City of Aurora
Building Codes Division
15151 E Alameda Pkwy
Aurora, CO 80012
(303) 739-7420

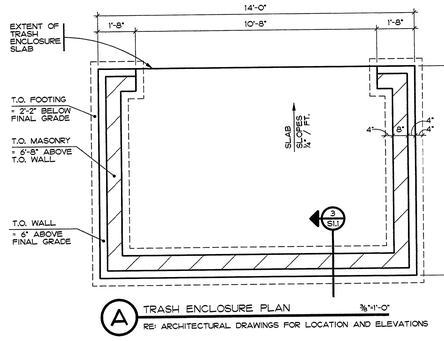
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GENERAL NOTES

1. Design Stenderds:
   A. International Building Code 2003 Edition
   B. ASCE 7-02 Minimum Design Loads for Buildings and Other Structures
   C. ACI 318-02 Building Code Requirements for Structural Concrete
   D. AISC Manual of Steel Construction LRFD 3rd Edition
   E. ACI 530-02 Building Code Requirements for Masonry Structures
2. Gravity Loads Used in Design:
   A. Roof Live Load (non-reducible)..... 30 psf
   B. Superimposed Roof Dead Load..... 32 psf
   C. Floor Live Load..... 80 psf
   D. Floor Dead Load (structure weight + superimposed dead load of 13 psf)..... 70 psf
   E. Partition Live Load..... 20 psf
   F. Lobby, Halls, Stairs Live Load..... 100 psf
   G. Roof top mechanical units..... 80 psf
   H. Snow Load Design Criteria
      1. Ground Snow Load = 30 psf
      2. Roof Snow Load = 30 psf + drifting requirements
      3. Snow Exposure Factor, Ce = 1.0
      4. Thermal Factor, Ct = 1.0
      5. Snow Importance Factor, I = 1.0
3. Wind Load Design Criteria
   A. Basic Wind Speed (3-second gust) = 105 mph
   B. Exposure Category B
   C. Building Category II
   D. Building Enclosure Classification: Enclosed
   E. Topographic Factor, Kzt = 1.0
   F. Analysis Method Used for MWFRS design:2-Analytical
   G. Analysis method used for components and cladding design: 2-Analytical
   H. Wind Importance Factor, I = 1.0
4. Seismic Load Design Criteria
   A. Seismic Design Category B
   B. Seismic Use Group I
   C. Soil Site Class D
   D. Basic Seismic Force Resisting System: Ordinary Braced Frames
   E. Response Modification Factor, R = 3.0
   F. System Overstrength Factor, Omega = 3.0
   G. Site Spectral Response Accelerations:
      1. Ss = 0.188
      2. S1 = 0.057
      3. SDs = 0.201
      4. SD1 = 0.091
   H. Seismic Importance Factor, I = 1.0
   I. Seismic Response Coefficient, Cs = 0.067
   J. Design Base Shear = 397 kips
5. Foundations:
   A. Soil data was taken from recommendations set forth in Report # 37,708 by CTL/Thompson, Inc. dated 11/3/03. See geotechnicol report for complete soils information.
   B. Maximum total load soil bearing pressure capacity used in design is 4,000 psf.
   C. Maximum total load soil bearing pressure capacity used in design for load combinations including wind or seismic forces is 4,000 psf.
   D. Equivalent fluid pressure used in design of retaining walls is 45 pcf.
6. Concrete:
   A. All concrete shall be made with stone aggregate and shall develop the following 28 day compressive strength:
      Footings, foundation walls..... 4000 psi
      Interior slabs on grade..... 3000 psi
      Exterior slabs-on-grade..... 4000 psi
      Slabs on metal deck..... 3500 psi
      Other concrete..... 3000 psi
   B. All reinforcing bars shall be ASTM A615, Grade 60, unless otherwise noted. Welded wire fabric shall conform to ASTM A185 or ASTM A497. Headed studs shall conform to ASTM A108 with 60 ksi minimum tensile strength and shall be automatically machine welded, providing complete fusion.
   C. Concrete Protection for Reinforcement (unless otherwise noted):
      1. Concrete poured against earth..... 3"
      2. Concrete poured in forms but exposed to weather or earth:
         a. #5 bars or smaller..... 1-1/2"
         b. Bars larger than #5..... 2"
      3. Slabs and walls not exposed to weather or earth..... 3/4"
   D. No splices of reinforcement shall be made and no welding to reinforcing shall be permitted except as detailed or authorized by the structural engineer. Reinforcing lap splices are to be a minimum of 36 bar diameters unless noted otherwise on drawings. Wire fabric reinforcement must lap one full mesh plus 2" at side and end laps, but not less than 6", and shall be wired together. Make all bars continuous at corners or provide corner bars of equal size and spacing.
   E. Detail bars in accordance with the ACI Detailing Manual and ACI Building Code Requirements for Reinforced Concrete.
   F. Provide all accessories necessary to support reinforcing at positions shown on the plans.
   G. Place 2-#5 bars (1 ea. face) with 2'-0" projection around all openings and re-entrant corners in concrete slabs and walls, unless noted otherwise.
   H. Footings, slabs, and stem walls shall not have joints in a horizontal plane. Any stop in concrete work must be made at center one-third of span with vertical bulkheeds and horizontal keyways, unless otherwise shown. All construction joints shall be as detailed or as approved by the Engineer.
   I. No chloride admixtures shall be added to concrete without Engineer's approval.
   J. Provide 6 x 6 - W1.4 x W1.4 WWF as minimum reinforcing in mechanical pads.
   K. Quote price separately for providing an allowance for additional reinforcing, fabrication and placing of 200 feet of #5 (Grade 60) reinforcing bar. Reinforcing shall be added on the shop drawing or placed in the field at the discretion of the Engineer.
   L. Concrete wall tolerances shall be as follows:
      Plumbness - entire height of wall..... H/500
      not to exceed..... 1-1/2"
      Plan Alignment..... 1/2" to +1/2"
      Wall thickness..... -1/4" to + 1/2"
      Variation in size & location of sleeve & blockouts..... -1/2" to +1/2"
   M. Allow for additional concrete thickness to compensate for structural member and formwork deflections.
   N. Slab-on-grade control joints are to be spaced a maximum of 15'-0" on center and are to coincide with column center lines and re-entrant corners.
7. Steel:
   A. All structural steel shall conform to ASTM A36 except wide flange shapes which shall conform to ASTM A992 (Grade 50), pipe sections which shall conform to ASTM A53 (Grade B), and HSS sections which shall conform to ASTM A500 (Grade B).
   B. Structural steel shall be detailed, fabricated and erected in accordance with the AISC "Manual of Steel Construction."
   C. Provide connections as detailed. Alternate connection details of equal capacity may be considered, provided they are submitted along with calculations. Alternate connection detail drawings and calculations which are submitted must be stamped and signed by a Colorado Registered Professional Engineer.
   D. All welders shall have evidence of passing the AWS standard qualification tests, and have c current certification.
   E. Minimum weld size shall not be less than 3/16" continuous fillet weld, unless noted otherwise on the plans.
   F. Connections made with high strength steel bolts shall conform in all respects to the AISC "Specifications for Structural Joints."
      1. All high strength steel bolts shall conform to ASTM A325 and shall be installed to a "snug tight" condition, unless noted otherwise.
      2. All high strength steel bolts indicated as "tension control" bolts shall conform to ASTM A490. Note that "tension control" bolts shall be used only at specific locations indicated on the drawings.
   G. Anchor rods are to be straight, headed, ASTM F1554 55ksi, unless noted otherwise.
   H. Steel joists shall be designed, fabricated, and erected in accordance with Steel Joist Institute Specifications. Where steel joists bear on structural steel framing, the joist(s) at or nearest each column on each side of the beam shall be bolted to the beam. Joist bridging shall conform to SJI specifications unless otherwise shown on plans.
   I. Install all required bridging and related miscellaneous steel prior to installing deck.
   J. Steel Deck:
      1. All steel deck shall be erected in accordance with the specifications of the Steel Deck Institute.
      2. Steel deck capacities are based upon published data by Vulcraft. Steel deck by other manufacturers may be supplied in lieu of that shown provided section properties are equal to or greater than those of deck specified, and if approved by the Engineer.
      3. Deck gages shown on the drawings are to be considered minimum. The deck supplier shall provide the required deck gage based upon the loadings shown above and the actual number of spans resulting from the final layout configuration.
      4. Contractor shall provide all additional framing as required for openings through deck larger than 6".
      5. Steel deck shall be fastened to supports to attain the minimum diaphragm shear (plf) as indicated on the metal deck schedule on sheet S1.1.
      6. All structural framing necessary to support deck edges not shown shall be L3x3x1/4.
      7. Deck supplier shall provide metal closures as required to form slab edges, unless noted otherwise.
   K. Cold Formed Steel Framing:
      1. See Cold Formed Steel General Notes on sheet S6.1
   L. Fireproofing considerations (Re: Architectural plans for building classification):
      1. All columns, beams or bracing connected to columns are to be considered as primary framing members.
      2. All horizontal framing members are to be considered restrained or unrestrained in accordance with ASTM E119, Table X3.1.
8. Masonry:
   A. All masonry block units shall be lightweight and shall conform to ASTM C90. Masonry block units below grade and all exterior walls shall be grade N-1. Interior masonry block walls not exposed to weather or earth shall be grade N-1 or S-1.
   B. All mortar for exterior walls and interior bearing walls shall conform to ASTM C270, Type S, 1800 psi compressive strength, mixed in proportions of 1: 1/2: 4-1/2. Grout for masonry bond beams, walls, and pilasters shall be made with pea gravel stone aggregate and shall develop 3000 psi compressive strength in 28 days, mixed in proportions of 1:3:2 (Portland Cement:Sand:Pea gravel). Alternate mix proportions may be considered, provided they are submitted for review.
   C. All masonry assemblies shall develop 1500 psi ultimate compressive strength (f 'm) in 28 days.
   D. Horizontal reinforcing in all masonry walls shall be standard Dur-O-Wal or equal spaced at 16" on center, unless otherwise noted. Terminate joint reinforcing at control joints and expansion joints.
   E. Vertical reinforcing shown on the drawings shall extend for the full height of the wall and shall be grouted in place, using a maximum of 4'-0" lifts. Provide one additional bar at all corners, end of wall, each side of openings, and each side of control joint. High lift grouting may be done up to a height of 15'-0" as long as cleanouts are provided at the base of each grouted cell. All cells must be cleaned prior to grouting, and all vertical reinforcing must be adequately secured in position.
   F. Fill all voids and block cells solidly with grout for a distance of 24" beneath and 16" each side of all beam reactions or other concentrated loads unless otherwise noted.
   G. Provide horizontal bond beam with a minimum of 2-#5's x continuous at the top of all structural walls, at all floor and roof levels, and at levels called for on the plans. Terminate bond beam reinforcing at expansion joints. Top of wall bond beam reinforcing must be continuous at control joints.
   H. Cells at portions of exterior masonry walls extending below grade shall be grouted solid.
9. All dimensions on structural drawings shall be checked against architectural.
10. Engineer's approval must be secured for all substitutions.
11. Verify all openings through floor, roof and walls with mechanical and electrical contractors.
12. During erection of the building, the contractor shall be responsible for temporary bracing to withstand all loads to which the structure may be subjected, including lateral loads, stockpiles of materials and equipment. Such bracing shall be left in place as long as may be required for safety and until all structural framing and diaphragms are in place with connections completed.
13. Do not reproduce contract documents, copy standard printed information, or use electronic CAD files as the basis for shop drawings.
14. Special inspection shall be in accordance with Chapter 17 of the Building Code for the following:
   A. Structural steel construction, Table 1704.3.
   B. All shop and field welding.
   C. Structural steel details.
   D. High-strength steel bolted connections.
   E. Structural concrete construction, Table 1704.4.
   F. Masonry construction, Table 1704.5.
   G. Soils.
   H. Sprayed fire-resistant materials.
15. General contractor is responsible for supplying sill angles, machine room beams, and divider beams. Supply and installation of additional required supports and brackets not shown on drawings are the responsibility of the elevator supplier.
16. Significant permanent equipment sizes, weights, and locations are indicated on the drawings as provided to the engineer during design. Changes in sizes, weights, or locations must be submitted in writing for review by the Structural Engineer. Required supports or braces not shown on the drawings are the responsibility of the equipment supplier.
17. All steel or steel connections permanently exposed below grade shall be coated with c zinc rich paint or an asphaltic mastic.
18. Conduit to be buried in elevated slabs shall have an outside diameter not greater than 1" and shall be spaced no closer than 1'-6" o.c. Conduit buried in topping slabs on metal decks may be placed directly on the deck surface.
19. Explanation of section designated used:

