEL FRAMING SECTIONS AND DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

S5-04

9/10/2025 5:08:05 PM S5-04 TYPICAL STEEL FRAMING SECTIONS AND DETAILS

TABLE 1 - SSP1 - 3/4" DIA GR A325N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E.

SINGLE PLATE SHEAR CONNECTION — VER 1.1 — 12/22/2023

NUMBER OF BOLTS	2	3	4	5	6	7	8	9	10	11	12
PLATE THICKNESS (in)	1/4	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16
MIN PLATE DEPTH (in)	6	9	12	15	18	21	24	27	30	33	36
FILLET WELD SIZE (in)	3/16	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4

| | BEAM SIZE | MAX DEPTH OF TOP COPE (in) | MAX REACTION V/P (kips) | MAX COPE LENGTH (in) (TOP COPE ONLY) | MAX COPE LENGTH (in) (TOP & BOT COPE

TABLE 1 - DABB1 - 3/4" DIA GR A325N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E.

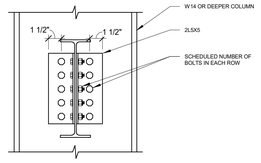
VER 1.1

DOUBLE ANGLE SHEAR CONNECTION (BOLTED-BOLTED)

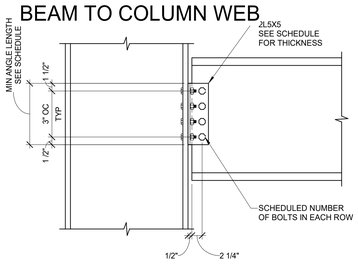
12/22/2023

NUMBER OF BOLTS PER ROW	2	3	4	5	6	7	8	9	10	11	12
ANGLE THICKNESS (in)	5/16	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
MIN ANGLE LENGTH (in)	6	9	12	15	18	21	24	27	30	33	36

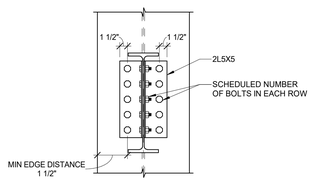
	BEAM SIZE	MAX DEPTH OF TOP COPE (in)	2: MAX REACTION V (kips)	2: MAX COPE LENGTH (in) (TOP COPE ONLY)	2: MAX COPE LENGTH (in) (TOP & BOT COPE)	3: MAX REACTION V (kips)	3: MAX COPE LENGTH (in) (TOP COPE ONLY)	3: MAX COPE LENGTH (in) (TOP & BOT COPE)	4: MAX REACTION V (kips)	4: MAX COPE LENGTH (in) (TOP COPE ONLY)	4: MAX COPE LENGTH (in) (TOP & BOT COPE)	5: MAX REACTION V (kips)	5: MAX COPE LENGTH (in) (TOP COPE ONLY)	5: MAX COPE LENGTH (in) (TOP & BOT COPE)	6: MAX REACTION V (kips)	6: MAX COPE LENGTH (in) (TOP COPE ONLY)	6: MAX COPE LENGTH (in) (TOP & BOT COPE)	7: MAX REACTION V (kips)	7: MAX COPE LENGTH (in) (TOP COPE ONLY)	7: MAX COPE LENGTH (in) (TOP & BOT COPE)	8: MAX REACTION V (kips)	8: MAX COPE LENGTH (in) (TOP COPE ONLY)	8: MAX COPE LENGTH (in) (TOP & BOT COPE)	9: MAX REACTION V (kips)	9: MAX COPE LENGTH (in) (TOP COPE ONLY)	9: MAX COPE LENGTH (in) (TOP & BOT COPE)	10: MAX REACTION V (kips)	10: MAX COPE LENGTH (in) (TOP COPE ONLY)	10: MAX COPE LENGTH (in) (TOP & BOT COPE)	11: MAX REACTION V (kips)	11: MAX COPE LENGTH (in) (TOP COPE ONLY)	11: MAX COPE LENGTH (in) (TOP & BOT COPE)	12: MAX REACTION V (kips)	12: MAX COPE LENGTH (in) (TOP COPE ONLY)	12: MAX COPE LENGTH (in) (TOP & BOT COPE)
39	W44X230 TO W44X335	3																250	15	13 1/2	286	15	15	322	15	15	357	15	15	393	15	15	429	15	15
38	W40X503 TO W40X655	4 7/8																250	15	15	286	15	15	322	15	15	357	15	15	393	--	--	K		
37	W40X324 TO W40X431	3 5/8																250	15	15	286	15	15	322	15	15	357	15	15	393	--	--			
36	W40X199 TO W40X297	3																250	15	12	286	15	13 1/2	322	15	15	357	15	15	393	--	--			
35	W40X264 TO W40X392	3 7/8																250	15	15	286	15	15	322	15	15	357	15	15	393	--	--			
34	W40X149 TO W40X235	3																250	15	11 1/4	286	15	12 3/4	322	15	14 1/4	353	15	15	393	--	--			
33	W36X487 TO W36X652	4 7/8													214	15	15	250	15	15	286	15	15	322	15	15	357	--	--	J					
32	W36X302 TO W36X441	3 3/4													214	15	15	250	15	15	286	15	15	322	15	15	357	15	15						
31	W36X231 TO W36X282	2 7/8													214	15	13	250	15	15	286	15	15	322	15	15	357	15	15						
30	W36X232 TO W36X256	3													214	15	15	250	15	15	286	15	15	322	15	15	357	15	15						
29	W36X135 TO W36X210	2 3/8													208	15	9 1/2	243	15	11	283	15	12	311	15	14	338	15	15						
28	W33X201 TO W33X318	3													214	15	12	250	15	13 3/4	286	15	15	322	15	15	I								
27	W33X118 TO W33X169	2 7/8													194	15	9 1/2	223	15	11	260	15	12	286	15	14									
26	W30X173 TO W30X261	3										178	15	8 3/4	214	15	10 1/4	250	15	12	286	15	13	H										
25	W30X90 TO W30X148	2 5/8										139	15	7 3/4	164	15	9 1/2	190	15	11	222	15	12											
24	W27X146 TO W27X194	3										178	15	7 3/4	211	15	9 1/2	245	15	11	G												
23	W27X84 TO W27X129	2 7/8										136	15	7 3/4	159	15	9 1/2	185	15	11													
22	W24X104 TO W24X176	3							122	15	6	148	15	7 3/4	174	15	9 1/2	F															
21	W24X68 TO W24X103	2 3/4							101	15	6	122	15	7 3/4	144	14	9 1/2																
20	W24X55 TO W24X62	3							96	15	6	117	14	7 3/4	137	12 1/2	9 1/2																
19	W21X101 TO W21X147	2 1/2							123	15	6	149	15	7 3/4	E																		
18	W21X48 TO W21X93	2							85	13 1/2	6	103	12	7 3/4																		
17	W21X44 TO W21X57	2							86	13 1/4	6	104	11 3/4	7 3/4																		
16	W18X76 TO W18X130	2							103	15	6	178	--	--																		
15	W18X50 TO W18X71	2							87	12 3/4	6	155	--	--																		
14	W18X35 TO W18X46	2							73	10 1/4	6	131	--	--																		
13	W16X67 TO W16X100	1 3/4							96	13 1/2	6	D																				
12	W16X36 TO W16X57	1 3/4				56	11 3/4	4 1/2	72	9 3/4	6																					
11	W16X26 TO W16X31	1 5/8				46	9 1/2	4 1/2	61	7 3/4	6																					
10	W14X61 TO W14X82	2				71	12 3/4	4 1/2	C																							
9	W14X43 TO W14X53	2				57	10 1/2	4 1/2																								
8	W14X30 TO W14X38	2				50	9 1/2	4 1/2																								
7	W14X22 TO W14X26	2				44	7 1/2	4 1/4																								
6	W12X40 TO W12X58	--				77	--	--																								
5	W12X26 TO W12X35	1 1/8				44	7 3/4	4 1/4																								
4	W12X14 TO W12X22	1	26	7 3/4	3	38	6	4 1/2																								
3	W10X33 TO W10X45	1 5/8	38	9	3	B																										
2	W10X22 TO W10X30	2	28	8 1/2	3																											
1	W10X12 TO W10X19	2	25	5 3/4	3																											
			A																													



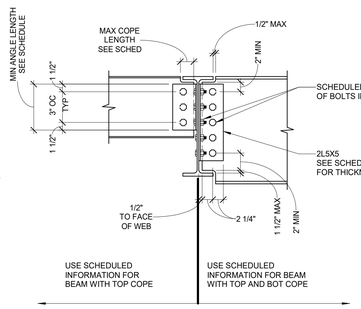
BEAM TO COLUMN WEB



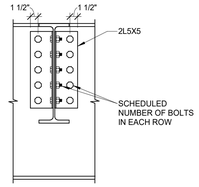
BEAM TO COLUMN FLANGE



BEAM TO COLUMN FLANGE



BEAM TO BEAM



BEAM TO BEAM

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

TABLE 2 - DABB2 - 7/8" DIA GR A325N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E.

VER 1.1

DOUBLE ANGLE SHEAR CONNECTION (BOLTED-BOLTED)

12/22/2023

NUMBER OF BOLTS PER ROW	2	3	4	5	6	7	8	9	10	11	12
ANGLE THICKNESS (in)	3/8	1/2	1/2	1/2	1/2	1/2	1/2	1/2	1/2	1/2	1/2
MIN ANGLE LENGTH (in)	6	9	12	15	18	21	24	27	30	33	36

	BEAM SIZE	MAX DEPTH OF TOP COPE (in)	2: MAX REACTION V (kips)	2: MAX COPE LENGTH (in) (TOP COPE ONLY)	2: MAX COPE LENGTH (in) (TOP & BOT COPE)	3: MAX REACTION V (kips)	3: MAX COPE LENGTH (in) (TOP COPE ONLY)	3: MAX COPE LENGTH (in) (TOP & BOT COPE)	4: MAX REACTION V (kips)	4: MAX COPE LENGTH (in) (TOP COPE ONLY)	4: MAX COPE LENGTH (in) (TOP & BOT COPE)	5: MAX REACTION V (kips)	5: MAX COPE LENGTH (in) (TOP COPE ONLY)	5: MAX COPE LENGTH (in) (TOP & BOT COPE)	6: MAX REACTION V (kips)	6: MAX COPE LENGTH (in) (TOP COPE ONLY)	6: MAX COPE LENGTH (in) (TOP & BOT COPE)	7: MAX REACTION V (kips)	7: MAX COPE LENGTH (in) (TOP COPE ONLY)	7: MAX COPE LENGTH (in) (TOP & BOT COPE)	8: MAX REACTION V (kips)	8: MAX COPE LENGTH (in) (TOP COPE ONLY)	8: MAX COPE LENGTH (in) (TOP & BOT COPE)	9: MAX REACTION V (kips)	9: MAX COPE LENGTH (in) (TOP COPE ONLY)	9: MAX COPE LENGTH (in) (TOP & BOT COPE)	10: MAX REACTION V (kips)	10: MAX COPE LENGTH (in) (TOP COPE ONLY)	10: MAX COPE LENGTH (in) (TOP & BOT COPE)	11: MAX REACTION V (kips)	11: MAX COPE LENGTH (in) (TOP COPE ONLY)	11: MAX COPE LENGTH (in) (TOP & BOT COPE)	12: MAX REACTION V (kips)	12: MAX COPE LENGTH (in) (TOP COPE ONLY)	12: MAX COPE LENGTH (in) (TOP & BOT COPE)
39	W44X230 TO W44X335	3																			274	15	12	314	15	13 1/2	350	15	15	387	15	15	424	15	15	460	15	15
38	W40X503 TO W40X655	4 7/8																			340	15	15	389	15	15	438	15	15	487	15	15	535	--	--	K		
37	W40X324 TO W40X431	3 5/8																			340	15	14 1/4	389	15	15	438	15	15	487	15	15	535	--	--			
36	W40X199 TO W40X297	3																			250	15	12	287	15	13 1/2	320	15	15	354	15	15	535	--	--			
35	W40X264 TO W40X392	3 7/8																			340	15	13 1/2	389	15	15	438	15	15	487	15	15	535	--	--			
34	W40X149 TO W40X235	3																			241	15	12	277	15	13 1/2	310	15	15	342	15	15	535	--	--			
33	W36X487 TO W36X652	4 7/8																292	15	15	340	15	15	389	15	15	438	15	15	487	--	--	J					
32	W36X302 TO W36X441	3 3/4																292	15	11 1/4	340	15	13	389	15	14 3/4	438	15	15	487	--	--						
31	W36X231 TO W36X282	2 7/8																257	15	10	292	15	12	335	15	13 1/2	375	15	15	414	15	15						
30	W36X232 TO W36X256	3																257	15	10	334	15	12	383	15	13 1/2	428	15	15	473	15	15						
29	W36X135 TO W36X210	2 3/8																202	15	10	230	15	12	264	15	13 1/2	295	15	15	326	15	15						
28	W33X201 TO W33X318	3																242	15	10	275	15	12	313	15	13 1/2	353	15	15	I								
27	W33X118 TO W33X169	2 7/8																186	15	10	212	15	12	243	15	13 1/2	271	15	15									
26	W30X173 TO W30X261	3																219	15	10	250	15	12	287	15	13 1/2	H											
25	W30X90 TO W30X148	2 5/8																159	15	10	181	15	12	207	15	13 1/2												
24	W27X146 TO W27X194	3																171	15	10	205	15	12	233	15	12	G											
23	W27X84 TO W27X129	2 7/8																129	15	8 1/2	155	15	10	176	15	12												
22	W24X104 TO W24X176	3										117	15	6 1/2	140	15	8 1/2	168	15	10	F															
21	W24X68 TO W24X103	2 3/4										97	15	6 1/2	116	15	8 1/2	139	14 1/4	10																
20	W24X55 TO W24X62	3										92	15	6 1/2	111	14 1/2	8 1/2	133	12 3/4	10																
19	W21X101 TO W21X147	2 1/2										118	15	6 1/2	141	15	8 1/2	E																		
18	W21X48 TO W21X93	2										82	14	6 1/2	98	12 1/2	8 1/2																			
17	W21X44 TO W21X57	2										82	13 3/4	6 1/2	98	12 1/4	8 1/2																			
16	W18X76 TO W18X130	2										99	15	6 1/2	217	--	--																			
15	W18X50 TO W18X71	2										83	13 1/4	6 1/2	181	--	--																			
14	W18X35 TO W18X46	2										70	10 1/2	6 1/2	153	--	--																			
13	W16X67 TO W16X100	1 3/4										92	14	6 1/2	D																					
12	W16X36 TO W16X57	1 3/4							52	12 1/4	5	69	10	6 1/2																						
11	W16X26 TO W16X31	1 5/8							44	9 3/4	5	58	8	6 1/2																						
10	W14X61 TO W14X82	2							67	13 1/2	5	C																								
9	W14X43 TO W14X53	2							54	11	5																									
8	W14X30 TO W14X38	2							47	9 3/4	5																									
7	W14X22 TO W14X26	2							40	8	5																									
6	W12X40 TO W12X58	--							93	--	--																									
5	W12X26 TO W12X35	1 1/8							40	8 1/4	5																									
4	W12X14 TO W12X22	1	26	7 3/4	3				35	6 1/2	5																									
3	W10X33 TO W10X45	1 5/8	38	9	3	B																														
2	W10X22 TO W10X30	2	31	7 3/4	3																															
1	W10X12 TO W10X19	2	25	5 3/4	3																															
			A																																

TABLE 3 - DABB3 - 1" DIA GR A490N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E.