SECTION IDENTIFICATION — XX / SXX — SHEET LOCATION OF DETAILS



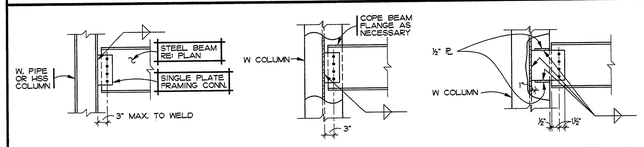
A TRASH ENCLOSURE PLAN 3/8"=1'-0"
RE: ARCHITECTURAL DRAWINGS FOR LOCATION AND ELEVATIONS

STEEL BEAM CONNECTION SCHEDULE (LRFD)

BEAM SIZE	NUMBER OF BOLTS	SHEAR TAB THK'L'LENGTH	FILLET WELD SIZE	CONNECTION CAPACITY - ΦRn (KIPS) UNCOPED	SINGLE COPE 1	SINGLE COPE 2	DOUBLE COPED	REMARKS
W8	2	5/16" × 6"	1/4"	19.0	11.4	-	4.7	-
W10	2	5/16" × 6"	1/4"	19.0	19.0	-	10.3	-
W12	3	5/16" × 9"	1/4"	37.9	25.5	-	18.3	-
W14	3	5/16" × 9"	1/4"	37.9	37.9	37.4	30.4	-
W16	4	5/16" × 12"	1/4"	60.8	53.5	59.8	46.5	-
W18	4	5/16" × 12"	1/4"	60.8	60.8	60.8	60.8	- -
W21	5	3/8" × 15"	5/16"	84.4	84.4	-	84.4	-
W24	6	3/8" × 18"	5/16"	108.0	108.0	-	108.0	-
W27	7	3/8" × 21"	5/16"	131.0	131.0	-	131.0	-
-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-

NOTES:
1. CAPACITIES SHOWN ARE BASED ON THE FOLLOWING PARAMETERS:
   -MINIMUM YIELD STRESS OF 36 KSI FOR ALL CONNECTED PARTS.
   -MINIMUM EDGE DISTANCE= 1½" FOR ALL CONNECTED PARTS.
   -BOLT SPACING = 3"
   -STANDARD HOLES IN SUPPORTED WEB.
   -HORIZ. SHORT SLOTTED HOLES IN SHEAR PLATES.
   -E70XX ELECTRODES.
2. ALL BOLTS ARE ⅞"Φ A325-N, EXCEPT DESIGNATED "SC" CONNECTIONS ON PLANS ARE ⅞"Φ A490-SC.
3. A325-N BOLTS ARE FULLY THREADED BEARING TYPE CONNECTIONS AND ARE TO BE "SNUG-TIGHT" AS DEFINED IN THE AISC MANUAL OF STEEL CONSTRUCTION LOAD AND RESISTANCE FACTOR DESIGN, 3RD EDITION), U.N.O.

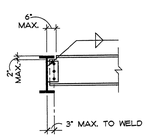
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TYPICAL BEAM TO COLUMN CONNECTIONS

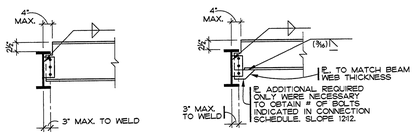
ALTERNATE BEAM TO COLUMN WEB CONNECTION

SINGLE COPE 1



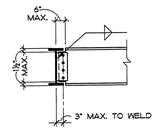
TYPICAL BEAM TO TO BEAM CONNECTIONS

SINGLE COPE 2



TYPICAL BEAM TO BEAM CONNECTIONS

DOUBLE COPE



TYPICAL BEAM TO TO BEAM CONNECTIONS

FOOTING SCHEDULE

MARK	FOOTING SIZE	REINFORCING	REMARKS
A	5'-0"×5'-0"×2'-0"	6-#6 BARS EACH WAY BOTTOM	-
B	8'-0"×8'-0"×2'-0"	10-#6 BARS EACH WAY BOTTOM	-
C	9'-0"×9'-0"×2'-0"	10-#8 BARS EACH WAY BOTTOM	-
D	10'-0"×10'-0"×2'-0"	11-#8 BARS EACH WAY TOP AND BOTTOM	BRACED FRAME LOCATION RE: PLAN
E	9'-6"×9'-6"×2'-0"	10-#8 BARS EACH WAY TOP AND BOTTOM	BRACED FRAME LOCATION RE: PLAN
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

NOTES:
1. CENTER ALL PAD FOOTING ON GRIDLINES UNLESS NOTED OTHERWISE.
2. EQUALLY SPACE REINFORCING BARS WITHIN FOOTING UNLESS NOTED OTHERWISE.

COLUMN SCHEDULE

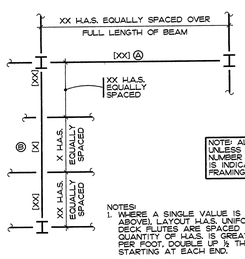
MARK	COLUMN SIZE	BASE PLATE T×B×N	REMARKS
1	W12×40	1⅛"×14"×1'-2"	4 - 1" ø ANCHOR RODS RE: PLAN DETAILS ON S1.1
2	W12×45	1⅛"×14"×1'-2"	4 - 1" ø ANCHOR RODS RE: PLAN DETAILS ON S1.1
3	W12×53	1⅛"×16"×1'-4"	4 - 1" ø ANCHOR RODS RE: PLAN DETAILS ON S1.1
4	8" ø EXTRA STRONG	1"×15"×1'-3"	4 - 1" ø ANCHOR RODS RE: PLAN DETAILS ON S1.1
5	6" ø STD.	1"×14"×1'-2"	4 - 1" ø ANCHOR RODS RE: PLAN DETAILS ON S1.1
6	W12×53	RE: BRACED FRAME DETAILS ON S1.1	RE: BRACED FRAME DETAILS ON S1.1
7	W12×50	RE: BRACED FRAME DETAILS ON S1.1	RE: BRACED FRAME DETAILS ON S1.1
8	W12×58	RE: BRACED FRAME DETAILS ON S1.1	RE: BRACED FRAME DETAILS ON S1.1
9	HSS6×6×¼	-	RE: DETAIL 11/S1.1
10	HSS4×4×⅜	-	RE: DETAIL 13/S1.1
11	HSS6×6×⅜	1"×13"×1'-1"	4 - 1" ø ANCHOR RODS RE: PLAN DETAILS ON S1.1

NOTES:
1. USE TEMPLATES TO SET ANCHOR RODS IN CONCRETE.
2. ANCHOR RODS ARE TO BE 1" ø WITH 1'-6" MINIMUM EMBEDMENT, UNLESS NOTED OTHERWISE.
3. COLUMN CAP PLATES ARE TO BE ½" THICK UNLESS NOTED.

STEEL LINTEL SCHEDULE

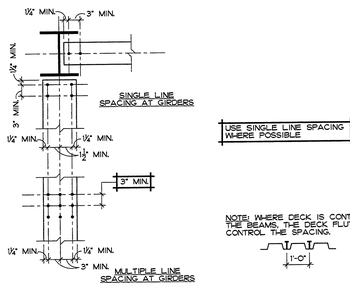
OPENING SIZE	LINTEL	BEARING EACH END	REMARKS
5'-6" OR LESS	L3½×3½×⅜	6"	-
OVER 5'-6" THRU 6'-3"	L4×3½×⅜ LLV	6"	-

NOTES:
1. PROVIDE ONE STEEL ANGLE FOR EACH 4" WIDTH OR LESS OF MASONRY.
2. THIS SCHEDULE IS TO BE USED FOR NON-LOAD BEARING WALL PUNCHED WINDOWS ONLY. RE: ARCH. FOR SPECIFIC LOCATIONS.
3. RE: STRUCTURAL DETAILS FOR CONT. RELIEF ANGLES AT THE EXTERIOR 2ND AND 3RD FLOOR WINDOW HEADS.



NOTES:
1. WHERE A SINGLE VALUE IS GIVEN (E.G. Ⓐ ABOVE), LAYOUT H.A.S. UNIFORMLY. WHERE DECK FLUTES ARE SPACED 12" O.C. AND THE QUANTITY OF H.A.S. IS GREATER THAN ONE PER FOOT, DOUBLE UP ½ THE DIFFERENCE STARTING AT EACH END.
2. MULTIPLE VALUES (E.G. Ⓑ ABOVE) INDICATES SPACING IN EACH AREA BETWEEN CONCENTRATED LOADS.

1 TYP. HEADED ANCHOR STUD (H.A.S.) LAYOUT NOT TO SCALE



2 HEADED ANCHOR STUD (H.A.S.) SPACING NOT TO SCALE

STRUCTURAL DRAWING INDEX

S1.0	GENERAL NOTES, SCHEDULES AND PLAN
S1.1	SCHEDULES AND DETAILS
S2.1	FOUNDATION PLAN
S2.2	SECOND FLOOR FRAMING PLAN
S2.3	THIRD FLOOR FRAMING PLAN
S2.4	ROOF FRAMING PLAN
S4.1	FOUNDATION DETAILS
S5.1	FRAMING DETAILS
S5.2	BRACED FRAME ELEVATIONS AND DETAILS
S6.1	COLD FORMED FRAMING DETAILS AND GENERAL NOTES

STRUCTURAL PLAN REVIEW IS LIMITED TO A GENERAL SURVEY FOR CODE COMPLIANCE. NO REVIEW IS IMPLIED NOR WAS UNDERTAKEN TO VERIFY STRUCTURAL ADEQUACY.

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 11/20/04

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MedCap PROPERTIES

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SCI Job Number: 02-333

ISSUE/REVISION	DATE	NO.
DESIGN DEVELOPMENT	11/17/03	
50% CD'S	9/21/04	
95% CD'S	10/8/04	
100% CD'S	10/26/04	

COLORADO REGISTERED PROFESSIONAL ENGINEER 36269 10/26/04

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

Health ONE

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

GENERAL NOTES, DETAILS, SCHEDULES AND PLAN

DATE	10/26/04
DRAWN BY	JEM
CHK'D BY	FEB
X-REF FILE	02-333_S10

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SHEET NUMBER:

S1.0

D.P.A. PROJECT # 02419.00

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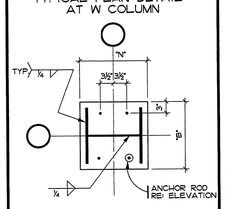
METAL DECK SCHEDULE

DECK TYPE	REQUIRED SHEAR CAPACITY	I	Sp	Sn	Fy
1.5B18	865 PLF	0.292 IN^4/FT.	0.318 IN^3/FT.	0.327 IN^3/FT.	33 KSI
2.0VLI 20	1230 PLF	I_P 0.418 IN^4/FT. I_N 0.415 IN^4/FT.	0.355 IN^3/FT.	0.360 IN^3/FT.	40 KSI

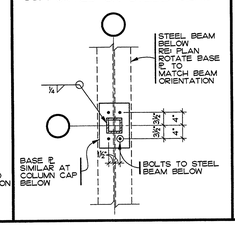
NOTES:
1. RE: PLANS FOR DECK TYPE LOCATIONS.
2. PROPERTIES INDICATED ARE MINIMUM.
3. RE: SPECIFICATIONS FOR METAL DECK FINISHES.
4. DECK SHALL BE INSTALLED OVER 4 SUPPORTS (3 SPAN CONT.) EXCEPT AS NOTED ON PLANS.
5. POUR STOPS AND SUPPORTS AT COLUMNS ARE TO BE PROVIDED BY THE DECK SUPPLIER.
6. SEE SPECIFICATION FOR DECK FASTENING CRITERIA.

BASE PLATE DETAILS

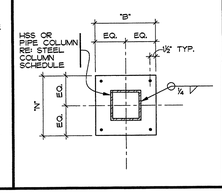
TYPICAL PLAN DETAIL AT W COLUMN



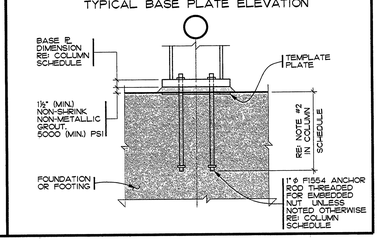
PLAN DETAIL AT COLUMN SUPPORTED ON STEEL BEAM



TYPICAL PLAN DETAIL AT HSS COLUMN OR PIPE COLUMN

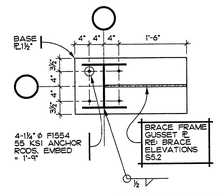


TYPICAL BASE PLATE ELEVATION

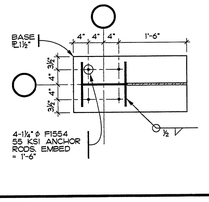


STEEL COLUMN BASE PLATE DETAILS AT BRACED FRAMES

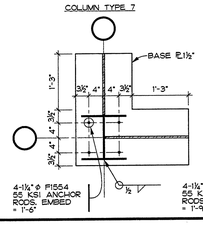
COLUMN TYPE 6 AND 8



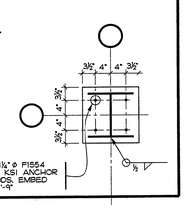
COLUMN TYPE 7

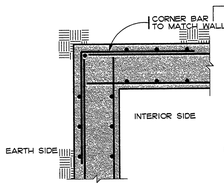


COLUMN TYPE 7

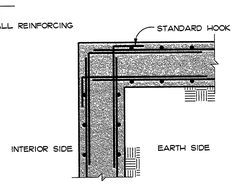


COLUMN TYPE 6 AT UNBRACED SIDE

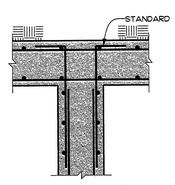




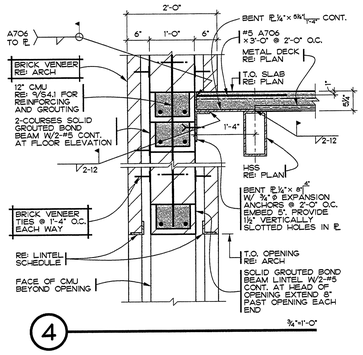
OUTSIDE CORNER



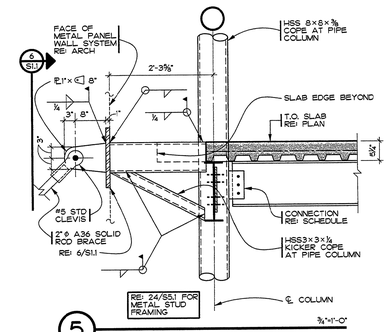
INSIDE CORNER



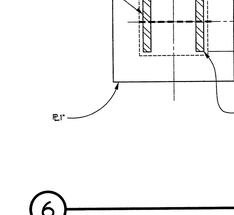
INTERSECTING WALLS



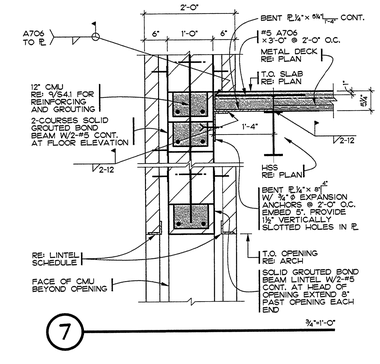
4 ¾"=1'-0"



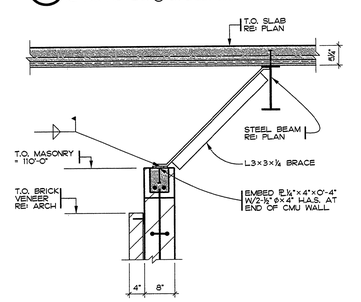
5 ¾"=1'-0"



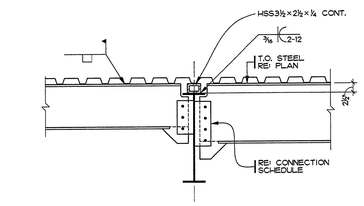
6 1½"=1'-0"



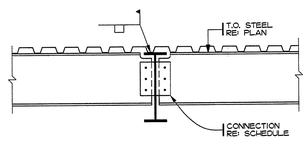
7 ¾"=1'-0"



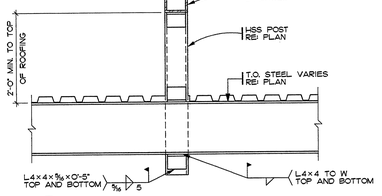
8 ¾"=1'-0"



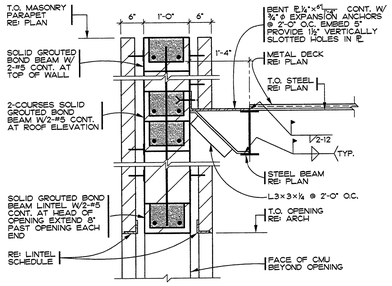
9 ¾"=1'-0"



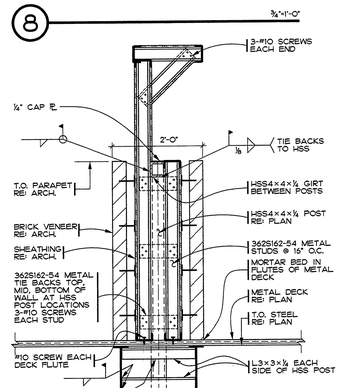
10 ¾"=1'-0"



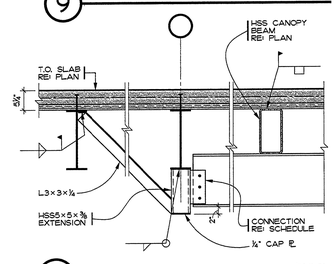
11 ¾"=1'-0"



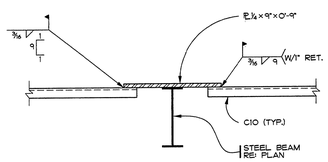
12 ¾"=1'-0"



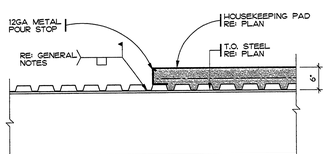
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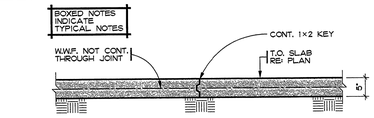
14 ¾"=1'-0"



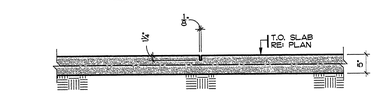
15 ¾"=1'-0"



16 ¾"=1'-0"



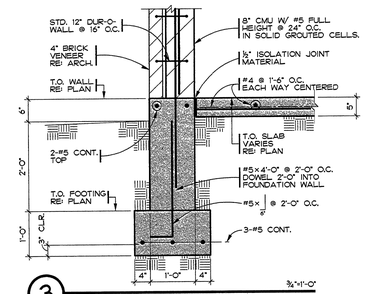
TYPICAL CONSTRUCTION JOINT



NOTES:
1. JOINTS MUST BE TOOLED OR SAW CUT AS SOON AS THE SLAB CAN SUPPORT EQUIPMENT.
2. LOCATE AT COLUMN LINES AND MAXIMUM SPACING OF APPROXIMATELY 15'-0" EACH DIRECTION AND AT RE-ENTRANT CORNERS.

TYPICAL CONTROL JOINT

1 ¾"=1'-0"



3 ¾"=1'-0"
RE: PARTIAL PLAN Ⓐ ON SHEET S1.0

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 1/20/05

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(303) 399-5154, FAX (303) 332-9501
SCI Job Number: 02-333

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COLORADO REGISTERED PROFESSIONAL ENGINEER 36269 STRUCTURAL

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

Health ONE

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

SCHEDULES AND DETAILS

DATE	10/26/04
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CHK'D BY	FEB
X-REF FILE	02-333_S11