VER 1.1

DOUBLE ANGLE SHEAR CONNECTION (BOLTED-BOLTED)

12/22/2023

NUMBER OF BOLTS PER ROW	2	3	4	5	6	7	8	9	10	11	12
ANGLE THICKNESS (in)	1/2	5/8	5/8	5/8	5/8	5/8	5/8	5/8	5/8	5/8	5/8
MIN ANGLE LENGTH (in)	6	9	12	15	18	21	24	27	30	33	36

	BEAM SIZE	MAX DEPTH OF TOP COPE (in)	2: MAX REACTION V (kips)	2: MAX COPE LENGTH (in) (TOP COPE ONLY)	2: MAX COPE LENGTH (in) (TOP & BOT COPE)	3: MAX REACTION V (kips)	3: MAX COPE LENGTH (in) (TOP COPE ONLY)	3: MAX COPE LENGTH (in) (TOP & BOT COPE)	4: MAX REACTION V (kips)	4: MAX COPE LENGTH (in) (TOP COPE ONLY)	4: MAX COPE LENGTH (in) (TOP & BOT COPE)	5: MAX REACTION V (kips)	5: MAX COPE LENGTH (in) (TOP COPE ONLY)	5: MAX COPE LENGTH (in) (TOP & BOT COPE)	6: MAX REACTION V (kips)	6: MAX COPE LENGTH (in) (TOP COPE ONLY)	6: MAX COPE LENGTH (in) (TOP & BOT COPE)	7: MAX REACTION V (kips)	7: MAX COPE LENGTH (in) (TOP COPE ONLY)	7: MAX COPE LENGTH (in) (TOP & BOT COPE)	8: MAX REACTION V (kips)	8: MAX COPE LENGTH (in) (TOP COPE ONLY)	8: MAX COPE LENGTH (in) (TOP & BOT COPE)	9: MAX REACTION V (kips)	9: MAX COPE LENGTH (in) (TOP COPE ONLY)	9: MAX COPE LENGTH (in) (TOP & BOT COPE)	10: MAX REACTION V (kips)	10: MAX COPE LENGTH (in) (TOP COPE ONLY)	10: MAX COPE LENGTH (in) (TOP & BOT COPE)	11: MAX REACTION V (kips)	11: MAX COPE LENGTH (in) (TOP COPE ONLY)	11: MAX COPE LENGTH (in) (TOP & BOT COPE)	12: MAX REACTION V (kips)	12: MAX COPE LENGTH (in) (TOP COPE ONLY)	12: MAX COPE LENGTH (in) (TOP & BOT COPE)
39	W44X230 TO W44X335	3																			275	15	13	293	15	15	326	15	15	359	15	15	392	15	15
38	W40X503 TO W40X655	4 7/8																			413	15	15	461	15	15	532	15	15	591	15	15	650	--	--
37	W40X324 TO W40X431	3 5/8																			365	15	13	412	15	15	459	15	15	506	15	15	650	--	--
36	W40X199 TO W40X297	3																			237	15	13	268	15	15	298	15	15	329	15	15	650	--	--
35	W40X264 TO W40X392	3 7/8																			350	15	13	395	15	15	440	15	15	485	15	15	650	--	--
34	W40X149 TO W40X235	3																			229	15	13	258	15	15	288	15	15	342	15	15	649	--	--
33	W36X487 TO W36X652	4 7/8																			413	15	15	473	15	15	532	15	15	591	--	--	J		
32	W36X302 TO W36X441	3 3/4																			299	15	11	343	15	13	387	15	15	431	15	15	591	--	--
31	W36X231 TO W36X282	2 7/8																			242	15	11	277	15	13	313	15	15	348	15	15	384	15	15
30	W36X232 TO W36X256	3																			276	15	11	317	15	13	358	15	15	398	15	15	439	15	15
29	W36X135 TO W36X210	2 3/8																			190	15	11	218	15	13	246	15	15	275	15	15	303	15	15
28	W33X201 TO W33X318	3																			228	15	11	261	15	13	295	15	15	328	15	15	I		
27	W33X118 TO W33X169	2 7/8																			175	15	11	201	15	13	229	15	15	252	15	15			
26	W30X173 TO W30X261	3																			206	15	11	237	15	13	268	15	15	H					
25	W30X90 TO W30X148	2 5/8																			149	15	11	171	15	13	193	15	15						
24	W27X146 TO W27X194	3																			193	15	11	221	15	13	G								
23	W27X84 TO W27X129	2 7/8																			145	15	11	167	15	13									
22	W24X104 TO W24X176	3																			159	15	11	F											
21	W24X68 TO W24X103	2 3/4																			131	15	11												
20	W24X55 TO W24X62	3																			125	13 1/4	11												
19	W21X101 TO W21X147	2 1/2																			E														
18	W21X48 TO W21X93	2																																	
17	W21X44 TO W21X57	2																																	
16	W18X76 TO W18X130	2																																	
15	W18X50 TO W18X71	2																																	
14	W18X35 TO W18X46	2																																	
13	W16X67 TO W16X100	1 3/4																																	
12	W16X36 TO W16X57	1 3/4													52	12 1/4	5	66	10 1/4	7 1/2															
11	W16X26 TO W16X31	1 5/8													44	9 3/4	5	55	8 1/2	7															
10	W14X61 TO W14X82	2													67	13 1/2	5	C																	
9	W14X43 TO W14X53	2													54	11	5																		
8	W14X30 TO W14X38	2													47	9 3/4	5																		
7	W14X22 TO W14X26	2													40	8	5																		
6	W12X40 TO W12X58	--													93	--	--																		
5	W12X26 TO W12X35	1 1/8													40	8 1/4	5																		
4	W12X14 TO W12X22	1	26	7 3/4	3										35	6 1/2	5																		
3	W10X33 TO W10X45	1 5/8	38	9	3	B																													
2	W10X22 TO W10X30	2	31	7 3/4	3																														
1	W10X12 TO W10X19	2	25	5 3/4	3																														
			A																															

DOUBLE ANGLE BOLTED-BOLTED CONNECTION

1/2" = 1'-0"

NOTES:

1. THESE TABLES ARE BASED ON THE AISC MANUAL 15th EDITION.
2. USE THESE TYPICAL CONNECTION DETAILS UNLESS A SPECIFIC CONNECTION DETAIL IS NOTED OTHERWISE.
3. USE THE APPROPRIATE SCHEDULE (TABLE 1, TABLE 2, OR TABLE 3) FOR THE SELECTED BOLT DIAMETER AND GRADE. CONTRACTOR TO COORDINATE BOLT DIAMETER AND GRADE BETWEEN STEEL FABRICATOR AND STEEL ERECTOR.
4. SEE PLAN FOR BEAM REACTIONS (V). THOSE ARE THE MINIMUM REQUIRED CAPACITIES. BEAM REACTIONS ARE ULTIMATE VALUES (LRFD)
5. "V" IN THE SCHEDULE DENOTES SHEAR AT THE END OF THE BEAM (KIPS)

   ENTER THE SCHEDULE FOR A GIVEN BEAM SIZE. THE SCHEDULE IDENTIFIES THE MAXIMUM SHEAR (V) THAT THE BEAM CAN RESIST. THE REQUIRED V MAY NOT EXCEED THE CORRESPONDING VALUE IN THE SCHEDULE.

   SCHEDULED VALUES OF V ARE ULTIMATE CAPACITIES (LRFD)
6. THE MINIMUM THICKNESS OF THE SUPPORTING BEAM WEB OR, COLUMN FLANGE TO WHICH THE LEG OF THE DOUBLE ANGLES ATTACHES IS AS FOLLOW

TABLE 1 - DABW1 - 3/4" DIA GR A325N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E.

VER 1.1

DOUBLE ANGLE SHEAR CONNECTION (BOLTED-WELDED)

12/22/2023

NUMBER OF BOLTS PER ROW	2	3	4	5	6	7	8	9	10	11	12
ANGLE THICKNESS (in)	5/16	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
MIN ANGLE LENGTH (in)	6	9	12	15	18	21	24	27	30	33	36
FILLET WELD SIZE (in)	1/4	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16

| | BEAM SIZE | MAX DEPTH OF TOP COPE (in) | MAX REACTION V (kips) | MAX COPE LENGTH (in) (TOP COPE ONLY) | MAX COPE LENGTH (in) (TOP & BOT COPE

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E.

TABLE 1 - DAWB1 - 3/4" DIA GR A325N BOLTS

VER 1.1

DOUBLE ANGLE SHEAR CONNECTION (WELDED-BOLTED) 12/22/2023

NUMBER OF BOLTS PER ROW	2			3			4			5			6			7			8			9			10			11			12		
ANGLE THICKNESS (in)	5/16			3/8			3/8			3/8			3/8			3/8			3/8			3/8			3/8			3/8			3/8		
MIN ANGLE LENGTH (in)	6			9			12			15			18			21			24			27			30			33			36		
FILLET WELD SIZE (in)	1/4			5/16			5/16			5/16			5/16			5/16			5/16			5/16			5/16			5/16			5/16		
BEAM SIZE	MAX DEPTH OF TOP COPE (in)	MAX REACTION V (kips)	MAX COPE LENGTH (in) (TOP COPE ONLY)	MAX COPE LENGTH (in) (TOP & BOT COPE

RSN: 1937949 | PERMIT#: 25-2569703-LT *** REVISION FOR PERMIT# 25-2513934-LT | RSN: 1881854***

TABLE 1 - ESP1 - 3/4" DIA GR A325N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E. VER 1.0 05/07/2024

EXTENDED SINGLE PLATE SHEAR CONNECTION

NUMBER OF BOLTS	2	3	4	5	6	7	8	9	10	11	12
EXTENDED PLATE THICKNESS (in)	1/4	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16	5/16
MIN PLATE DEPTH (in)	6	9	12	15	18	21	24	27	30	33	36
FILLET WELD SIZE (in)	3/16	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4

MAX REACTION (kips) @ MAX ECCENTRICITY (e)

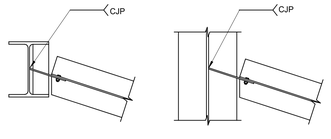
BEAM SIZE

V @ BM TO COL CONN | V @ BM TO BM CONN (e = 6.75") | V @ BM TO BM CONN (e = 7.75") | V @ BM TO BM CONN (e = 8.75") | V @ BM TO BM CONN (e = 10.75")

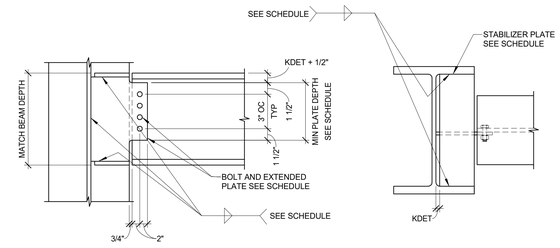
	2					3					4					5					6					7					8					9					10					11					12				
W44X230 TO W44X335																									N/A	64			N/A	81			N/A	99			N/A	117			N/A	137			N/A	N/A		
W40X503 TO W40X655																					109	N/A	64		128	N/A	81		147	N/A	99	165	N/A	117	180	N/A	120	195	N/A	139								
W40X324 TO W40X431																									N/A	64			N/A	81			N/A	99			N/A	117			N/A	137						
W40X199 TO W40X297																					109	N/A	64		128	N/A	81		147	N/A	99	165	N/A	117	180	N/A	120												
W40X264 TO W40X392																									N/A	64			N/A	81			N/A	99			N/A	117			N/A	137						
W40X149 TO W40X235																					109	N/A	64		128	N/A	81		147	N/A	99	165	N/A	117	180	N/A	120												
W36X487 TO W36X652															N/A	48					N/A	64			N/A	81			N/A	99			N/A	117	AB	AC	AD												
W36X302 TO W36X441															N/A	48					91	52	40	109	68	53	128	86	68	147	N/A	84	165	N/A	100														
W36X231 TO W36X282															N/A	48					91	52	40	109	68	53	128	86	68	147	N/A	84	165	N/A	100														
W36X232 TO W36X256															N/A	48					91	52	40	109	68	53	128	86	68	147	N/A	84	165	N/A	100														
W36X135 TO W36X210															N/A	48					91	52	40	109	68	53	128	86	68	147	N/A	84	165	N/A	100														
W33X201 TO W33X318											N/A	34			N/A	48					91	52	40	109	68	53	128	86	68	147	N/A	84	165	N/A	100	Y	Z	AA											
W33X118 TO W33X169									72	37	28	91	52	40	N/A	64			N/A	81																														
W30X173 TO W30X261									72	37	28	91	52	40	N/A	64			N/A	81	V	W	X																										
W30X90 TO W30X148									72	37	28	91	52	40	109	68	53	128	86	68																												
W27X146 TO W27X194									72	37	28	91	52	40	109	68	53	128	86	68																												
W27X84 TO W27X129									72	37	28	91	52	40	109	68	53		N/A	64	S	T	U																									
W24X104 TO W24X176							.28	.22	72	37	28	91	52	40	109	68	53		N/A	64																											
W24X68 TO W24X103							.28	.22	72	37	28	91	52	40	109	68	53		N/A	64																											
W24X55 TO W24X62							.28	.22	72	37	28	91	52	40					N/A	64																											
W21X101 TO W21X147							.28	.22	72	37	28	91	52	40					N/A	64																											
W21X48 TO W21X93							.28	.22	72	37	28	91	52	40					N/A	64																											
W21X44 TO W21X57							.28	.22	72	37	28	91	52	40					N/A	64																											
W18X76 TO W18X130							.28	.22	72	37	28	91	52	40					N/A	64																											
W18X50 TO W18X71							.28	.22	72	37	28	91	52	40					N/A	64																											
W18X35 TO W18X46							.28	.22	72	37	28																																					
W16X67 TO W16X100							.28	.22																																							
W16X36 TO W16X57				15	12		.28	.22																																							
W16X26 TO W16X31			33	13	9	52	24	18																																						
W14X61 TO W14X82				15	12		.28	.22																																							
W14X43 TO W14X53			33	13	9	52	24	18																																						
W14X30 TO W14X38				15	12																																									
W14X22 TO W14X26			33	13	9																																									
W12X40 TO W12X58				15	12																																									
W12X26 TO W12X35			33	13	9																																									
W12X14 TO W12X22		7	5	15	11																																									
W10X33 TO W10X45	16	6	4	32	13	9																																							
W10X22 TO W10X30		7	6	D	E	F																																								
W10X12 TO W10X19	16	6	4																																										
	15	6	4																																									
	A	B	C																																									