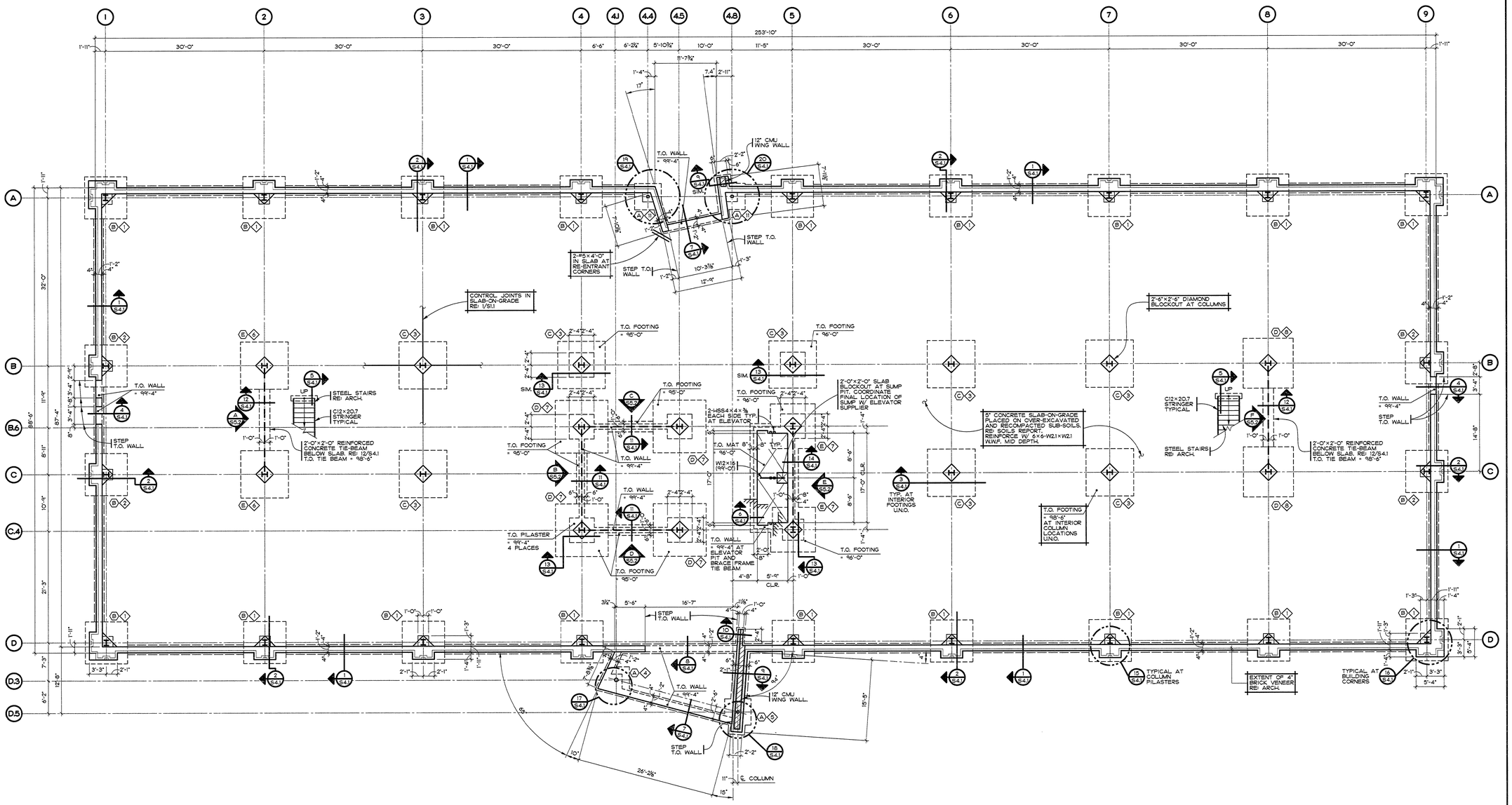
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S1.1

D.P.A. PROJECT # 02419.00

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FOUNDATION / FIRST FLOOR PLAN

3/32"=1'-0"

NOTES:

1. BOXED NOTES ═╪═ INDICATE A TYPICAL CONDITION UNLESS NOTED OTHERWISE.
2. TOP OF SLAB ELEVATION = 100'-0", UNLESS NOTED THUS: ◇ XXX'-XX". ELEVATION 100'-0" = ELEVATION 5598.5' U.S.G.S.
3. TOP OF FOOTING ELEVATION = 97'-6", UNLESS NOTED THUS: T.OF. = XXX'-XX" .
4. ⓧ INDICATES FOOTING TYPE, REF: SCHEDULE ON SHEET S1.0.
5. ◇ INDICATES STEEL COLUMN TYPE, REF: SCHEDULE ON SHEET S1.0.
6. TOP OF PILASTER ELEVATION = 99'-4", UNLESS NOTED OTHERWISE THUS: T.OP. = XXX'-XX" .
7. TOP OF WALL ELEVATION = 100'-0", UNLESS NOTED OTHERWISE THUS: T.OW. = XXX'-XX" .
8. FOR TYPICAL SLAB-ON-GRADE CONSTRUCTION AND CONTROL JOINTS, REF: 1/S1.1.
9. CENTER ALL COLUMNS AND FOOTINGS ON GRID LINES UNLESS NOTED OTHERWISE.
10. CENTER ALL STRIP FOOTINGS UNDER WALLS UNLESS NOTED OTHERWISE.
11. ═ ═ INDICATES BRACED FRAME LOCATION, REF: SHEET S5.2 FOR ELEVATIONS.
12. REF: 2/S11 FOR REINFORCING CONFIGURATION AT WALL INTERSECTIONS.
13. REF: ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR REQUIRED SLEEVES OR BLOCKOUTS IN FOUNDATION WALLS AND FLOOR SLABS.
14. COORDINATE FINAL ELEVATOR PIT DIMENSIONS, LOCATION, AND BLOCKOUT FOR PISTON WITH ELEVATOR SUPPLIER.

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 12/20/04

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SCI Job Number: 02-333

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COLORADO REGISTERED 36269 PROFESSIONAL ENGINEER 12/20/04

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

FOUNDATION / FIRST FLOOR PLAN

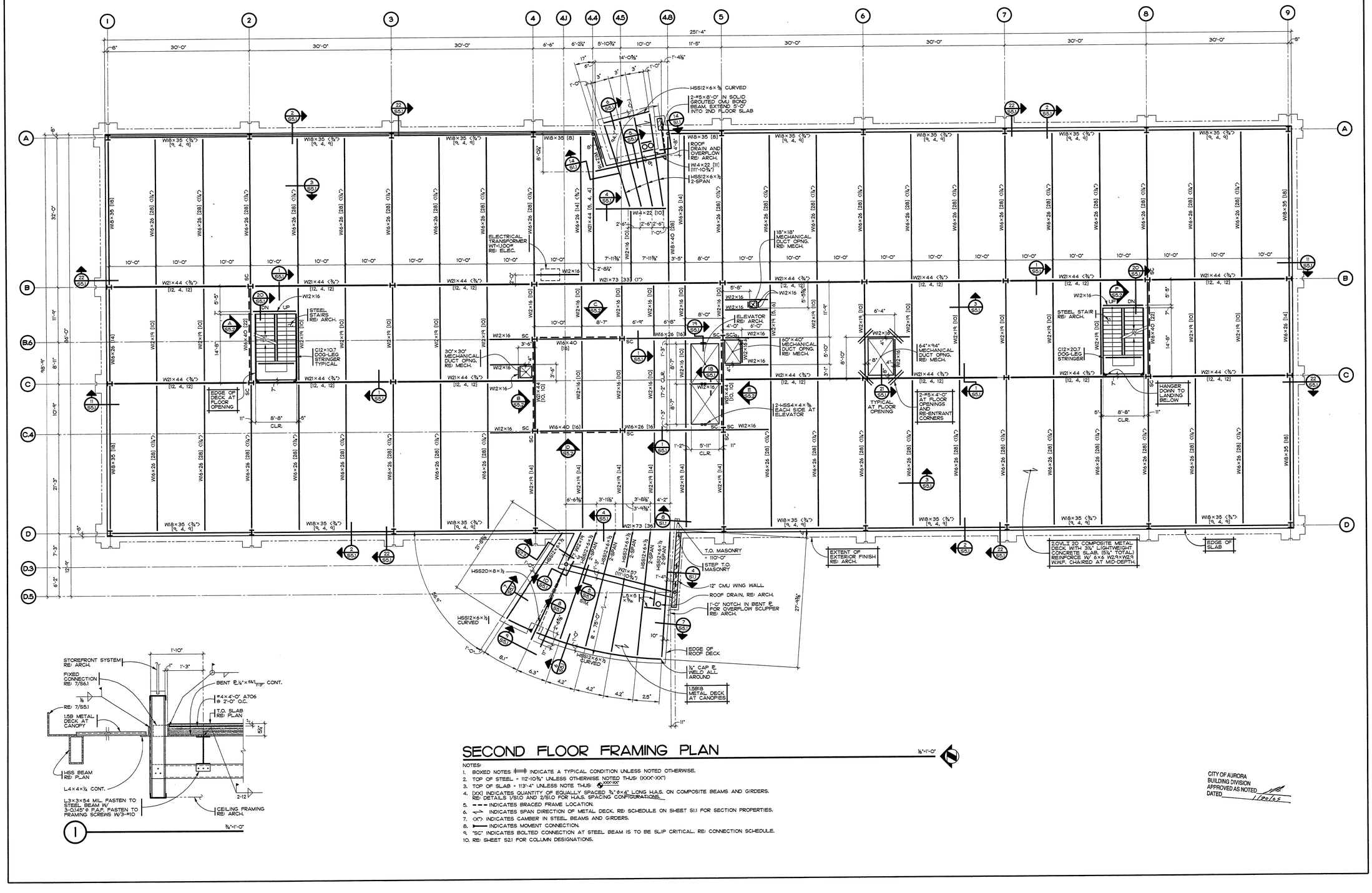
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D.P.A. PROJECT # 02419.00



SECOND FLOOR FRAMING PLAN

⅛"=1'-0"

NOTES:
1. BOXED NOTES ⫴ INDICATE A TYPICAL CONDITION UNLESS NOTED OTHERWISE.
2. TOP OF STEEL = 112'-10⅞" UNLESS OTHERWISE NOTED THUS: (XXX'-XX")
3. TOP OF SLAB = 113'-4" UNLESS NOTE THUS: ◇XXX'-XX"
4. [XX] INDICATES QUANTITY OF EQUALLY SPACED ⅞"Ø×4" LONG H.A.S. ON COMPOSITE BEAMS AND GIRDERS. REF DETAILS 1/S1.0 AND 2/S1.0 FOR H.A.S. SPACING CONFIGURATIONS.
5. — — — INDICATES BRACED FRAME LOCATION.
6. ⌁ INDICATES SPAN DIRECTION OF METAL DECK. REF SCHEDULE ON SHEET S1.1 FOR SECTION PROPERTIES.
7. ⟨X"⟩ INDICATES CAMBER IN STEEL BEAMS AND GIRDERS.
8. ▶— INDICATES MOMENT CONNECTION.
9. "SC" INDICATES BOLTED CONNECTION AT STEEL BEAM IS TO BE SLIP CRITICAL. REF CONNECTION SCHEDULE.
10. REF SHEET S2.1 FOR COLUMN DESIGNATIONS.