MIN STABILIZER PLATE THICKNESS (in)

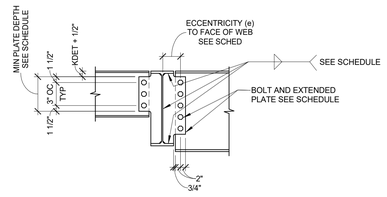
MIN COLUMN SIZE	NUMBER OF 3/4" DIA BOLTS	2	3	4	5	6	7	8	9	10	11	12
W8	x24	3/8	3/8	3/8	3/8	3/8	N/A	N/A	N/A	N/A	N/A	N/A
	x31	3/8	3/8	3/8	3/8	3/8	N/A	N/A	N/A	N/A	N/A	N/A
W10	x33	3/8	3/8	3/8	3/8	3/8	3/8	N/A	N/A	N/A	N/A	N/A
	x49	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W12	x53	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
	x65	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
	x61	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W14	x90	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
	x145	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W16	x77	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W18	x86	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W21	x111	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W24	x131	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W27	x178	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W30	x211	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W30	x263	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8
W36	x302	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8	3/8



DETAIL AT SKEWED BEAMS



BEAM TO COLUMN CONN



BEAM TO BEAM CONN

1 EXTENDED SHEAR PLATE CONNECTION
1/2" = 1'-0"

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

TABLE 2 - ESP2 - 7/8" DIA GR A325N BOLTS

THESE TABLES HAVE BEEN DEVELOPED BY AG&E EXCLUSIVELY FOR USE ON AG&E PROJECTS AND ARE THE INTELLECTUAL PROPERTY OF AG&E. VER 1.0 05/07/2024

EXTENDED SINGLE PLATE SHEAR CONNECTION

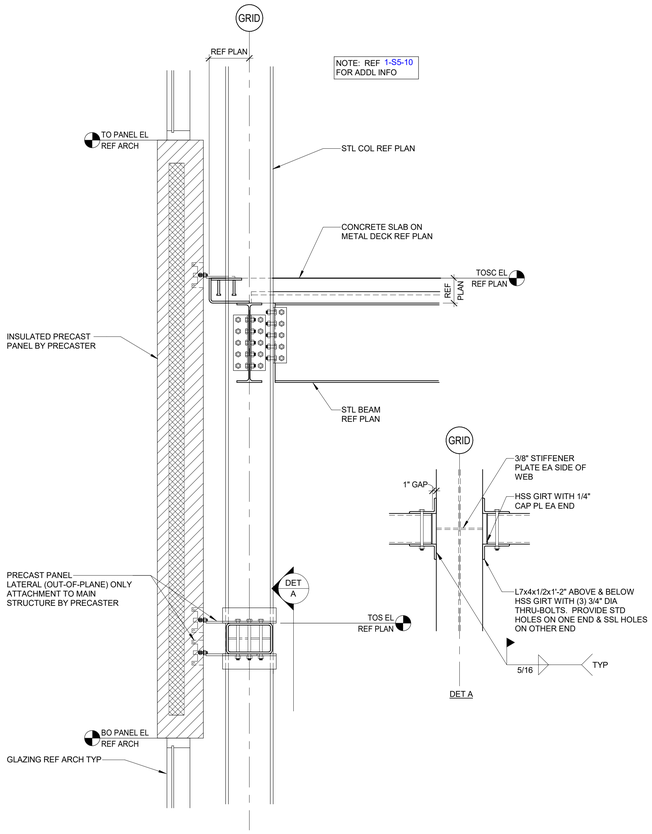
NUMBER OF BOLTS	2	3	4	5	6	7	8	9	10	11	12
EXTENDED PLATE THICKNESS (in)	1/4	5/16	5/16	5/16	3/8	3/8	3/8	3/8	3/8	3/8	3/8
MIN PLATE DEPTH (in)	6	9	12	15	18	21	24	27	30	33	36
FILLET WELD SIZE (in)	3/16	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4	1/4

MAX REACTION (kips) @ MAX ECCENTRICITY (e)

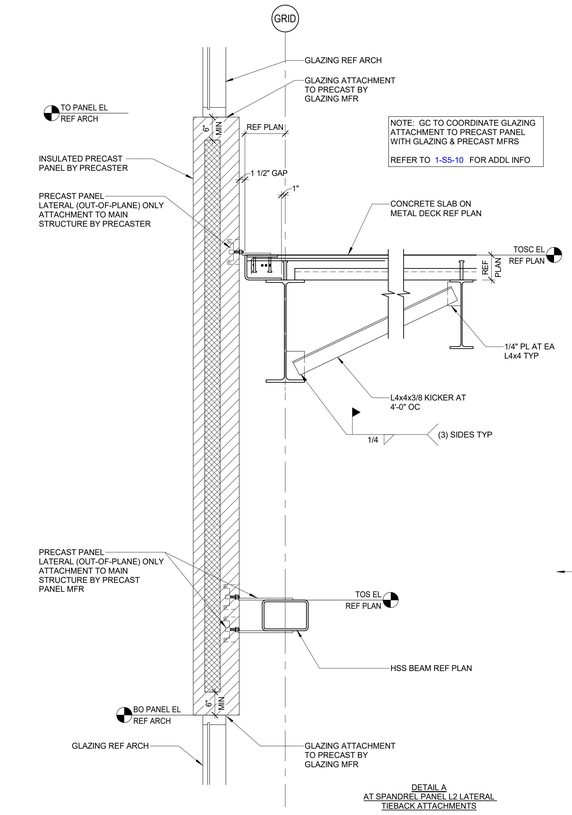
BEAM SIZE

V @ BM TO COL CONN | V @ BM TO BM CONN (e = 6.75") | V @ BM TO BM CONN (e = 7.75") | V @ BM TO BM CONN (e = 8.75") | V @ BM TO BM CONN (e = 10.75")

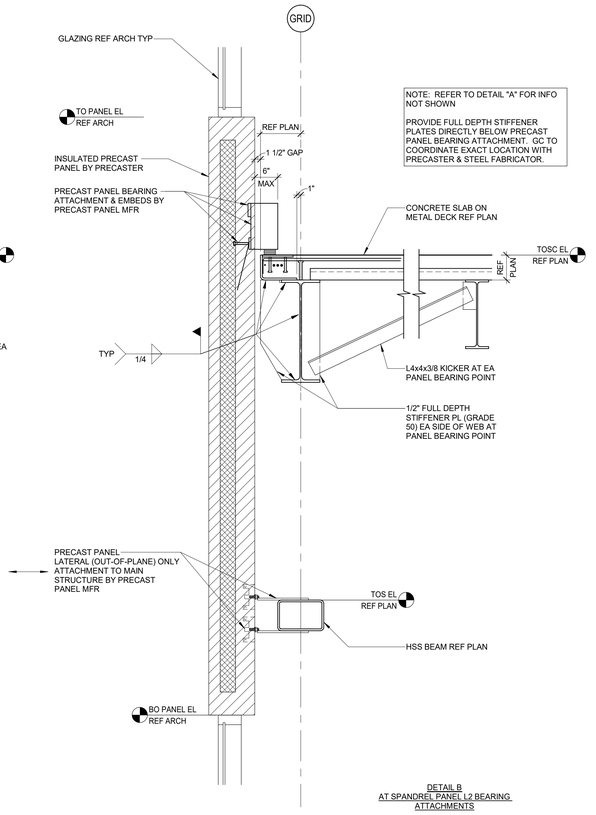
	2					3					4					5					6					7					8					9					10					11					12				
W44X230 TO W44X335																									N/A	87			N/A	110			N/A	134			N/A	159			N/A	187			N/A	N/A		
W40X503 TO W40X655																								141	N/A	87		163	N/A	110		185	N/A	134	206	N/A	159	220	N/A	187												
W40X324 TO W40X431																									N/A	87			N/A	110			N/A	134			N/A	159			N/A	187						
W40X199 TO W40X297																								141	N/A	87		163	N/A	110		185	N/A	134	205	N/A	159			N/A	187						
W40X264 TO W40X392																									N/A	87			N/A	110			N/A	134			N/A	159			N/A	187						
W40X149 TO W40X235																								141	N/A	87		163	N/A	110		185	N/A	134	205	N/A	159			N/A	187						
W36X487 TO W36X652																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159	AB	AC	AD								
W36X302 TO W36X441																	119	71	54	141	93	72	163	117	92	185	N/A	114	206	N/A	137	220	N/A	164	220	N/A	189												
W36X231 TO W36X282																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159			N/A	187	AE	AF	AG				
W36X232 TO W36X256																	119	71	54	141	93	72	163	117	92	185	N/A	114	191	N/A	137																		
W36X135 TO W36X210																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159			N/A	187						
W33X201 TO W33X318															N/A	47	119	71	54	141	93	72	163	117	92	185	N/A	114	186	N/A	137																		
W33X118 TO W33X169															80	50	38	119	71	54	141	93	72	163	117	92	171	N/A	114																				
W30X173 TO W30X261																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159			N/A	187						
W30X90 TO W30X148															80	50	38	119	71	54	141	93	72	149	117	92																						
W27X146 TO W27X194																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159			N/A	187						
W27X84 TO W27X129															80	50	38	119	71	54	137	93	72																									
W24X104 TO W24X176																			N/A	87					N/A	110			N/A	134			N/A	159			N/A	187											
W24X68 TO W24X103															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114	201	N/A	137																
W24X55 TO W24X62																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159											
W21X101 TO W21X147															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114	186	N/A	137																
W21X48 TO W21X93																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159											
W21X44 TO W21X57															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114	181	N/A	137	Y	Z	AA												
W18X76 TO W18X130																			N/A	66					N/A	87			N/A	110			N/A	134			N/A	159											
W18X50 TO W18X71															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W18X35 TO W18X46															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W16X67 TO W16X100																			N/A	87					N/A	110			N/A	134																				
W16X36 TO W16X57															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W16X26 TO W16X31															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W14X61 TO W14X82															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W14X43 TO W14X53															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W14X30 TO W14X38															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W14X22 TO W14X26															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W12X40 TO W12X58															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W12X26 TO W12X35															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W12X14 TO W12X22															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W10X33 TO W10X45															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W10X22 TO W10X30															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				
W10X12 TO W10X19															80	50	38	119	71	54	141	93	72	163	117	92	185	N/A	114																				



4 DETAIL
3/4" = 1'-0"



DETAIL A
AT SPANDREL PANEL L2 LATERAL TIEBACK ATTACHMENTS

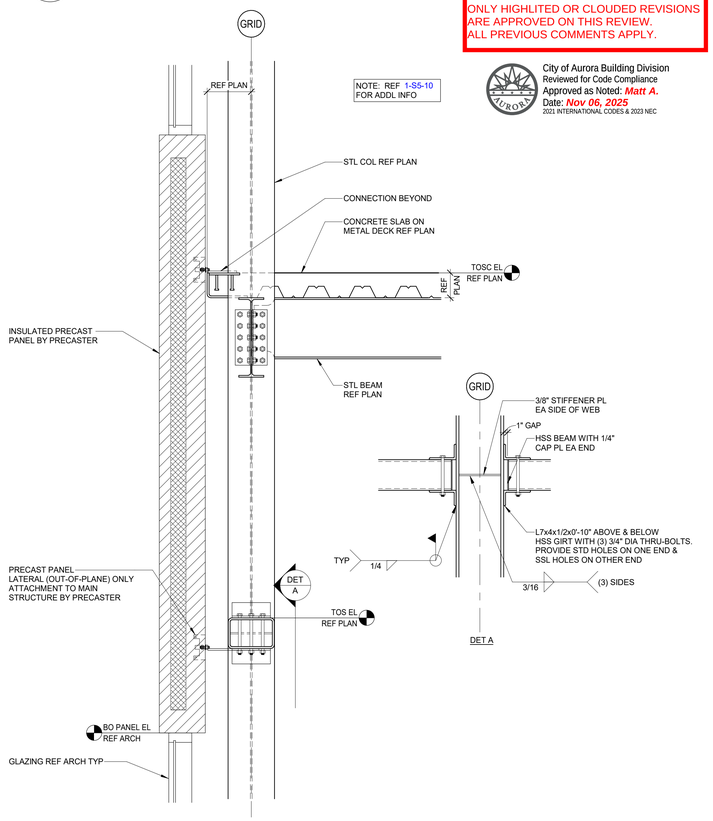


DETAIL B
AT SPANDREL PANEL L2 BEARING ATTACHMENTS

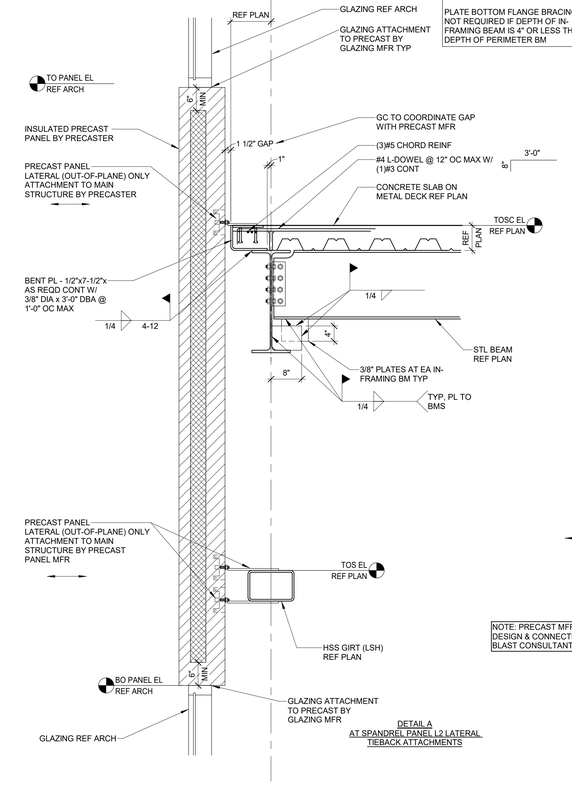
3 DETAIL
3/4" = 1'-0"

ONLY HIGHLITED OR CLOUDED REVISIONS ARE APPROVED ON THIS REVIEW. ALL PREVIOUS COMMENTS APPLY.