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 11/08/04

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100% CD'S	10/26/04	

COLORADO REGISTERED 36289 PROFESSIONAL ENGINEER

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

HealthONE

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

SECOND FLOOR FRAMING PLAN

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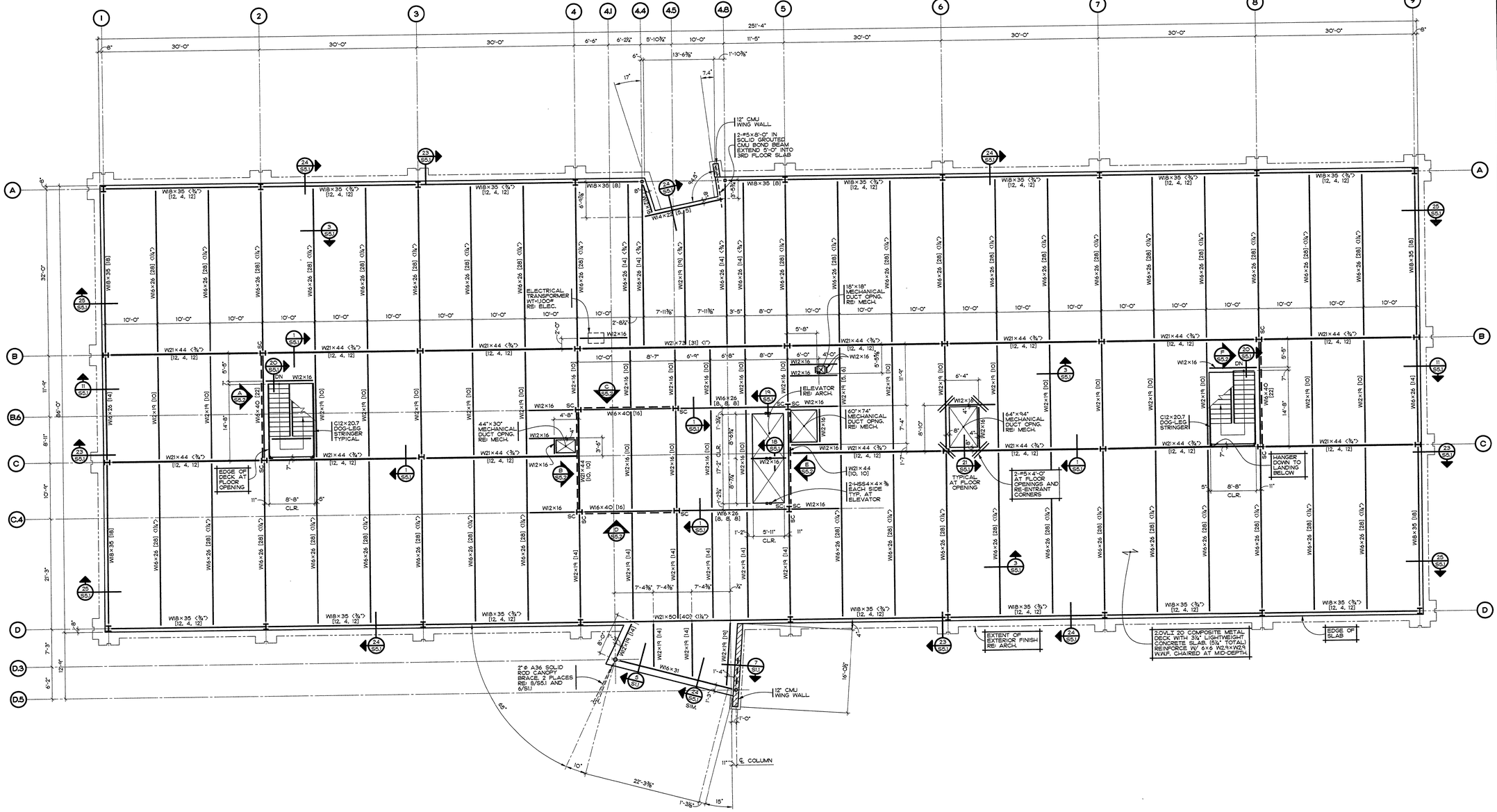
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S2.2

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THIRD FLOOR FRAMING PLAN

1/8"=1'-0"

NOTES:

1. BOXED NOTES ⧦ INDICATE A TYPICAL CONDITION UNLESS NOTED OTHERWISE.
2. TOP OF STEEL = 126'-2⅜" UNLESS OTHERWISE NOTED THUS: (XXX'-XX")
3. TOP OF SLAB = 126'-8" UNLESS NOTE THUS: ◇ XXX'-XX"
4. [XX] INDICATES QUANTITY OF EQUALLY SPACED ¾"ϕ x 4" LONG H.A.S. ON COMPOSITE BEAMS AND GIRDERS. RE: DETAILS 1/S1.0 AND 2/S1.0 FOR H.A.S. SPACING CONFIGURATIONS.
5. = = = INDICATES BRACED FRAME LOCATION.
6. → INDICATES SPAN DIRECTION OF METAL DECK. RE: SCHEDULE ON SHEET S1.1 FOR SECTION PROPERTIES.
7. (X") INDICATES CAMBER IN STEEL BEAMS AND GIRDERS.
8. "SC" INDICATES BOLTED CONNECTION AT STEEL BEAM IS TO BE SLIP CRITICAL. RE: CONNECTION SCHEDULE.
9. RE: SHEET S2.1 FOR COLUMN DESIGNATIONS.

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 11/26/04

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SCI Job Number: 02-333

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COLORADO REGISTERED 36269 PROFESSIONAL ENGINEER 11/26/04

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

THIRD FLOOR FRAMING PLAN

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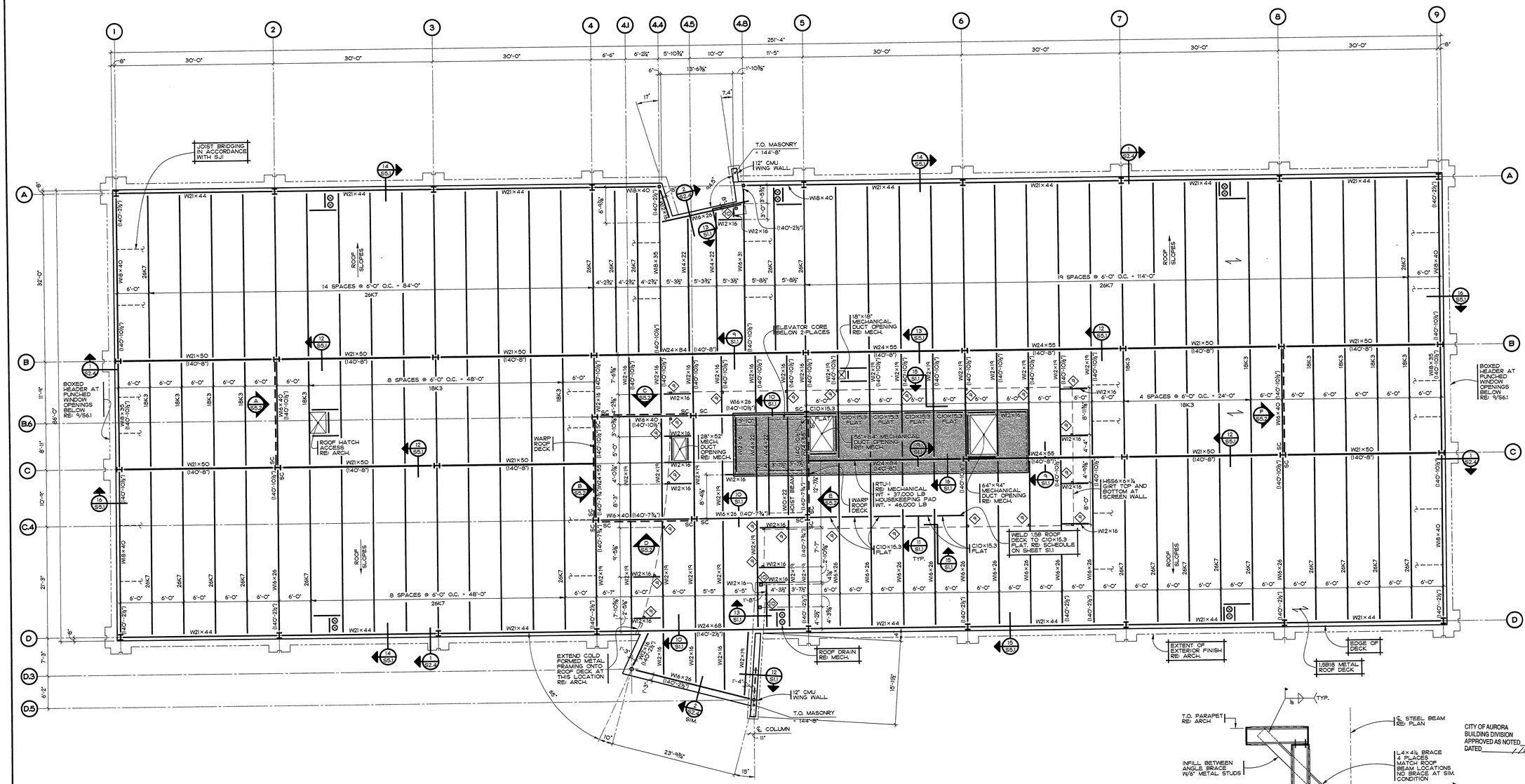
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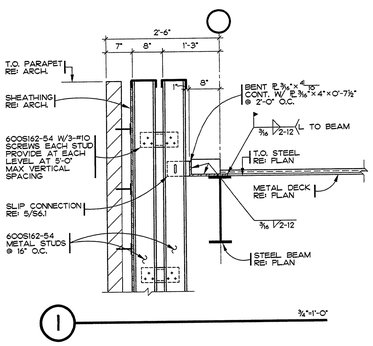
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S2.3

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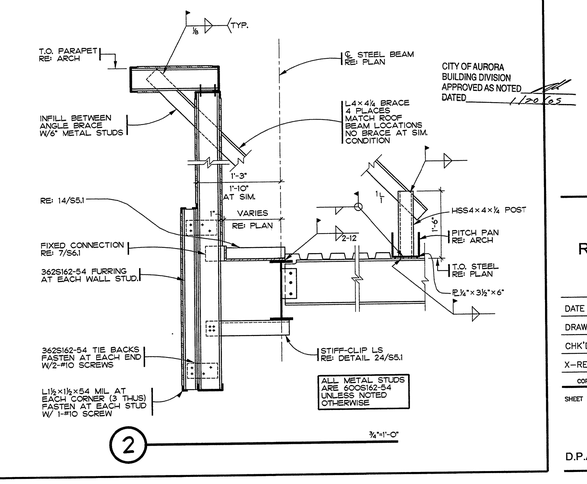


1 ⅜"=1'-0"

ROOF FRAMING PLAN

1/8"=1'-0"

NOTES:
1. BOXED NOTES ═╪═ INDICATE A TYPICAL CONDITION UNLESS NOTED OTHERWISE.
2. TOP OF STEEL = 140'-0" UNLESS OTHERWISE NOTED THUS: (XXX'-XX")
3. - - - INDICATES BRACED FRAME LOCATION.
4. ⇄ INDICATES SPAN DIRECTION OF METAL DECK. REI SCHEDULE ON SHEET S1.1 FOR SECTION PROPERTIES.
5. ▭ INDICATES 4" CONSTANT THICKNESS CONCRETE HOUSEKEEPING PAD AT ROOF TOP MECHANICAL UNIT. COORDINATE SIZE AND LOCATION OF PAD WITH UNIT SELECTED.
6. REI DETAIL 17/S5.1 FOR FRAMING AT MECHANICAL OPENINGS.
7. REI SHEET S2.1 FOR COLUMN DESIGNATIONS.