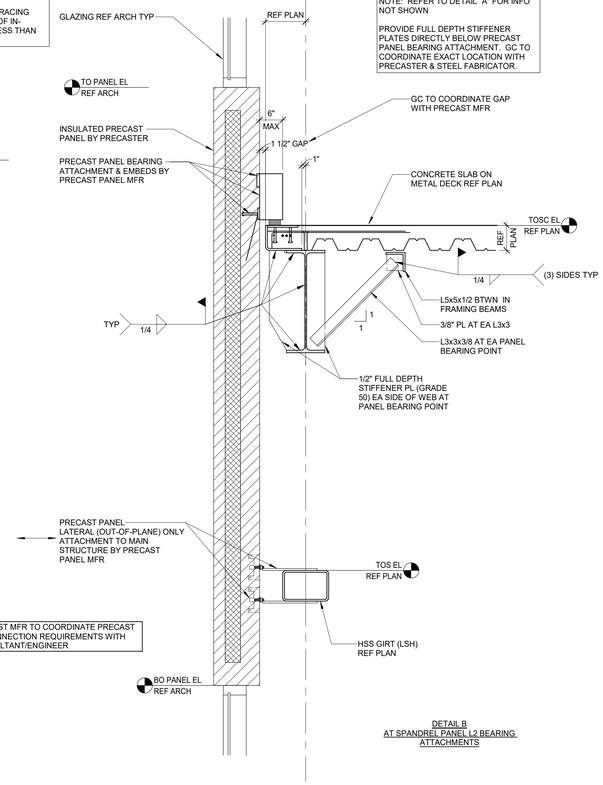
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Date: *Nov 06, 2025*
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2 DETAIL
3/4" = 1'-0"



DETAIL A
AT SPANDREL PANEL L2 LATERAL TIEBACK ATTACHMENTS



NOTE: PRECAST MFR TO COORDINATE PRECAST DESIGN & CONNECTION REQUIREMENTS WITH BLAST CONSULTANT/ENGINEER

DETAIL B
AT SPANDREL PANEL L2 BEARING ATTACHMENTS

1 SPANDREL TO FLOOR CONNECTION AT OFFICE WITH GLAZING
3/4" = 1'-0"

9/10/2025 5:08:17 PM S5-10 STEEL FLOOR FRAMING DETAILS

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SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

ISSUES		
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REVISIONS		
1	03.28.25	ADDENDUM 1

COLORADO LICENSED
NICHOLAS W. KEVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

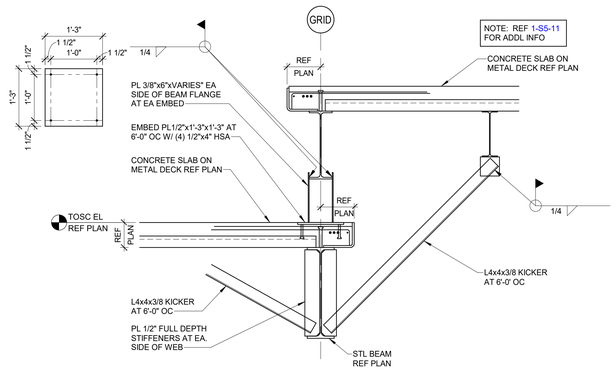
STEEL FLOOR FRAMING DETAILS

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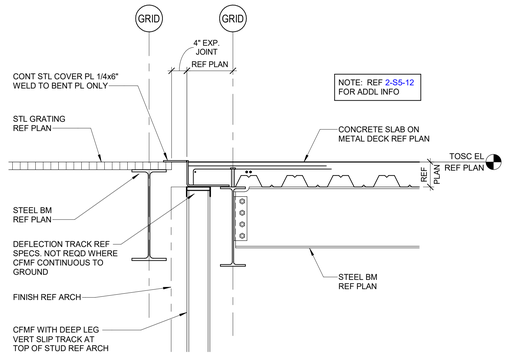
S5-10

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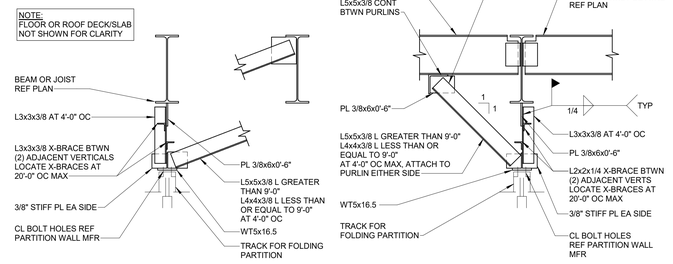
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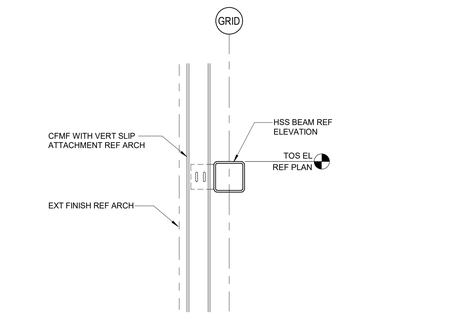
11 DIAPHRAGM TRANSFER AT SLAB STEP FOR RAISED ACCESS FLOOR
3/4" = 1'-0"



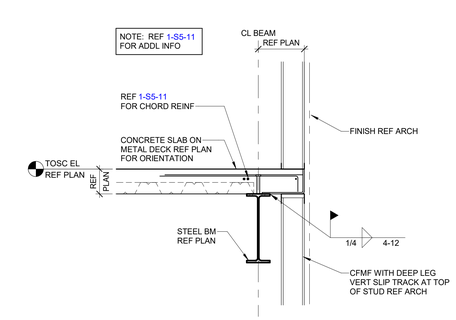
10 EXPANSION JOINT AT MECHANICAL PLATFORM
3/4" = 1'-0"



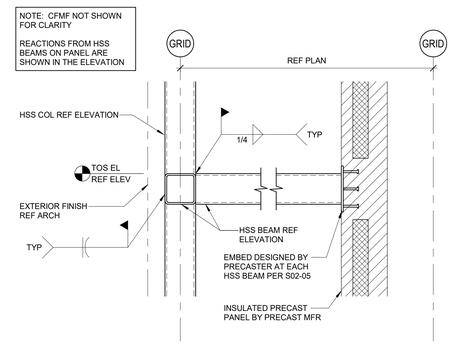
9 FOLDING PARTITION SUPPORT
3/4" = 1'-0"



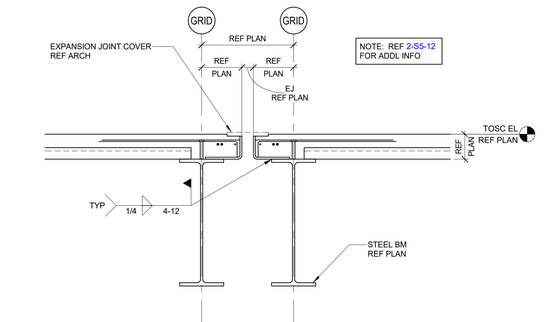
8 DETAIL
3/4" = 1'-0"



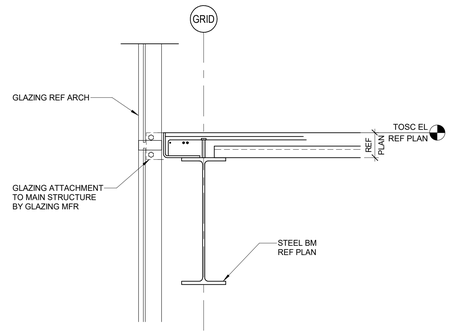
7 DETAIL
3/4" = 1'-0"



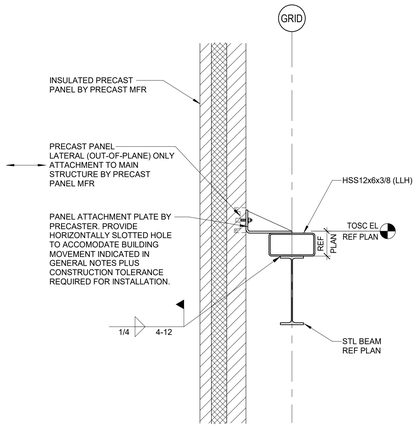
6 DETAIL
3/4" = 1'-0"



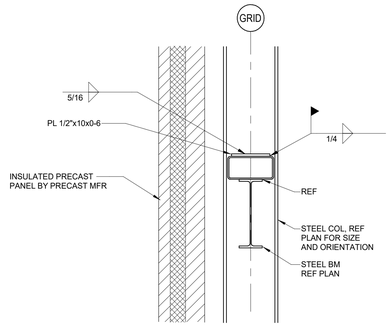
5 EXPANSION JOINT AT SECOND FLOOR
3/4" = 1'-0"



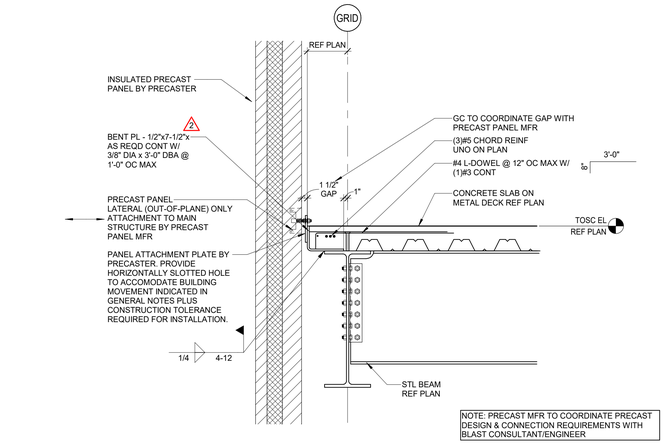
4 DETAIL
3/4" = 1'-0"



3 DETAIL
3/4" = 1'-0"



2 HSS CONNECTION TO COLUMN DETAIL
3/4" = 1'-0"



1 SECTION AT TYPICAL EXTERIOR CONCRETE WALL PANEL
3/4" = 1'-0"

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	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

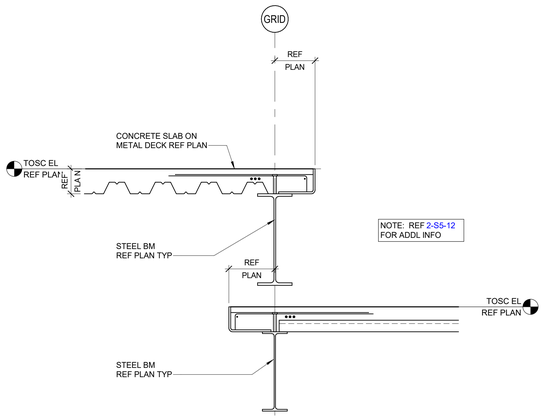
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STEEL FLOOR FRAMING DETAILS

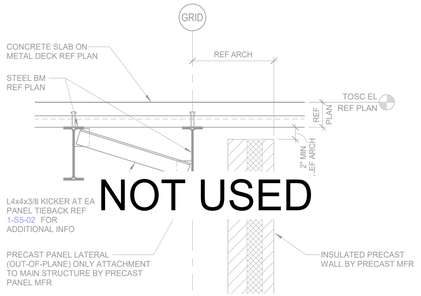
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S5-11

9/10/2025 5:08:18 PM S5-11 STEEL FLOOR FRAMING DETAILS



16 DETAIL
3/4" = 1'-0"

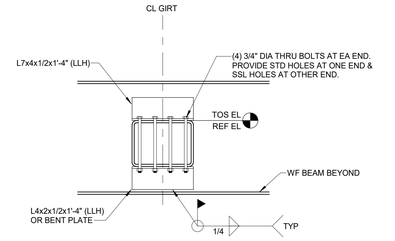


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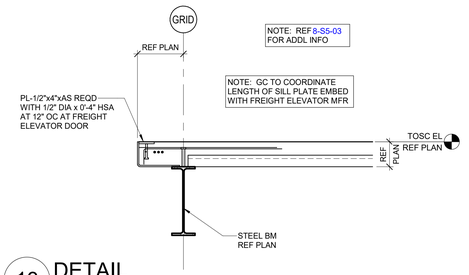
15 DETAIL
3/4" = 1'-0"

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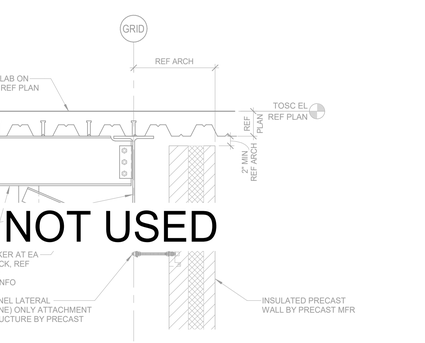
City of Aurora Building Division
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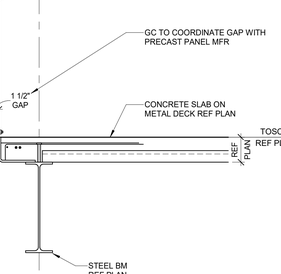
14 DETAIL
3/4" = 1'-0"



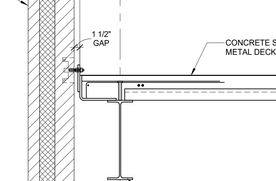
13 DETAIL
3/4" = 1'-0"



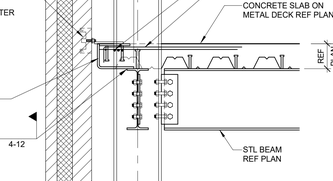
12 DETAIL
3/4" = 1'-0"



11 DETAIL
3/4" = 1'-0"

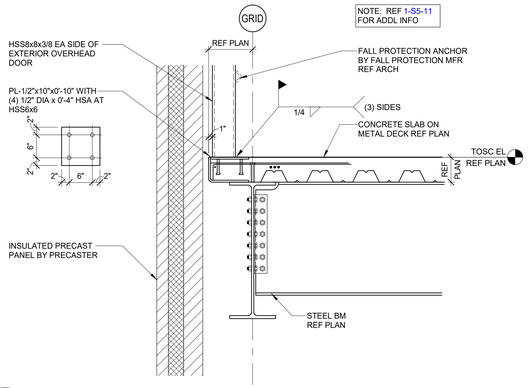


10 DETAIL
3/4" = 1'-0"

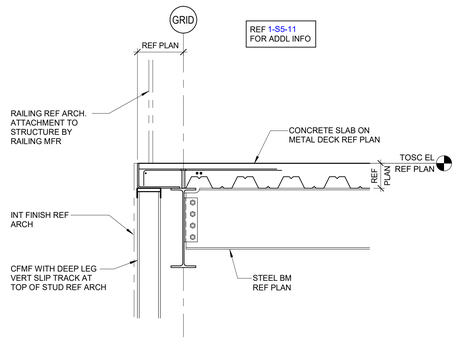


9 DETAIL
3/4" = 1'-0"

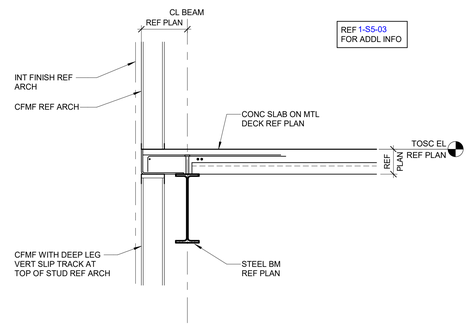
8 DETAIL
3/4" = 1'-0"



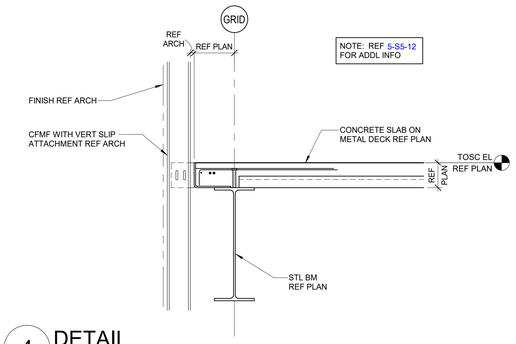
7 DETAIL
3/4" = 1'-0"



6 DETAIL
3/4" = 1'-0"



5 DETAIL
3/4" = 1'-0"



4 DETAIL
3/4" = 1'-0"

3 DETAIL
3/4" = 1'-0"

2 DETAIL
3/4" = 1'-0"

1 DETAIL
3/4" = 1'-0"

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	REVISIONS	
1	03.28.25	ADDENDUM 1

COLORADO LICENSED
NICHOLAS W. REYID
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AG&E Structural Engenuity

Date of issue:
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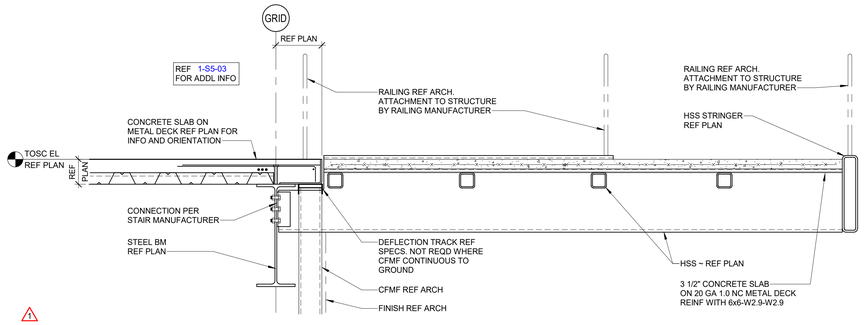
QTS DEN1 DC2
1140 N. GUN CLUB ROAD

STEEL FLOOR FRAMING DETAILS

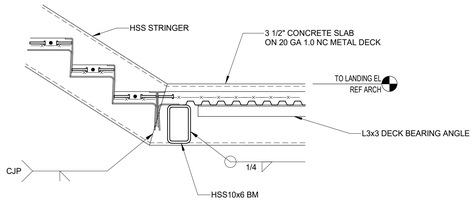
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S5-12

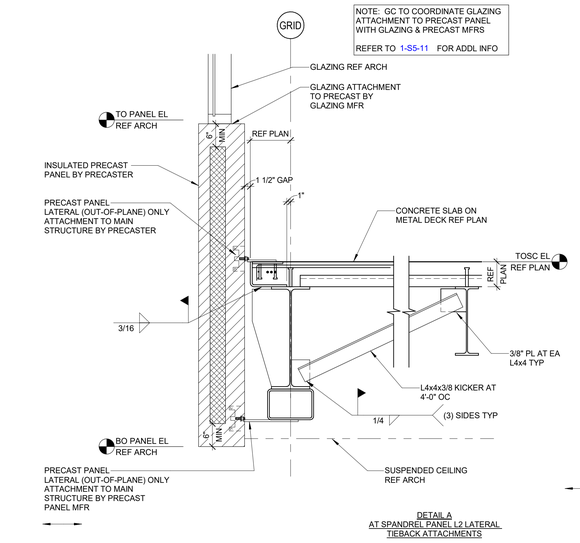
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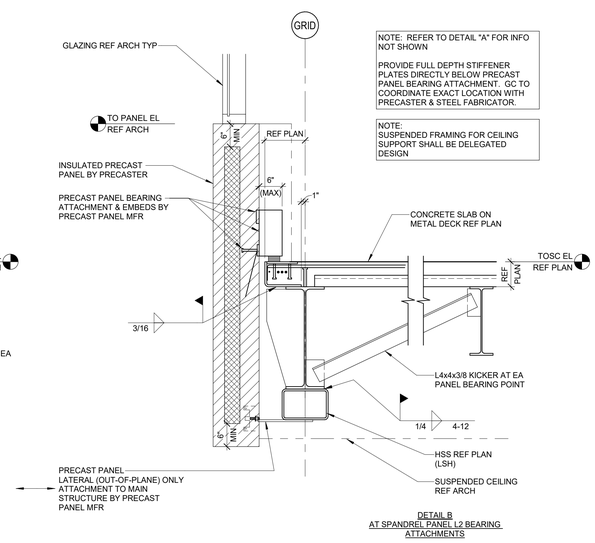


8 STAIR FRAMING SECTION
3/4" = 1'-0"



7 STAIR FRAMING SECTION
3/4" = 1'-0"





6 PRECAST SPANDREL PANEL CONNECTION SHALLOW SPANDREL
3/4" = 1'-0"

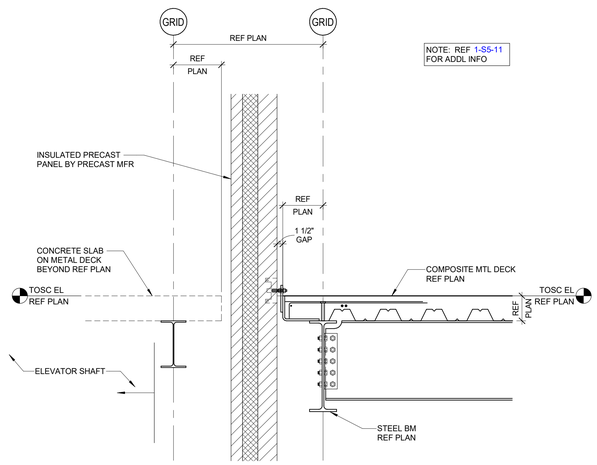
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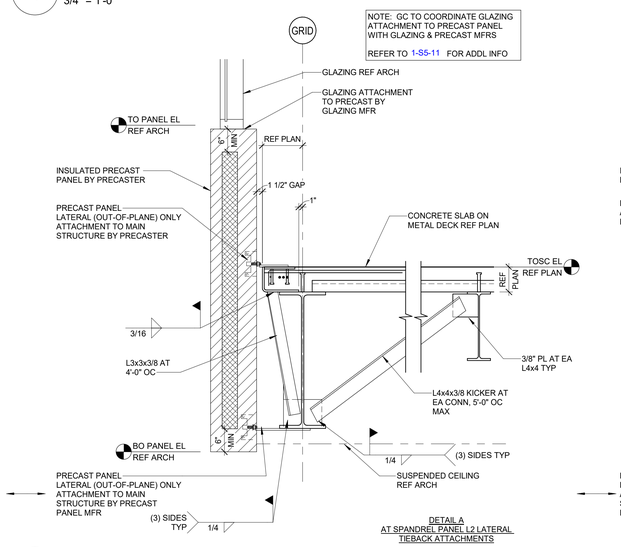
5 DETAIL
3/4" = 1'-0"

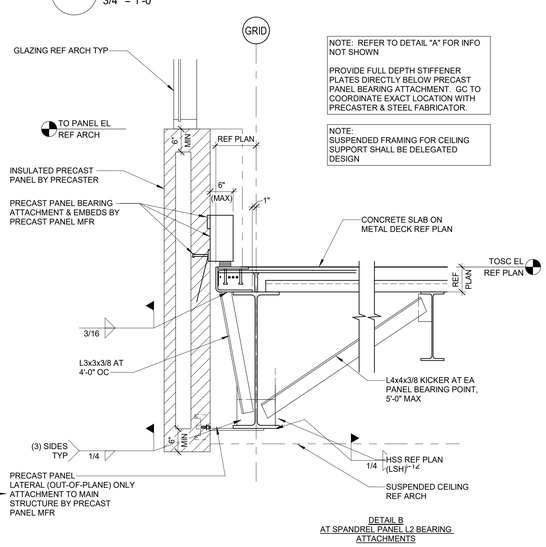
4 DETAIL
3/4" = 1'-0"

3 DETAIL
3/4" = 1'-0"



2 DETAIL
3/4" = 1'-0"





1 DETAIL AT SPANDREL CONNECTION AT FLOOR
3/4" = 1'-0"

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COLORADO LICENSED
NICHOLAS W. REVID
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QTS DEN1 DC2
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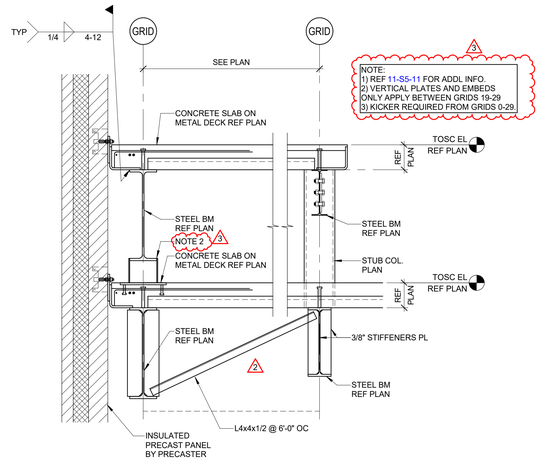
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STEEL FLOOR FRAMING DETAILS

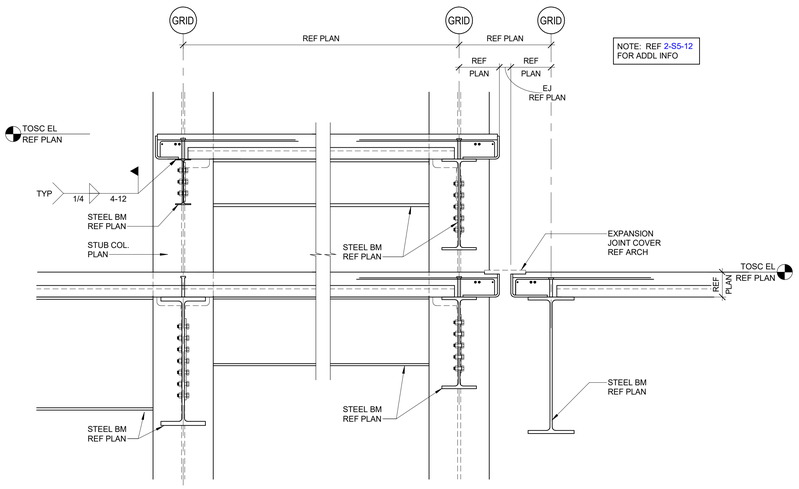
JOB 24464.0000
DATE 09.10.2025
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S5-13

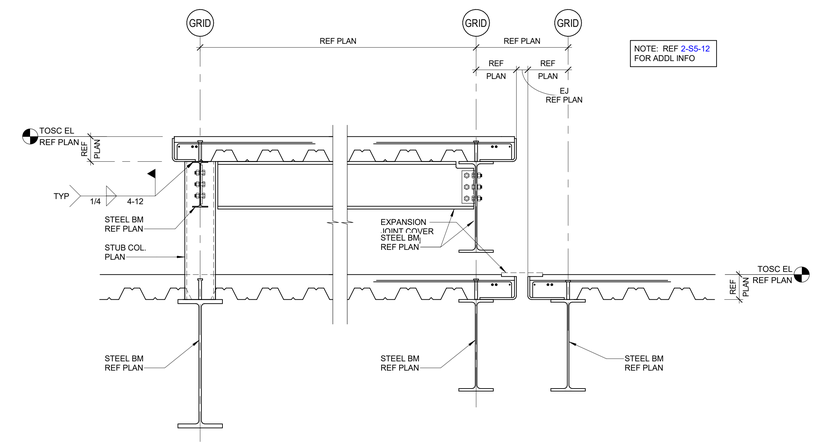
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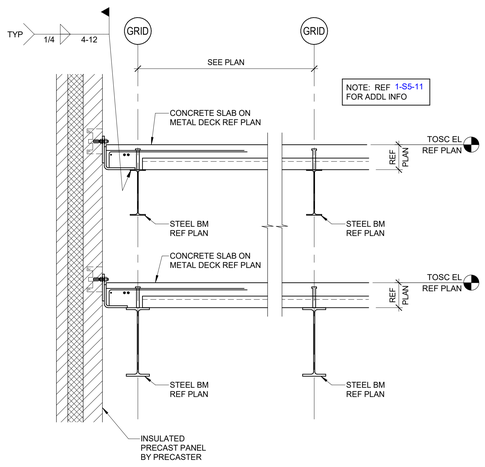
14 SECTION AT SECOND FLOOR
3/4" = 1'-0"



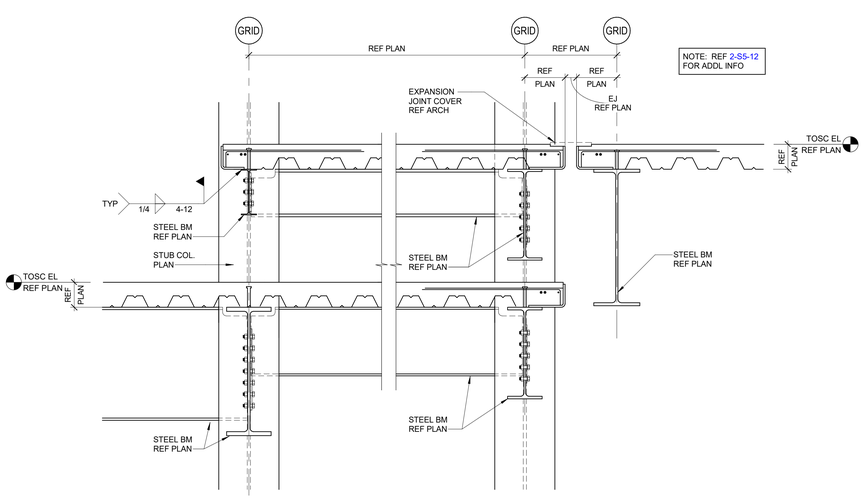
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3/4" = 1'-0"