2 ⅜"=1'-0"

CITY OF AURORA BUILDING DIVISION APPROVED AS NOTED DATED 11/20/05

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SCI Job Number: 02-333

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95% CD'S	10/8/04	
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COLORADO REGISTERED 36269 JAMES E. MUMMERT PROFESSIONAL ENGINEER 10/26/04

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

Health ONE

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

ROOF FRAMING PLAN

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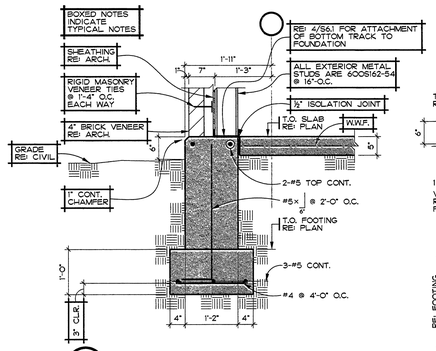
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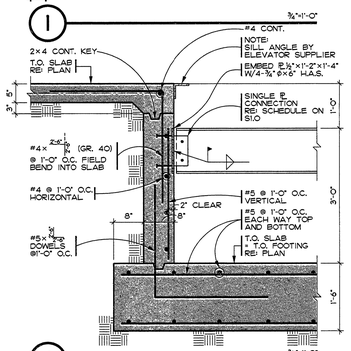
S2.4

D.P.A. PROJECT # 02419.00

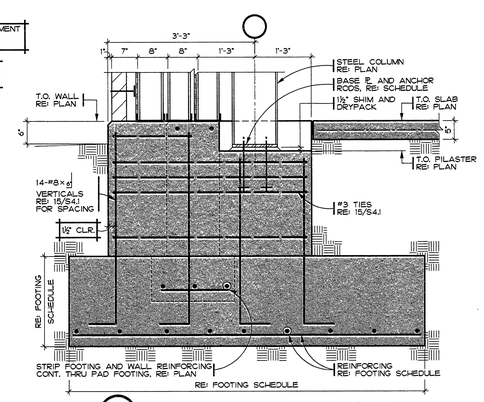
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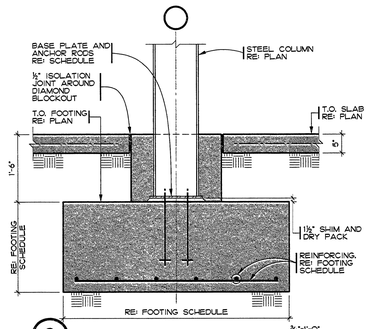
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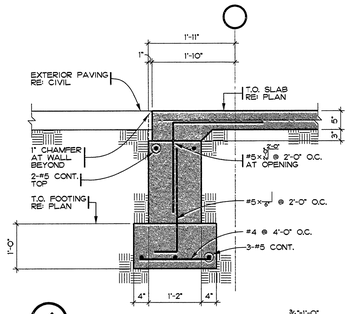
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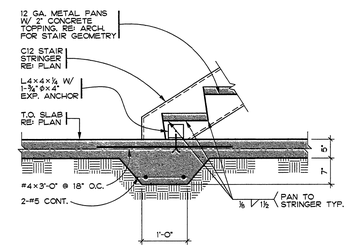
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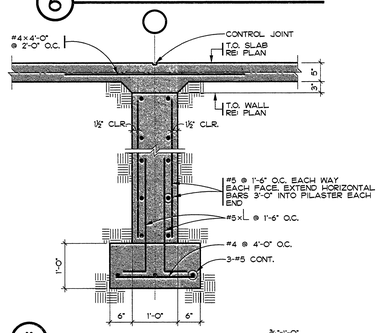
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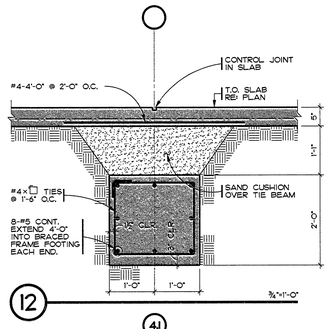
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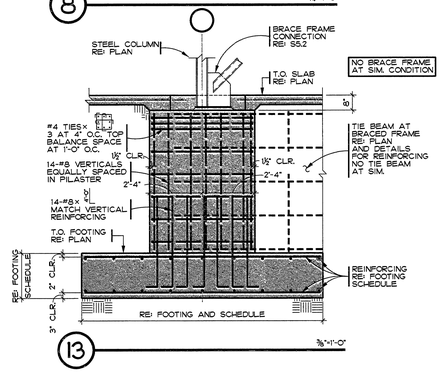
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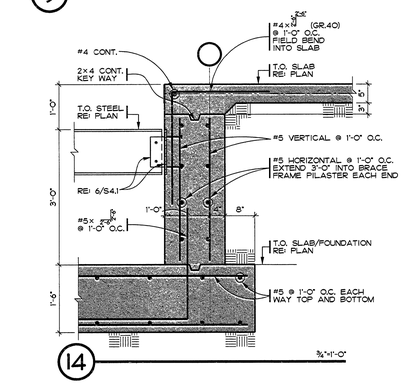
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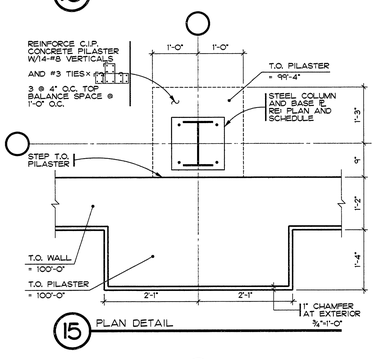
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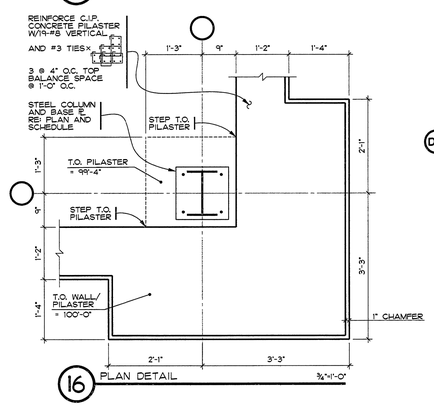
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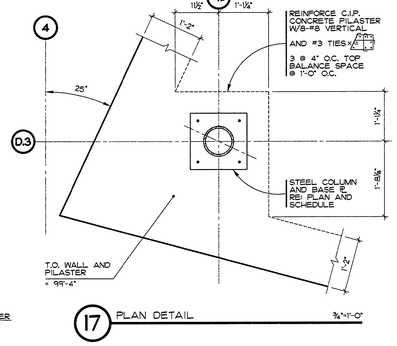
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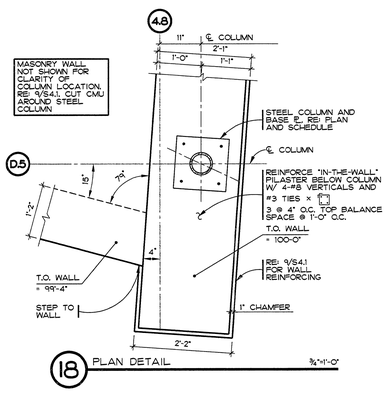
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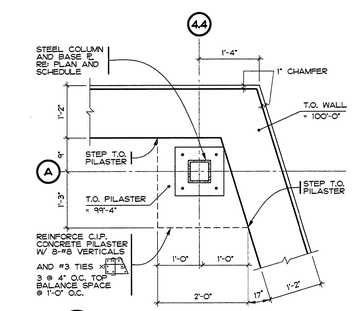
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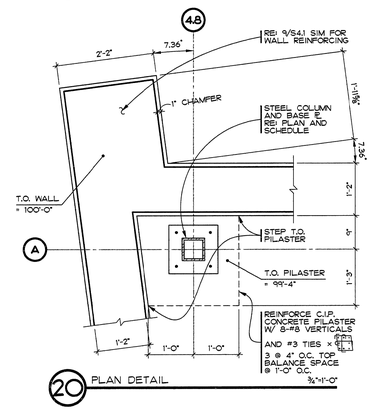
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18 PLAN DETAIL ¾"=1'-0"



19 PLAN DETAIL ¾"=1'-0"



20 PLAN DETAIL ¾"=1'-0"

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 12/20/04

Davis Partnership Architects
2301 BLAKE ST., STE. #100
DENVER, COLORADO 80205
303.861.8565 F.303.861.3027
0137 MAIN STREET, UNIT C106
POST OFFICE BOX 2711
EDWARDS, COLORADO 81632
970.926.8960 F.970.926.8961

weyland enterprises, inc.

MedCap PROPERTIES

Structural Consultants Incorporated
3400 E. Bayaud Avenue #300
Denver, Colorado 80209
(303) 399-5154, FAX (303) 333-9501
SCI Job Number: 02-333

ISSUE/REVISION	DATE	NO.
DESIGN DEVELOPMENT	11/17/03	
50% CD'S	9/21/04	
95% CD'S	10/8/04	
100% CD'S	10/26/04	

COLORADO REGISTERED PROFESSIONAL ENGINEER 36269 JOHN E. MCKINNEY 12/20/04

MEDICAL OFFICE BUILDING
MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

HealthONE

CONSTRUCTION DOCUMENTS

FOUNDATION DETAILS AND SECTIONS

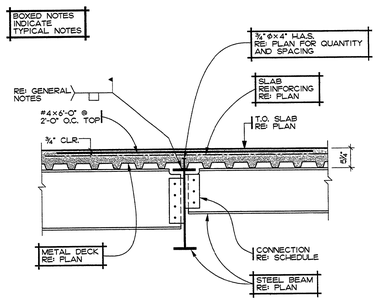
DATE	10/26/04
DRAWN BY	JEM
CHK'D BY	FEB
X-REF FILE	02-333_S41

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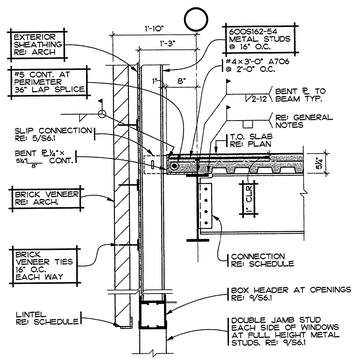
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S4.1

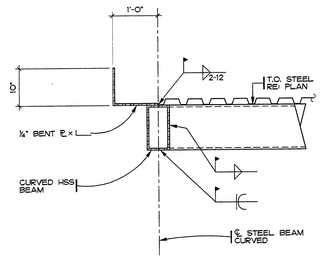
D.P.A. PROJECT # 02419.00



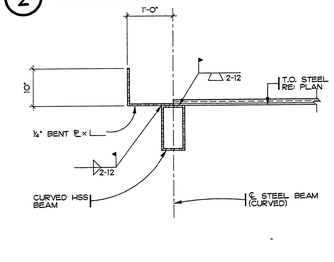
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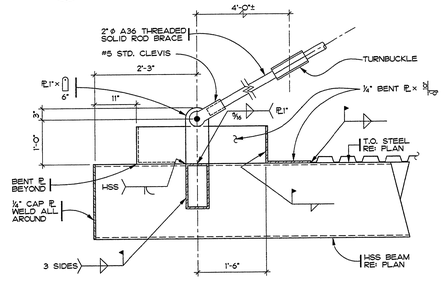
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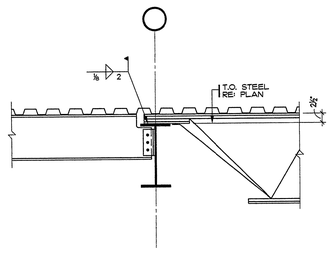
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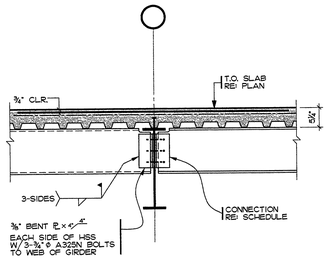
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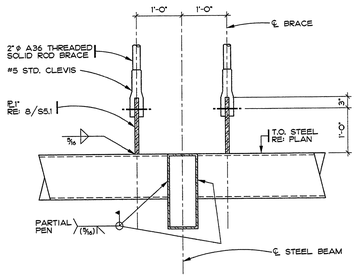
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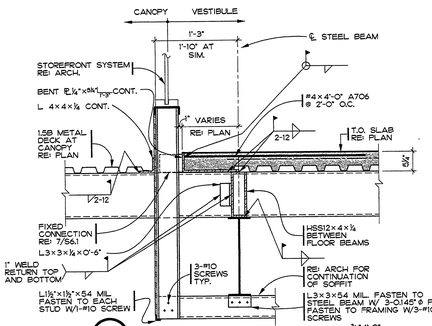
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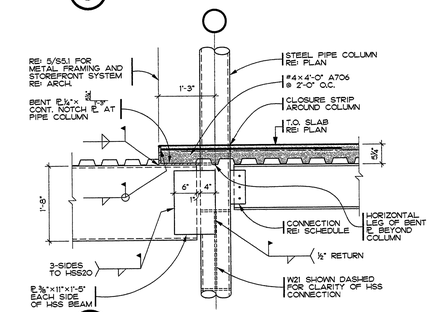
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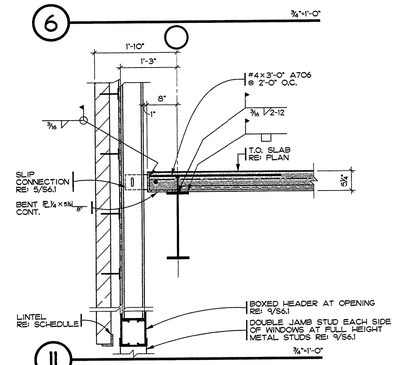
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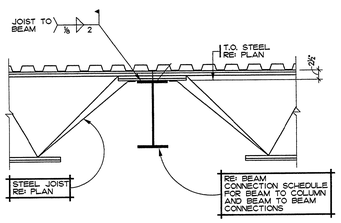
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10