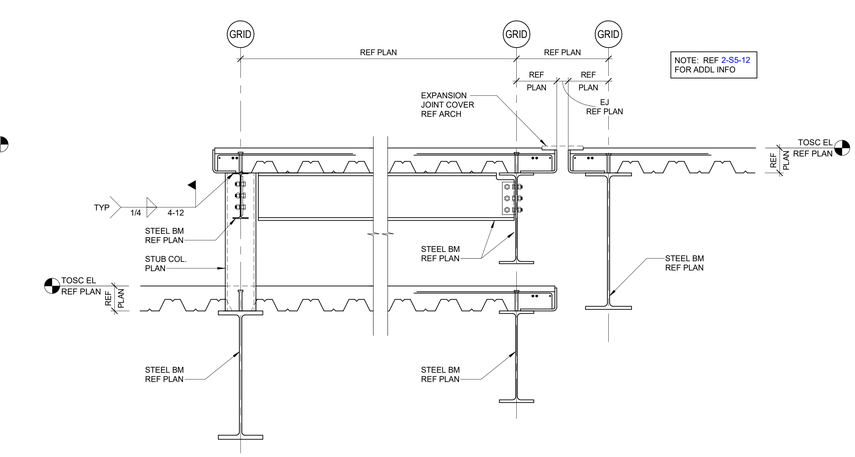
12 SECTION AT SECOND FLOOR
3/4" = 1'-0"



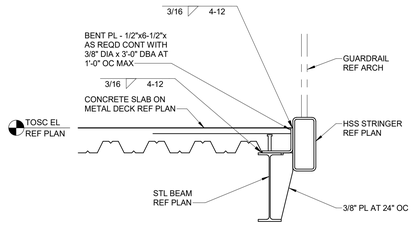
11 SECTION AT SECOND FLOOR
3/4" = 1'-0"



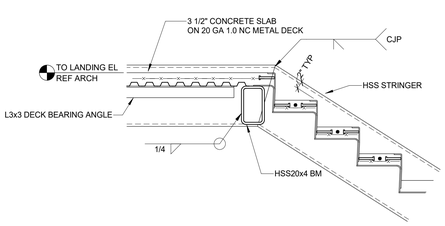
10 SECTION AT SECOND FLOOR
3/4" = 1'-0"



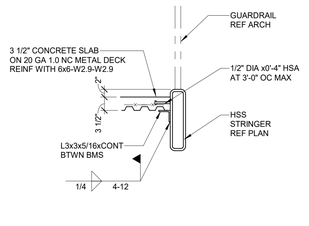
9 SECTION AT SECOND FLOOR
3/4" = 1'-0"



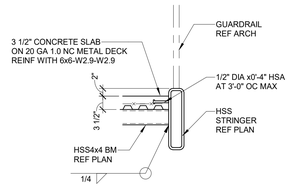
8 DETAIL
3/4" = 1'-0"



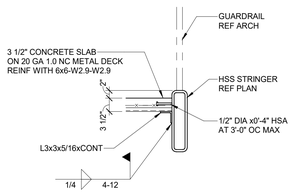
7 STAIR FRAMING SECTION
3/4" = 1'-0"



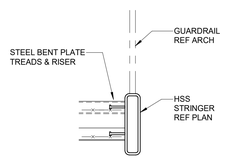
6 STAIR FRAMING SECTION
3/4" = 1'-0"



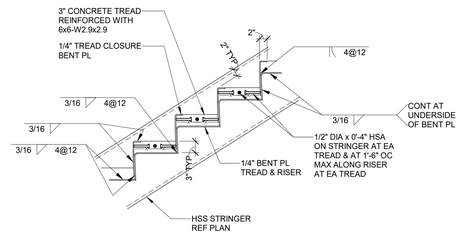
5 STAIR FRAMING SECTION
3/4" = 1'-0"



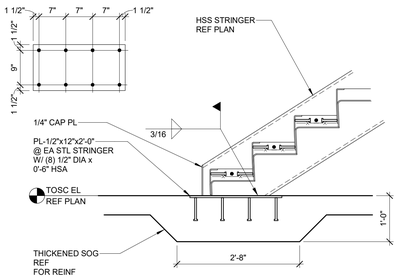
4 STAIR FRAMING SECTION
3/4" = 1'-0"



3 STAIR FRAMING SECTION
3/4" = 1'-0"



2 STAIR FRAMING SECTION
3/4" = 1'-0"



1 STAIR FRAMING SECTION
3/4" = 1'-0"

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City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC

9/10/2025 5:08:23 PM S5-14 STEEL FLOOR FRAMING DETAILS

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000

AG/E
SYSKA HENNESSY GROUP
DESIGN CONSULTING LLC

	ISSUES	
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7		
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10		

	REVISIONS	
1	03.28.25	ADDENDUM 1
2	05.30.25	ISSUE FOR CONSTRUCTION
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COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD
QTS

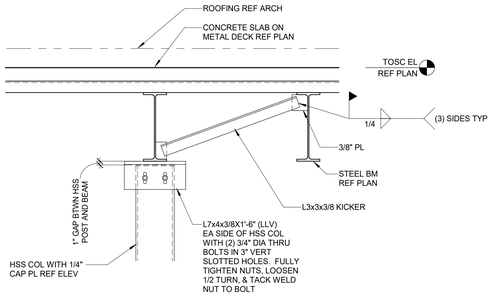
STEEL FLOOR FRAMING DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

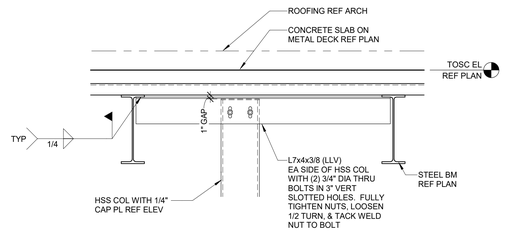
S5-14

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6 DETAIL
3/4" = 1'-0"

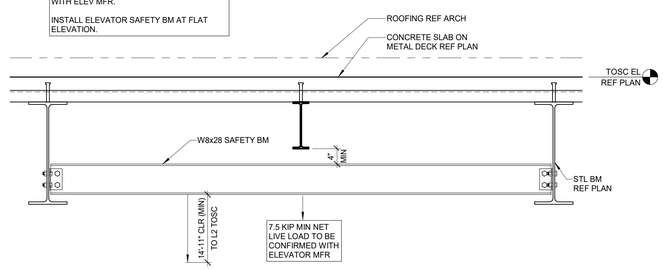


5 DETAIL
3/4" = 1'-0"

NOTE: GC TO COORDINATE PLACEMENT OF HOIST BM WITH ELEV MFR.

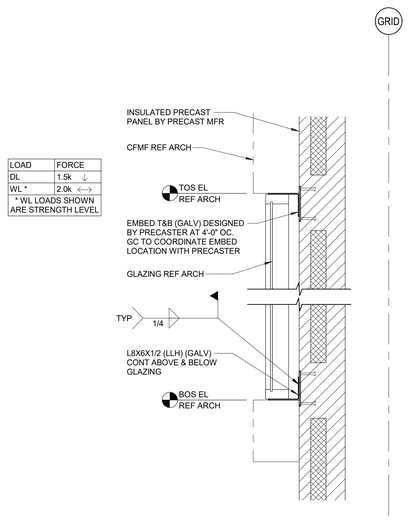
GC TO VERIFY MIN OVERHEAD CLEARANCE WITH ELEV MFR.

INSTALL ELEVATOR SAFETY BM AT FLAT ELEVATION.

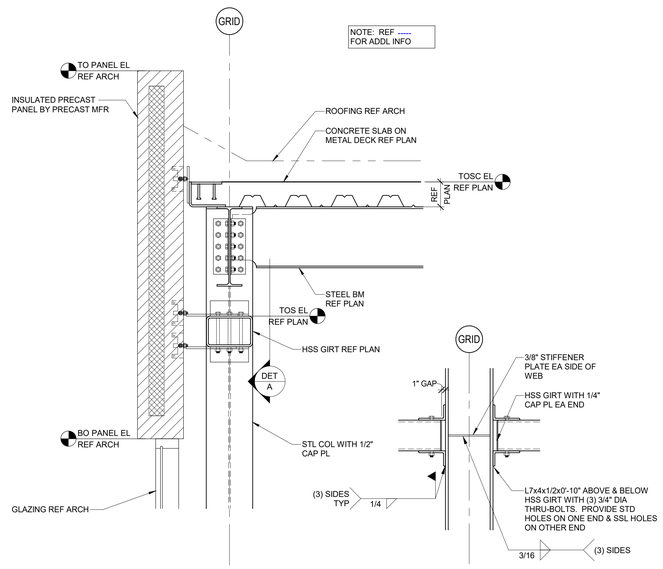


4 DETAIL
3/4" = 1'-0"

LOAD	FORCE
DL	1.5k ↓
WL *	2.0k ↔
* WL LOADS SHOWN ARE STRENGTH LEVEL	

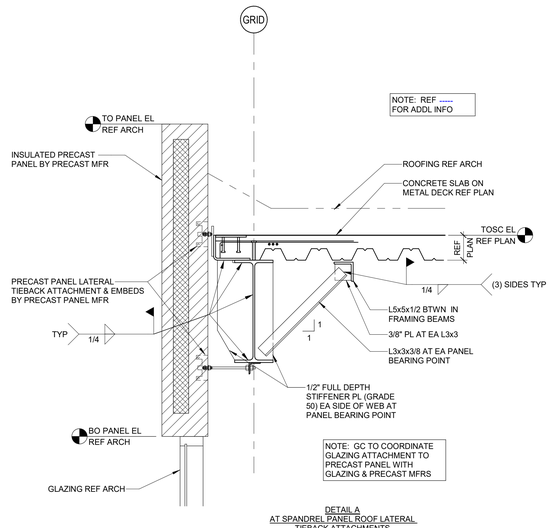


3 DETAIL
3/4" = 1'-0"

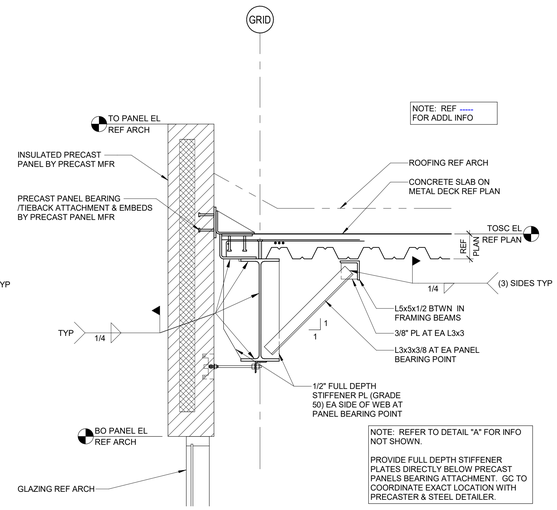


DETAIL A

2 DETAIL
3/4" = 1'-0"



DETAIL A
AT SPANDREL PANEL ROOF LATERAL TIEBACK ATTACHMENTS



DETAIL B
AT SPANDREL PANEL ROOF BEARING ATTACHMENTS

1 DETAIL
3/4" = 1'-0"

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SYSKA HENNESSY GROUP

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	REVISIONS	
A	04.24.25	COA - PERMIT COMMENTS

COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

STEEL ROOF FRAMING SECTIONS AND DETAILS

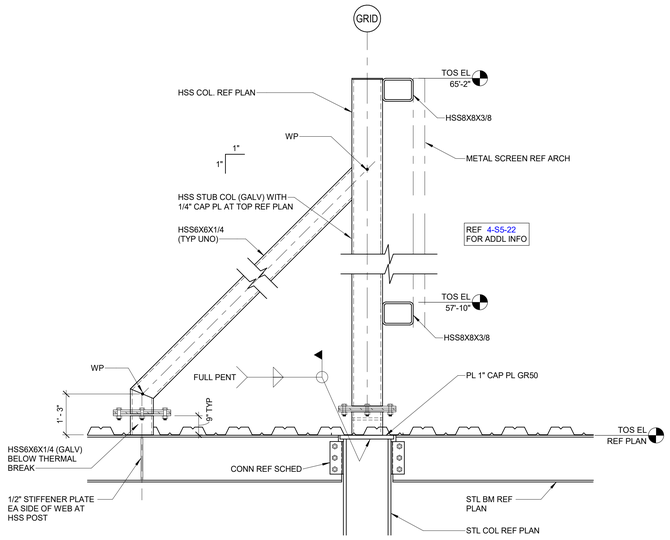
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S5-20

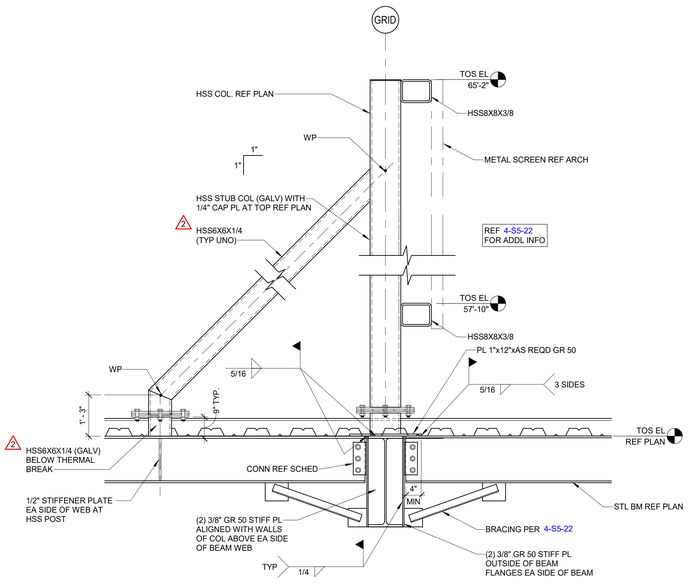
9/10/2025 5:08:24 PM S5-20 STEEL ROOF FRAMING SECTIONS AND DETAILS

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11 ROOFTOP SCREENWALL POST AT COLUMN
3/4" = 1'-0"



10 ROOFTOP SCREENWALL POST AT GIRDER
3/4" = 1'-0"

9 CFMF DETAIL AT VESTIBULE ROOF
3/4" = 1'-0"

8 CFMF DETAIL AT VESTIBULE ROOF
3/4" = 1'-0"

7 CFMF DETAIL AT VESTIBULE ROOF
3/4" = 1'-0"

6 DETAIL
3/4" = 1'-0"

5 PRECAST CONN AT PARAPET
3/4" = 1'-0"

4 PRECAST CONN AT PARAPET
3/4" = 1'-0"

3 PRECAST CONN AT PARAPET
3/4" = 1'-0"

2 TYPICAL EDGE OF ROOF DECK
3/4" = 1'-0"

1 TYPICAL EDGE OF ROOF DECK
3/4" = 1'-0"

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2	05.30.25	ISSUE FOR CONSTRUCTION

COLORADO LICENSED
NICHOLAS W. KEVIL
65236
PROFESSIONAL ENGINEER
AG&E Structural Engenuity

Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

STEEL ROOF FRAMING SECTIONS AND DETAILS

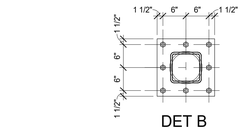
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S5-21

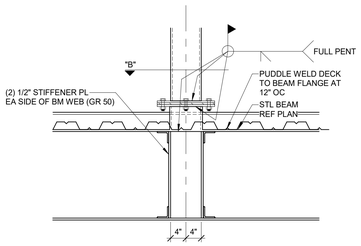
9/10/2025 5:08:25 PM S5-21 STEEL ROOF FRAMING SECTIONS AND DETAILS

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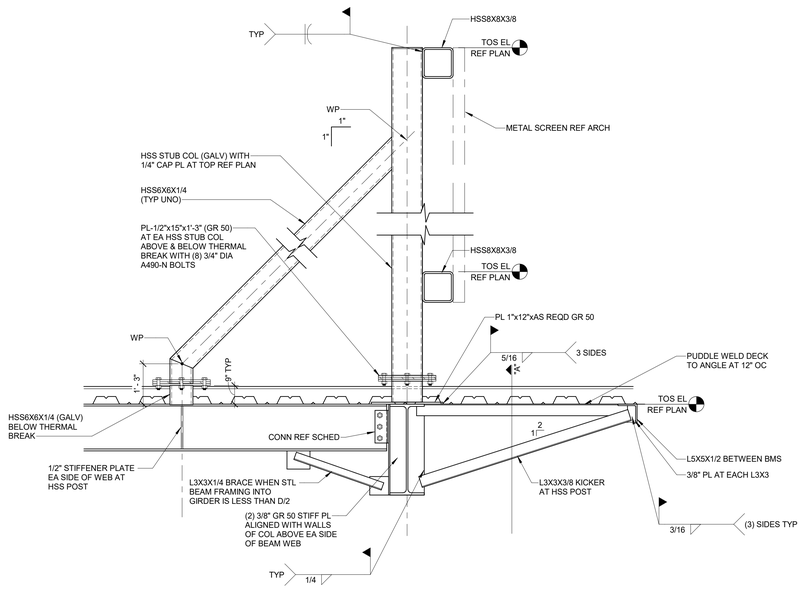
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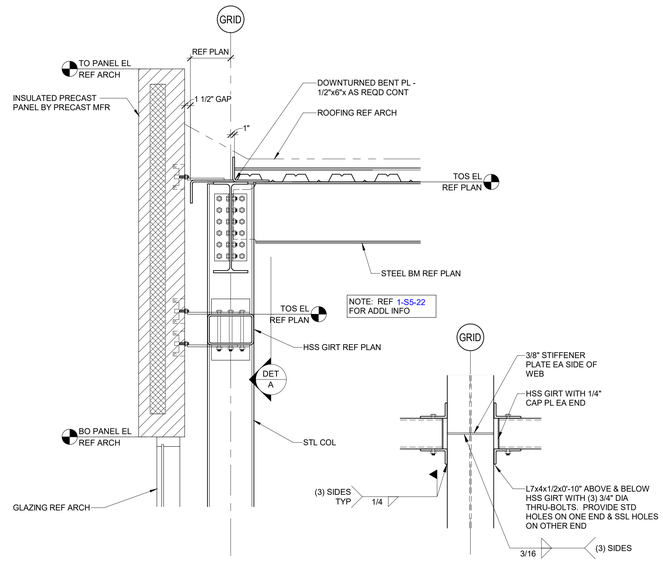
DET B



DET A

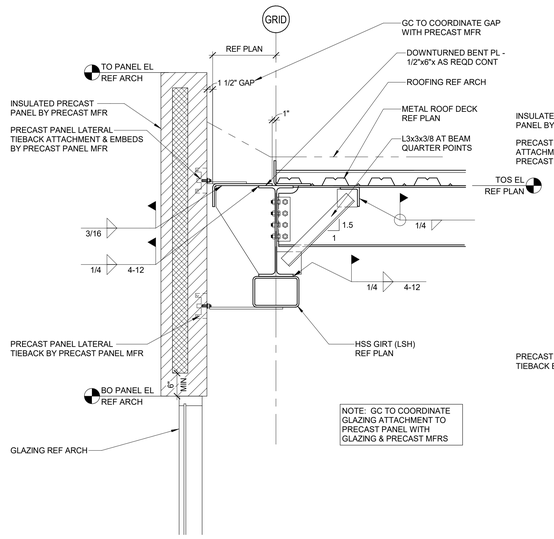


4 ROOFTOP SCREENWALL POST AT BEAM
3/4" = 1'-0"

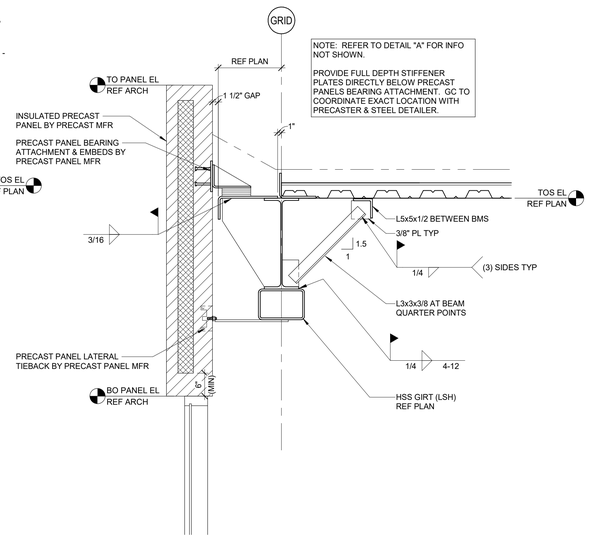


DET A

3 GIRT CONNECTION TO COLUMN AT PRECAST SPANDREL
3/4" = 1'-0"

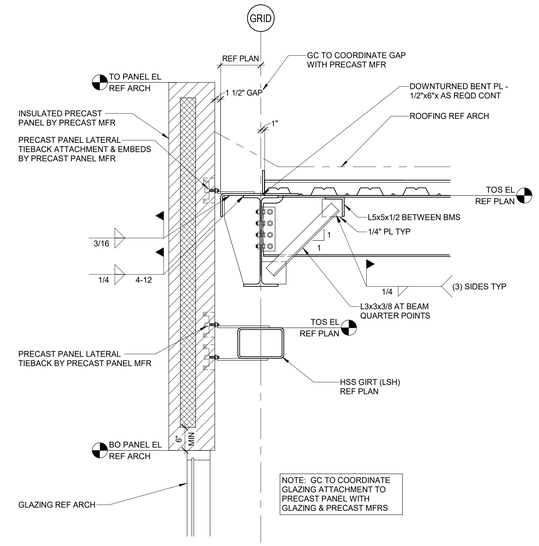


DETAIL A
AT SPANDREL PANEL ROOF LATERAL
TIEBACK ATTACHMENTS

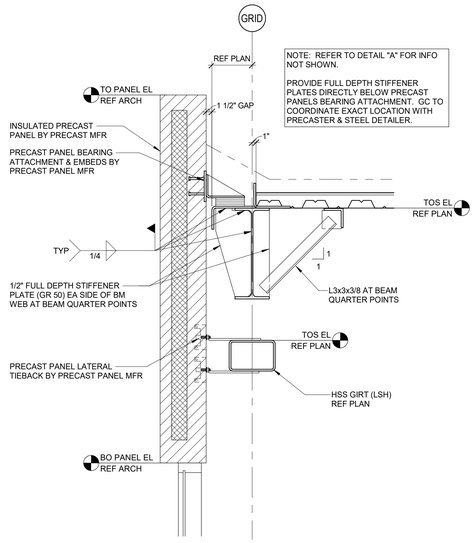


DETAIL B
AT SPANDREL PANEL ROOF BEARING
ATTACHMENTS

2 PRECAST SPANDREL PANEL CONN AT FOH
3/4" = 1'-0"



DETAIL A
AT SPANDREL PANEL ROOF LATERAL
TIEBACK ATTACHMENTS



DETAIL B
AT SPANDREL PANEL ROOF BEARING
ATTACHMENTS

1 PRECAST SPANDREL PANEL CONN AT FOH
3/4" = 1'-0"

9/10/2025 5:08:26 PM S5-22 STEEL ROOF FRAMING SECTIONS AND DETAILS

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△	REVISIONS	
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COLORADO LICENSED
NICHOLAS W. REVID
65236
PROFESSIONAL ENGINEER

AG&E Structural Engenuity

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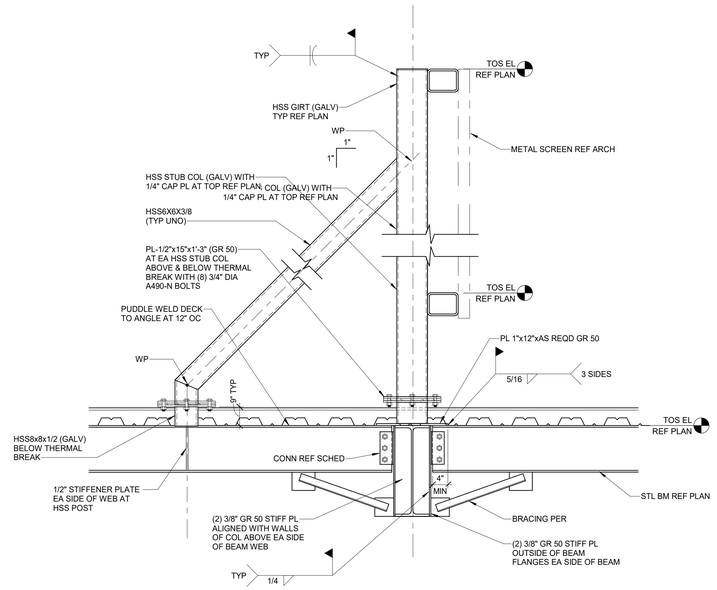
QTS DEN1 DC2
1140 N. GUN CLUB ROAD

QTS

STEEL ROOF FRAMING SECTIONS AND DETAILS

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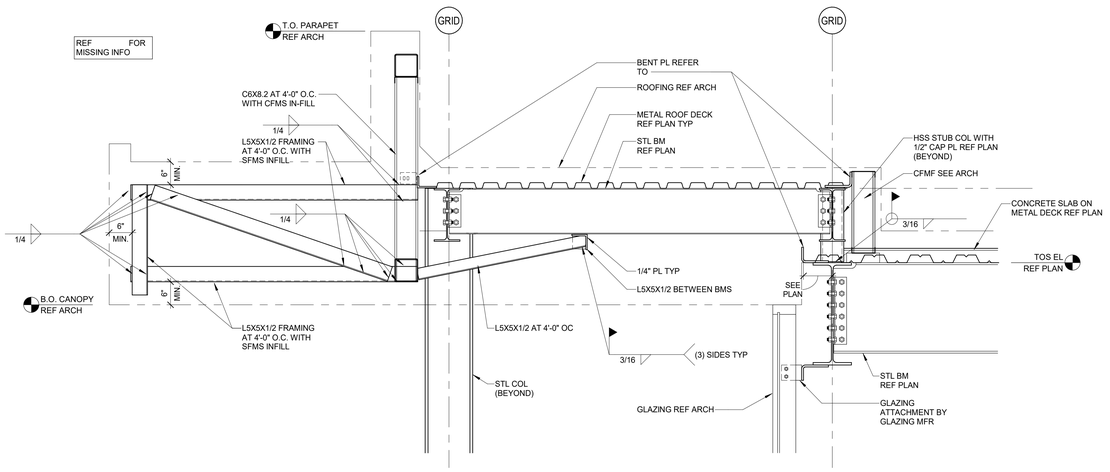
S5-22



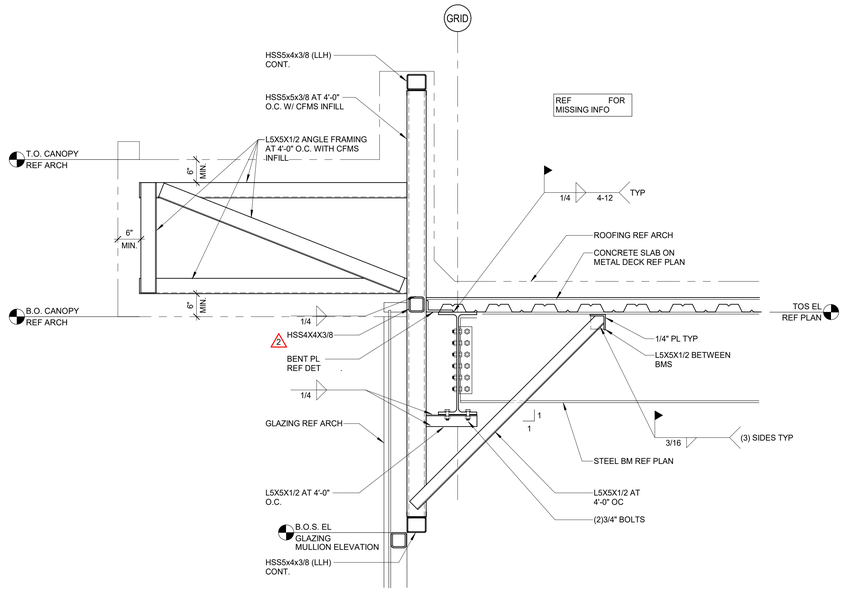
3 ROOFTOP SCREENWALL POST AT GIRDER
3/4" = 1'-0"

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2 TYPICAL DETAIL AT EYEBROW CANOPY
3/4" = 1'-0"



1 TYPICAL DETAIL AT EYEBROW CANOPY
3/4" = 1'-0"

9/10/2025 5:08:27 PM S5-23 STEEL ROOF FRAMING SECTIONS AND DETAILS

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NICHOLAS W. KEVID
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REVISIONS		
1	03.28.25	ADDENDUM 1