11 3/4"=1'-0"



12 3/4"=1'-0"

13 3/4"=1'-0"

14 3/4"=1'-0"

15 3/4"=1'-0"

16 3/4"=1'-0"

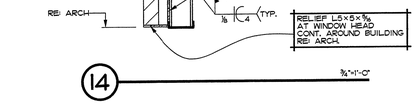


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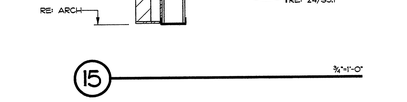


CONNECTION OF ELEVATOR RAIL SUPPORT TO BEAM

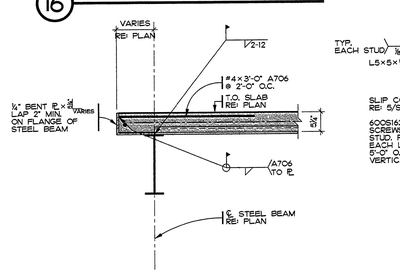
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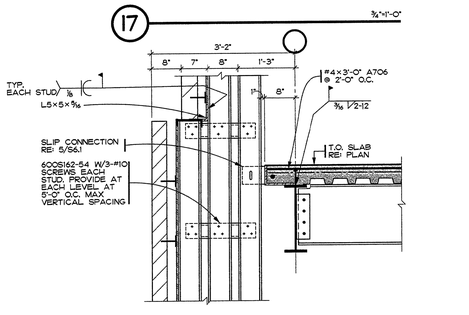
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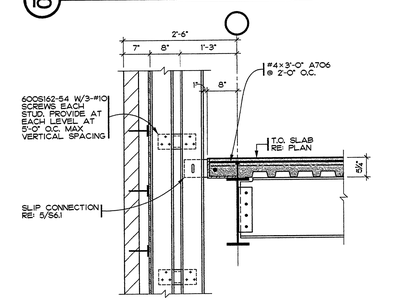
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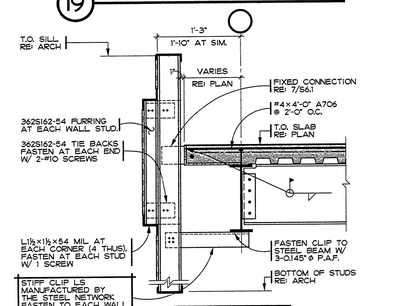
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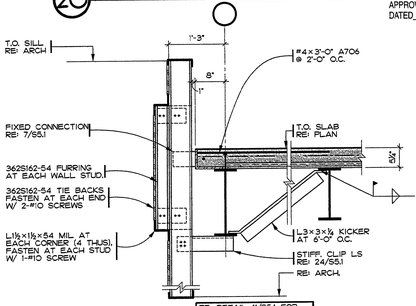
22 3/4"=1'-0"



23 3/4"=1'-0"



24 3/4"=1'-0"



25 3/4"=1'-0"

Davis Partnership Architects
2301 BLAKE ST., STE. #100
DENVER, COLORADO 80205
303.861.8555 F.303.861.3027
0137 MAIN STREET, UNIT C106
POST OFFICE BOX 2711
EDWARDS, COLORADO 81632
970.926.8960 F.970.926.8961
weyland enterprises, inc.
MedCap PROPERTIES
Structural Consultants Incorporated
3400 E. Bayaud Avenue #300
Denver, Colorado 80209
(303) 399-5154, FAX (303) 333-9501
SCI Job Number: 02-333

ISSUE/REVISION	DATE	NO.
DESIGN DEVELOPMENT	11/17/03	
50% CD'S	9/21/04	
95% CD'S	10/8/04	
100% CD'S	10/26/04	

COLORADO REGISTERED ARCHITECT 36269 10/26/04

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

Health ONE

MEDICAL OFFICE BUILDING

CITY OF AURORA BUILDING DIVISION APPROVED AS NOTED
DATED 11/2/04

CONSTRUCTION DOCUMENTS

FRAMING DETAILS

DATE	10/26/04
DRAWN BY	JEM
CHK'D BY	FEB
X-REF FILE	02-333_S51

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SHEET NUMBER:

S5.1

D.P.A. PROJECT # 02419.00

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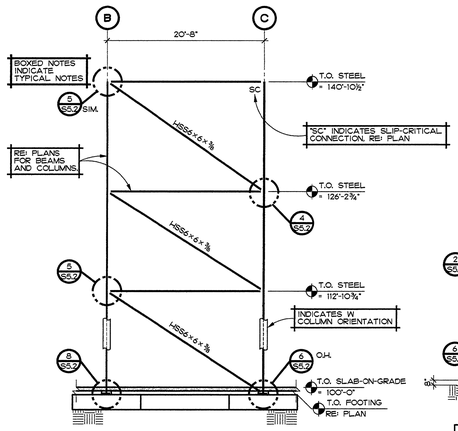
BRACE WELD SCHEDULE

BRACE SIZE	(EFFECTIVE) SEE NOTE 1 t_w	t_{w2}	L_w
HSS6×6	⅜"	9/16"	6"
HSS8×8	⅜"	9/16"	6"

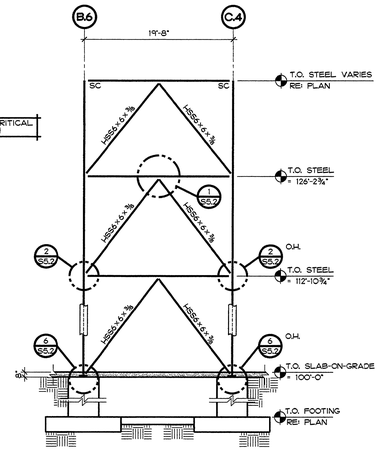
NOTES:
1. INCREASE WELD SIZE IN FIELD TO OBTAIN THE EFFECTIVE WELD SIZE SHOWN WITH REDUCTION CONSIDERED FOR GAP BETWEEN GUSSET PLATE AND HSS BRACE.

GUSSET PLATE SCHEDULE

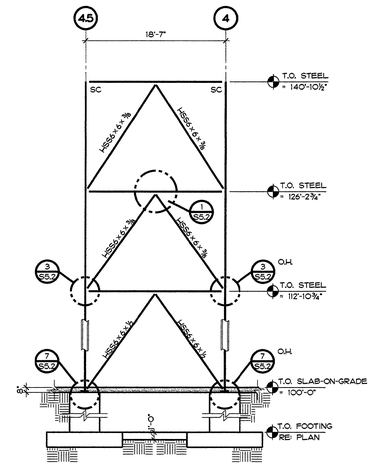
BRACE SIZE	GUSSET ℄ THICKNESS
HSS6×6	¾"
HSS8×8	¾"



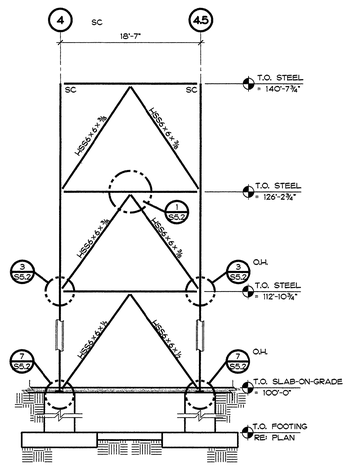
A BRACED FRAME ELEVATION ⅛"=1'-0"



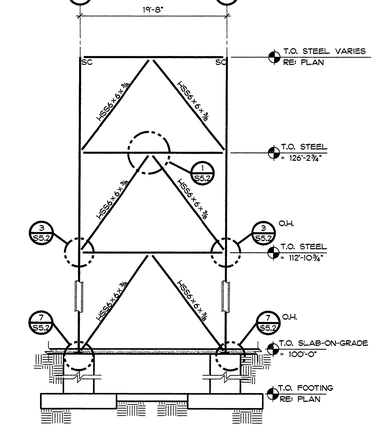
B BRACED FRAME ELEVATION ⅛"=1'-0"



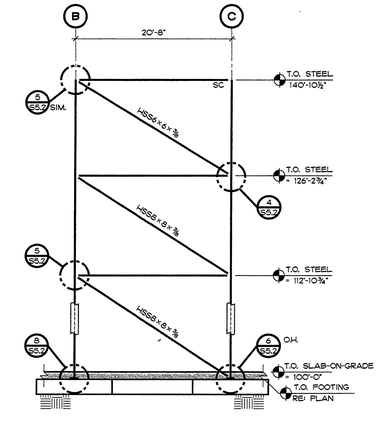
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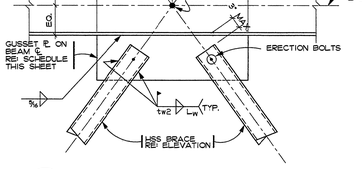
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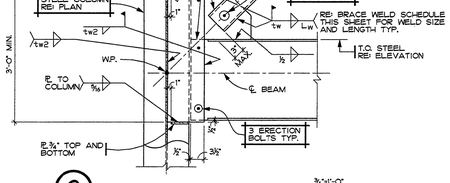
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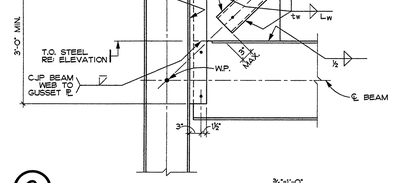
F FRAME ELEVATION ⅛"=1'-0"



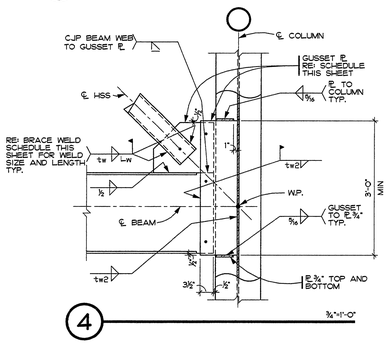
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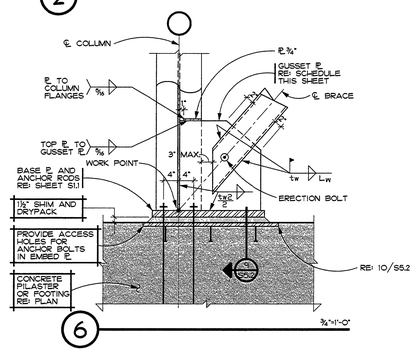
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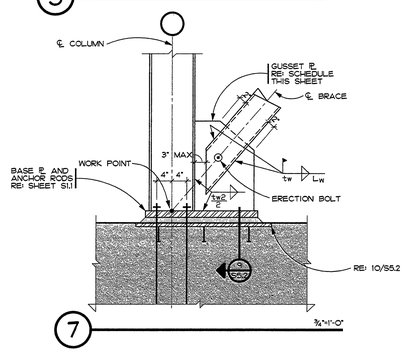
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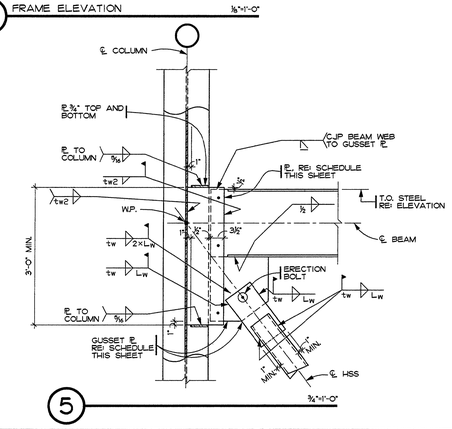
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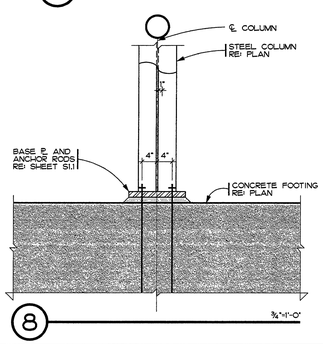
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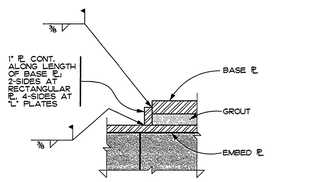
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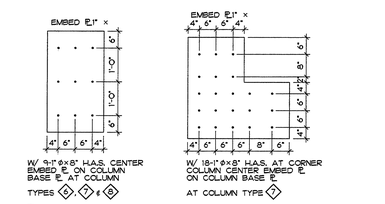
5 ¾"=1'-0"



8 ¾"=1'-0"



9 TYP. EACH SIDE OF BASE ℄ 1½"=1'-0"



10 ¾"=1'-0"

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 1/20/05

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SCI Job Number: 02-333

ISSUE/REVISION	DATE	NO.
50% CD'S	9/21/04	
95% CD'S	10/8/04	
100% CD'S	10/26/04	

COLORADO REGISTERED PROFESSIONAL ENGINEER 36269 FRANK E. BUMGARDNER JR. 10/26/04

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO
Health ONE
MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

BRACE FRAMES AND FRAMING DETAILS

DATE	10/26/04
DRAWN BY	JEM
CHK'D BY	FEB
X-REF FILE	02-333_S52

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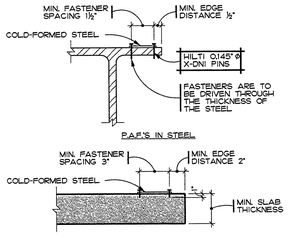
SHEET NUMBER:
S5.2
D.P.A. PROJECT # 02419.00

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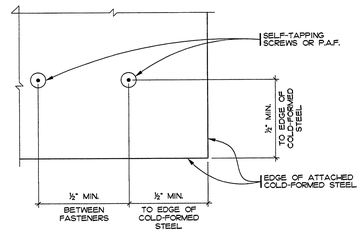
COLD-FORMED FRAMING GENERAL NOTES

1. The cold-formed steel framing materials are to be manufactured by a member of Steel Stud Manufacturers Association (SSMA) in accordance with ASTM A653, formerly A446, and C955. Material sizes and mil thickness are indicated on the drawings. Minimum section properties are to meet or exceed SSMA Product Technical Information publication and referenced in ICBO report No. 4943-P.
2. The minimum yield strength of the cold-formed steel framing components shall be as follows:
   A. 54 mil or thicker = 50,000 psi.
   B. 43 mil or thinner = 33,000 psi.
3. The cold-formed steel framing may be punched or unpunched.
4. The cold-formed steel framing has been designed in accordance with the following codes and specifications:
   A. IBC 2003
   B. AISI "Specifications for design of cold-formed steel structural members"- 2001 (WASPEC).
5. All connections shall be welded, screwed or powder fastened as indicated on these drawings.
   A. Welds- all welded connections shall be performed in accordance with the latest edition of the AWS D1.3 specification for welding sheet steel in structures. All welding shall be performed by AWS certified welders. All welds shall be cleaned and coated with rust inhibitive zinc paint. The minimum weld length shall be 1" for cold-formed steel to cold-formed steel connections.
   B. Screws- #10 self drilling screws, unless otherwise noted on drawings, manufactured by Buildex or Hilti and installed in accordance with the fastener manufacturer's specifications. Screws shall be spaced a minimum of 1/2" between adjacent screws and from metal edges. (RE: 2/S6.1)
   C. Powder actuated fasteners (P.A.F.) 0.145" minimum shank diameter P.A.F. as manufactured by Ramset or Hilti and installed in accordance with the fastener manufacturer's specifications.(RE: 1/S6.1)
      1. Provide minimum 1" long P.A.F. for cold-formed steel connections to concrete. P.A.F. in concrete shall be spaced a minimum of 3" between adjacent P.A.F. and a minimum of 2" from concrete edges. Minimum P.A.F. embedment in concrete shall be 1".
      2. Provide minimum 1/2" long P.A.F. for cold-formed steel connections to structural steel. P.A.F. shall be spaced a minimum 1-1/2" between adjacent P.A.F. in structural steel and a minimum of 1/2" from steel edges. The P.A.F. point shall be driven through the back of the structural steel member.
6. Provide Cold-Rolled channel bridging at 4'-0" O.C. vertically in wall framing. (RE: 6/S6.1)
7. All members shall be cut squarely for attachment to perpendicular members or slope cut as required for an angular fit against abutting members.
8. Field cutting of studs shall be done by sawing or shearing. Torch cutting of cold-formed steel members is not permitted.
9. Do not cut or splice cold-formed steel framing members unless indicated by these drawings.
10. Do not bear or connect cold-formed steel members within 9" inches of the punched openings in the member webs (RE: 12/S6.1)unless the members are reinforced with a minimum 12" long unpunched track or stud at the punched opening. The track or stud reinforcing piece shall be the same size and mil thickness as the punched member. Fasten the reinforcing piece to the member with a minimum of four screws.
11. Additional temporary bracing and shoring shall be provided as required to stabilize the framing and to support construction loads. Temporary bracing shall remain in place until permanent bracing is installed and/or additional construction loads are removed.
12. The contractor shall provide cold-formed steel members at the size and spacing indicated on these drawings. Larger sizes and/or closer spacing may be substituted provided the substitutions are coordinated with the project Architectural and Structural drawings.
13. These drawings are intended to indicate the member sizes and connections relevant to the cold-formed steel framing. These drawings are not intended to be "dimensioned" drawings and should not be used for material take-off lengths. Refer to the architectural drawings for required member lengths.
14. Refer to architectural drawings for all dimensions and locations of openings in walls.
15. Sheathing on the cold-formed steel framings shall be installed as indicated in the Architectural drawings and specifications, and in accordance with ASTM C955. Joints in sheathing must be installed at slip locations in the cold-formed steel framing as indicated in the Architectural Drawings and specifications.
16. These drawings do not include the design and detailing for the interior cold-formed steel framing.
17. Unless noted otherwise, all track sections shall as a minimum match the wall studs in size and mil thickness.
18. Explanation of section designated used:

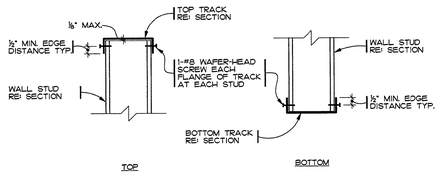
SECTION IDENTIFICATION — X / S-X — SHEET LOCATION OF DETAILS



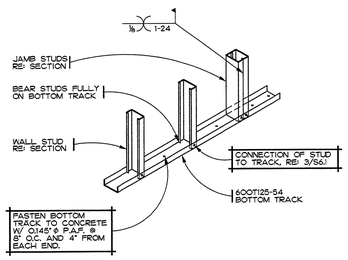
1 TYPICAL POWDER ACTUATED FASTENER (P.A.F.) DETAIL — NOT TO SCALE



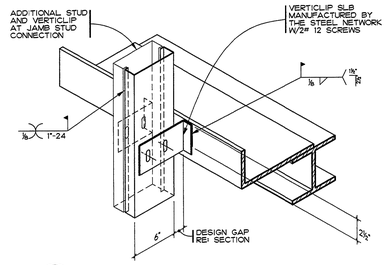
2 TYPICAL FASTENER EDGE SPACING DETAIL — NOT TO SCALE



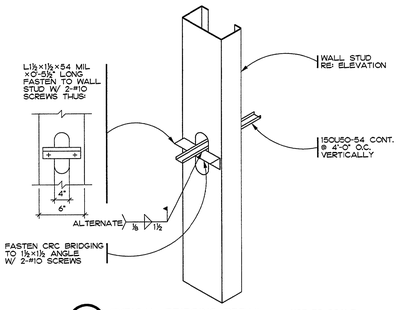
3 TYPICAL CONNECTION OF STUD TO TRACK — NOT TO SCALE



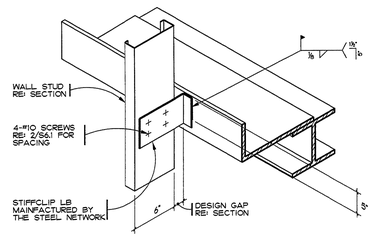
4 BOTTOM TRACK ATTACHMENT — NOT TO SCALE



5 TYPICAL SLIDE CLIP — NOT TO SCALE



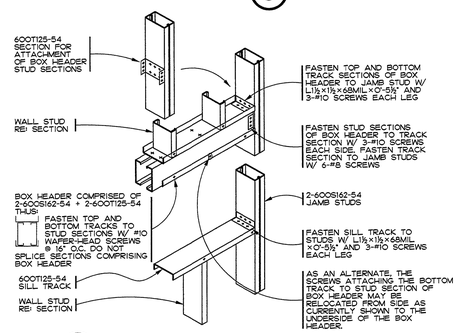
6 TYPICAL BRIDGING DETAIL — NOT TO SCALE



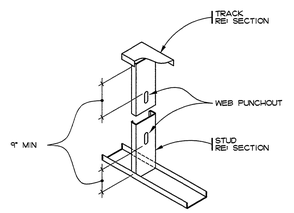
7 TYPICAL FIXED CLIP CONNECTION — NOT TO SCALE

NOT USED

8 — NOT TO SCALE



9 TYPICAL BOXED HEADER — NOT TO SCALE



12 PUNCHOUT LOCATION — NOT TO SCALE

CITY OF AURORA
BUILDING DIVISION
APPROVED AS NOTED
DATED 12/2/05

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SCI Job Number: 02-333

ISSUE/REVISION	DATE	NO.
DESIGN DEVELOPMENT	11/17/03	
50% CD'S	9/21/04	
95% CD'S	10/8/04	
100% CD'S	10/26/04	

COLORADO REGISTERED ARCHITECT JAMES E. MATTHEW 36269 PROFESSIONAL ENGINEER 12/2/05

MEDICAL CENTER OF AURORA SOUTH
AURORA, COLORADO

HealthONE

MEDICAL OFFICE BUILDING

CONSTRUCTION DOCUMENTS

COLD FORMED FRAMING DETAILS AND GENERAL NOTES

DATE	10/26/04
DRAWN BY	JEM
CHK'D BY	FEB
X-REF FILE	02-333-S61

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SHEET NUMBER:

S6.1

D.P.A. PROJECT # 02419.00

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