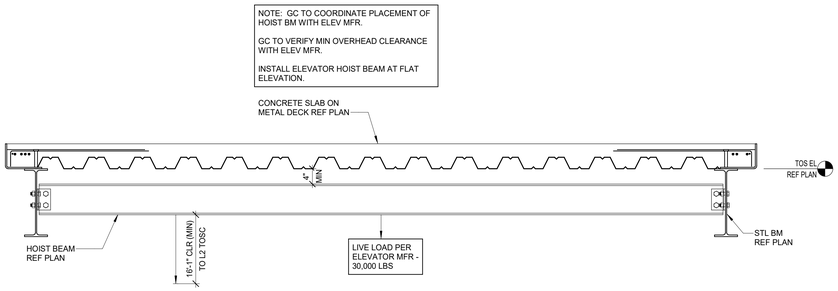
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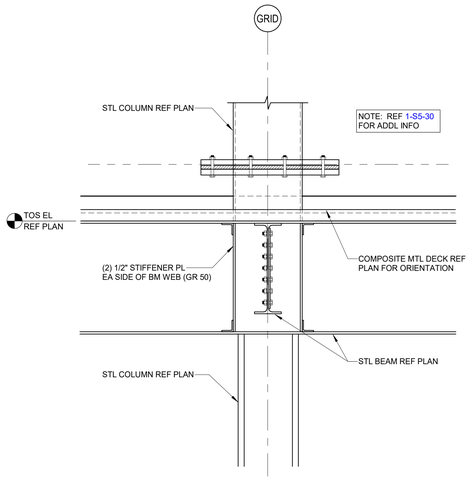
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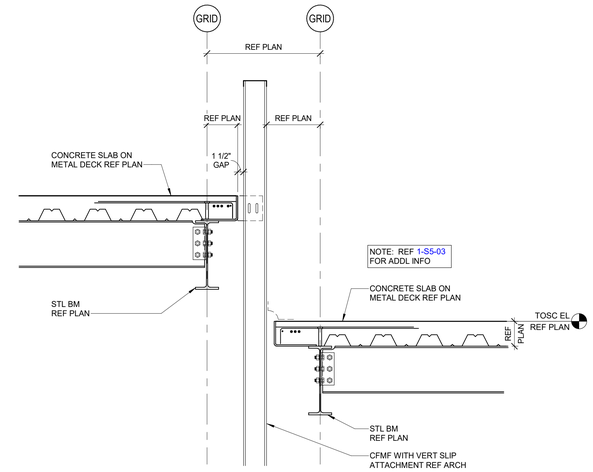
City of Aurora Building Division
Reviewed for Code Compliance
Approved as Noted: *Matt A.*
Date: *Nov 06, 2025*
2021 INTERNATIONAL CODES & 2023 NEC



3 ROOF FRAMING DETAIL
3/4" = 1'-0"



2 ROOF FRAMING DETAIL
3/4" = 1'-0"



1 ROOF FRAMING DETAIL
3/4" = 1'-0"

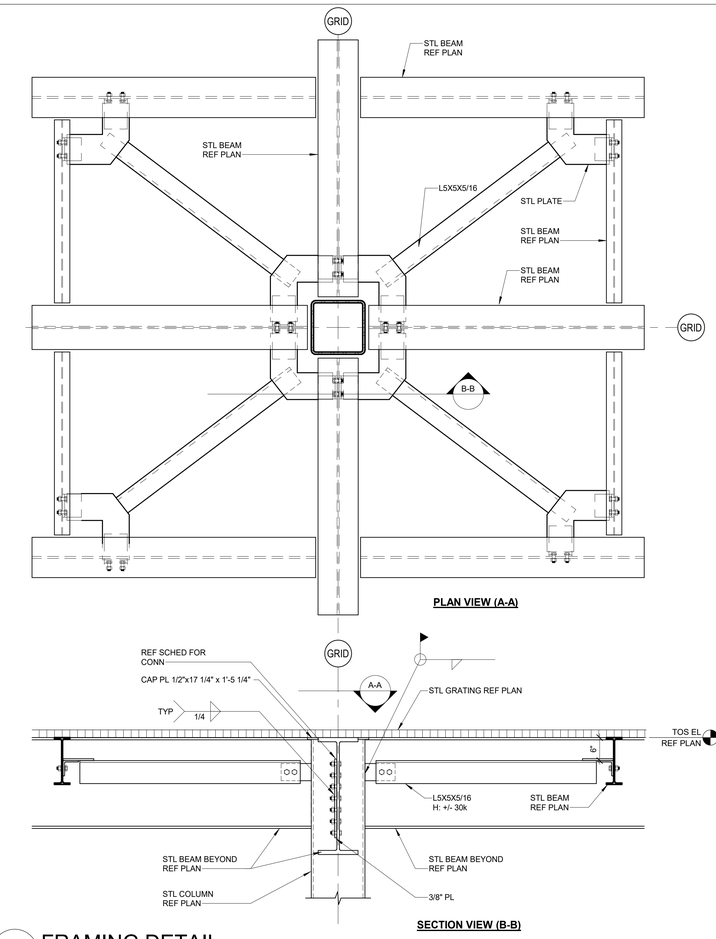
QTS DEN1 DC2
1140 N. GUN CLUB ROAD

STEEL ROOF FRAMING SECTIONS AND DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

S5-24

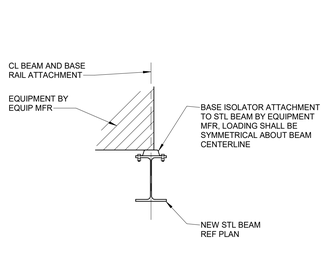
9/10/2025 5:08:27 PM S5-24 STEEL ROOF FRAMING SECTIONS AND DETAILS



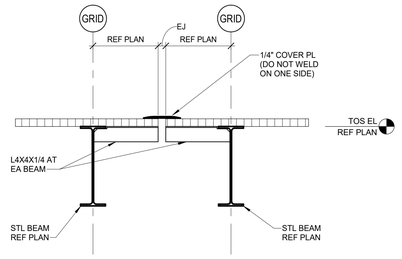
7 FRAMING DETAIL
3/4" = 1'-0"

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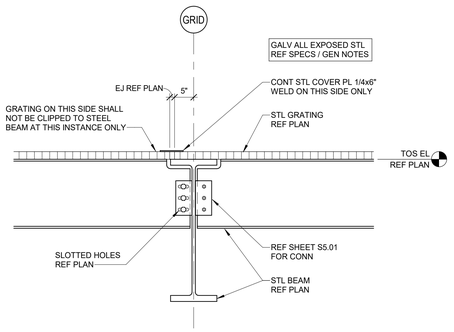
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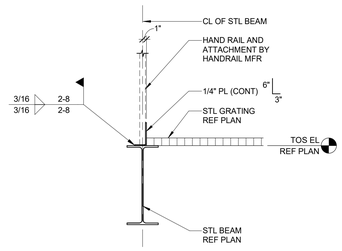
6 TYPICAL ACC ATTACHMENT AT STL BEAM
3/4" = 1'-0"



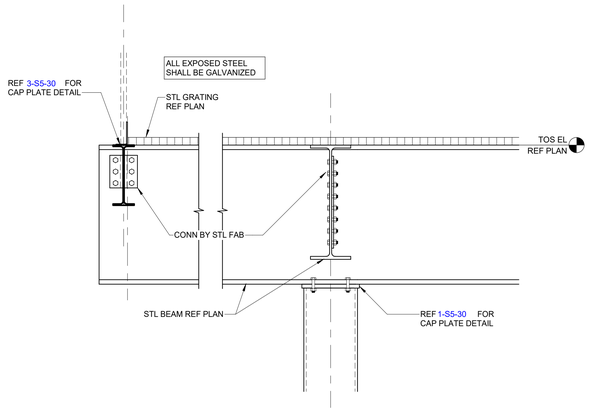
5 DETAIL
3/4" = 1'-0"



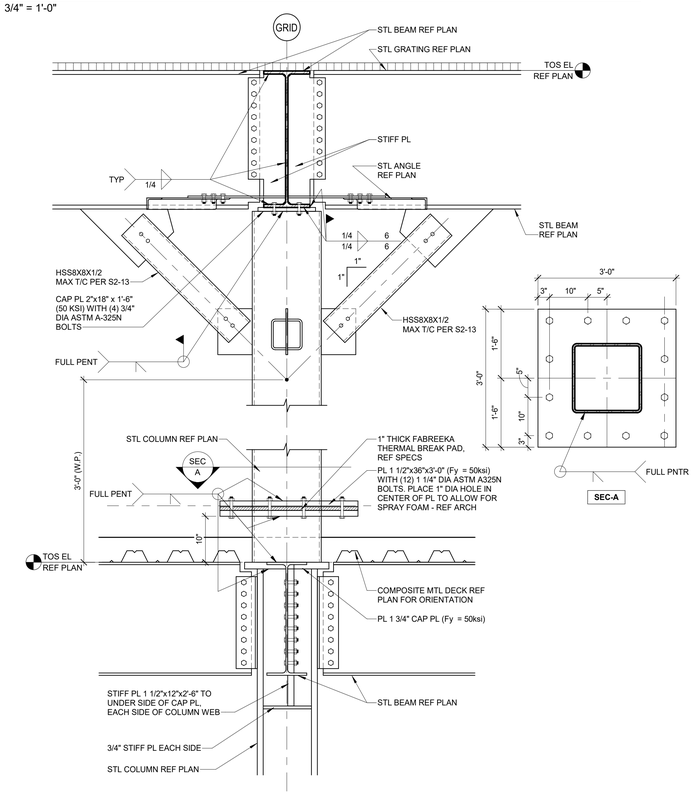
4 FRAMING DETAIL
3/4" = 1'-0"



3 TYPICAL HAND RAIL DETAIL
3/4" = 1'-0"



2 FRAMING DETAIL
3/4" = 1'-0"



1 STEEL COLUMN AT CHILLER PLATFORM/BRACE
3/4" = 1'-0"

9/10/2025 5:08:28 PM S5-30 PLATFORM FRAMING SECTION AND DETAILS

CORGAN
401 N. Houston Street Dallas, TX 75202
T: 214.748.2000
AG/E
SYSKA HENNESSY GROUP
TLD DESIGN CONSULTING LLC

	ISSUES	
1	12.20.24	DESIGN DEVELOPMENT
2	01.31.25	ISSUE FOR PERMIT
3	04.18.25	DH FIT OUT - DD
4	05.16.25	DH FIT OUT - IFP
5	05.30.25	ISSUE FOR CONSTRUCTION
6	09.10.25	IFC.1
7		
8		
9		
10		

	REVISIONS	
1	03.28.25	ADDENDUM 1

COLORADO LICENSED NICHOLAS W. REYID 65236 PROFESSIONAL ENGINEER
AG&E Structural Engenuity

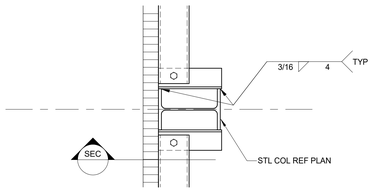
Date of issue:
09.10.2025

QTS DEN1 DC2
1140 N. GUN CLUB ROAD

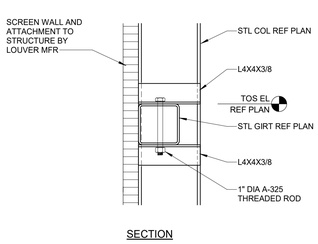
PLATFORM FRAMING SECTION AND DETAILS

JOB 24464.0000
DATE 09.10.2025
SHEET

S5-30

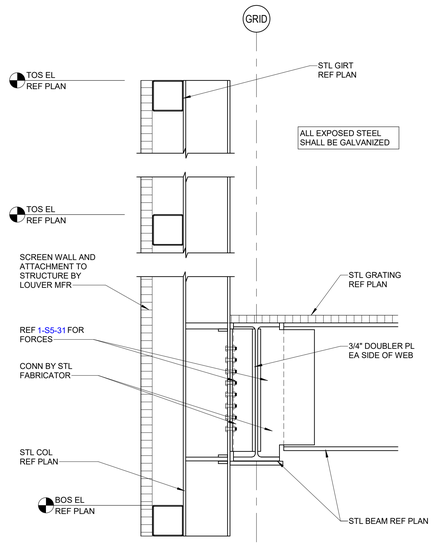


PLAN VIEW

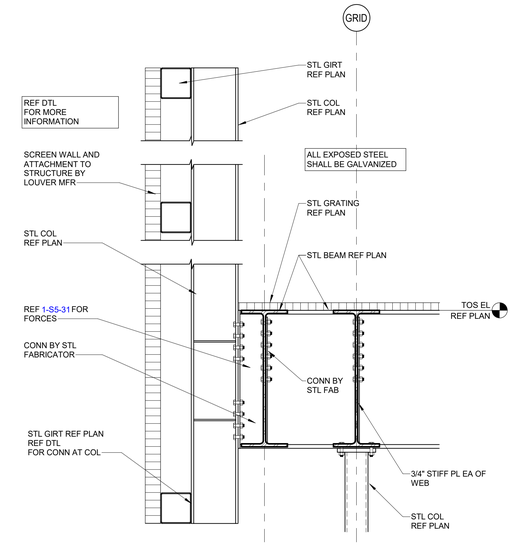


SECTION

6 FRAMING DETAIL
1" = 1'-0"



5 SCREEN WALL VERTICAL MEMBER CONNECTION AT BEAM
3/4" = 1'-0"



4 SCREEN WALL VERTICAL MEMBER CONNECTION AT BEAM
3/4" = 1'-0"

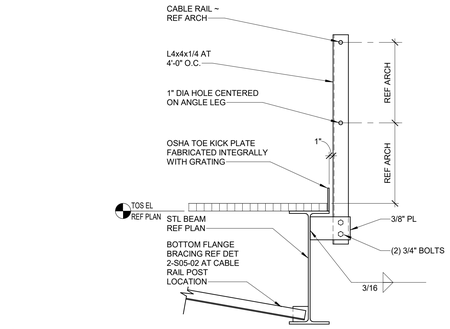
SECTION "A"

SECTION "B"

3 FRAMING DETAIL
3/4" = 1'-0"

2 SCREEN WALL VERTICAL MEMBER CONNECTION AT BEAM
3/4" = 1'-0"

1 SCREEN WALL VERTICAL MEMBER CONNECTION AT POST
3/4" = 1'-0"



7 FRAMING DETAIL
3/4" = 1'-0"

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AG&E Structural Engenuity

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1140 N. GUN CLUB ROAD

PLATFORM FRAMING SECTION AND DETAILS

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SHEET

S5-31

9/10/2025 5:08:29 PM S5-31 PLATFORM FRAMING SECTION AND DETAILS